

DEPARTMENT OF THE ARMY U.S. ARMY CORPS OF ENGINEERS, SAVANNAH DISTRICT 100 W. OGLETHORPE AVENUE SAVANNAH, GEORGIA 31401-3604

August 10, 2021

Regulatory Division SAS-2009-00344

JOINT PUBLIC NOTICE Savannah District/State of Georgia

The Savannah District has received an application for a Department of the Army Permit, pursuant to Section 10 of the Rivers and Harbors Act of 1899 (33 U.S.C. 403) and Section 404 of the Clean Water Act (33 U.S.C. 1344), as follows:

Application Number: SAS-2009-00344

Applicant: Mr. Eric Duff

State Environmental Administrator Georgia Department of Transportation 600 West Peachtree Street NW, 16th Floor Atlanta, Georgia 30308

Location of Proposed Work: In waters and wetlands located along approximately 6.14 miles of State Route (SR) 25/United States Highway (US) 17, 5.9 miles northeast of Brunswick, Glynn County, Georgia. The approximate project mid-point would be located at Latitude 31.2701, Longitude -81.4370.

Description of Work Subject to the Jurisdiction of the U.S. Army Corps of Engineers: Georgia Department of Transportation (GDOT), Project Identification Numbers (PI) 0016985, 0009874, and 532650 propose to widen and improve SR 25/US 17 from the intersection of County Road (CR) 372/Yacht Road, north, to approximately 1,000 feet north of the intersection of SR 25/US 17 and SR 99/Grants Ferry Road. The existing two-lane section would be improved to include four 12-foot travel lanes with a 24-foot raised median and 10-foot rural outside shoulders (6.5-foot paved) that would accommodate bicycles. The proposed design speed is 55 miles per hour. Auxiliary turn lanes would be provided at major intersections and proposed median openings. In addition, right turn lanes would be included at all crossroads. The existing bridges along SR 25/US 17 over Thornhill Creek and Wally's Leg Branch would be replaced with new four-lane structures. Both proposed bridge structures would be 60 feet in length and 88 feet in width. The existing right-of-way varies from 70 to 250 feet. The proposed project right-of-way would vary between 200 and 250 feet.

The project would result in impacts to 15.725 acres of wetlands and 503 linear feet of streams. Proposed project impacts would be compensated through the purchase of 0.1 Grandfathered Tidal Wetland Credit, 76.4 Grandfathered Wetland Credits, and 2,700 Grandfathered Stream Credits from Corps approved mitigation banks serving the

Brunswick River – Atlantic Ocean (and Altamaha River – Atlantic Ocean watersheds (Hydrologic Unit Codes 0307020302 and 0307010605, respectively).

BACKGROUND

On June 26, 2017, the Federal Highway Administration (FHWA) approved an Environmental Assessment/Finding of No Significant Impact which covered the entirety of the project under one PI number (PI 532650). PI 532650 was federally funded. Since that time, GDOT has assigned two additional PI numbers, PI 0016895 and PI 0009874 to allow for phased construction. PI 0009874 (a roundabout) would remain federally funded; however, state funding would be used for the SR 25 mainline widening construction for PI 0016895 and PI 532650. Each PI is described below:

- Unit 1 of construction is PI 0016895 SR 25 from CR 372/Yacht Rd to CR 415/Harry Driggers Boulevard and is approximately 3.86 miles in length.
- Unit 2 of construction is PI 532650 SR 25/US 17 from CR 415/Harry Driggers Boulevard to SR 99 and is approximately 1.78 miles in length.
- Unit 3, PI 0009874, includes the construction of a roundabout at the intersection of SR 25/US17 and SR 99. The roundabout is a rural to urban roundabout in that traffic entering the roundabout in the north or south direction would be two lanes. Exiting the roundabout to continue east or west along SR 99 would be one lane. The roundabout would have a truck apron and 12-foot-wide travel lanes with bicycle and pedestrian accommodations. The center island would be raised and grassed with lighting. The roundabout limits are approximately 0.5 mile in length.

This Joint Public Notice announces a request for authorizations from both the U.S. Army Corps of Engineers and the State of Georgia. The applicant's proposed work may also require local governmental approval.

STATE OF GEORGIA

<u>Water Quality Certification</u>: The Georgia Department of Natural Resources, Environmental Protection Division (Georgia EPD), will review the proposed project for water quality certification, in accordance with the provisions of Section 401 of the Clean Water Act. Prior to issuance of a Department of the Army permit for a project location in, on, or adjacent to the waters of the State of Georgia, review for Water Quality Certification is required. A reasonable period of time, which shall not exceed on year, is established under the Clean Water Act for the State to act on a request for Water Quality Certification, after which, issuance of such a Department of the Army permit may proceed. The applicant provided notification to Georgia EPD and requested a Section 401 Water Quality Certification pre-filing meeting via email dated July 14, 2021. <u>State-owned Property and Resources</u>: The applicant may also require assent from the State of Georgia, which may be in the form of a license, easement, lease, permit or other appropriate instrument.

<u>Georgia Coastal Management Program:</u> Prior to the Corps making a final permit decision on this application, the project must be certified by the Georgia Department of Natural Resources, Coastal Resources Division, to be consistent with applicable provisions of the State of Georgia Coastal Management Program (15 CFR 930). Anyone wishing to comment on Coastal Management Program certification of this project should submit comments in writing within 30 days of the date of this notice to the Federal Consistency Coordinator, Coastal Management Program, Coastal Resources Division, Georgia Department of Natural Resources, One Conservation Way, Brunswick, Georgia 31523-8600 (Telephone 912-264-7218).

U.S. ARMY CORPS OF ENGINEERS

The Savannah District must consider the purpose and the impacts of the applicant's proposed work, prior to a decision on issuance of a Department of the Army Permit.

<u>Cultural Resources Assessment</u>: There were nine (9) historic resources identified within the permit area; of those, one (1), The Gillman House, was demolished; two (2), the Needwood Baptist Church and School and the Hofwyl-Broadfield Plantation, are listed on the National Register of Historic Places (NRHP); six (6) the Miller Farm, Tuya House, Berry House, Aultman Property, Smith House and Store, and the New Hope Plantation, are recommended eligible for inclusion in the NRHP. Twenty-six (26) archaeological sites were identified within the permit area; of those one (1), site 9GN433, was identified for impact and mitigated; five (5), 9GN411, 9GN412, 9GN413, 9GN414, 9GN415 were recommended eligible for inclusion in the NRHP; and twenty (20), 9GN416, 9GN417, 9GN418, 9GN419, 9GN420, 9GN422, 9GN423, 9GN425, 9GN426, 9GN427, 9GN429, 9GN430, 9GN431, 9GN432, 9GN434, 9GN435, 9GN436, 9GN437, 9GN438, and 9GN87, were recommended as unknown eligibility for inclusion in the NRHP due to a lack of significant date potential.

A GDOT memo to file was completed on May 28, 2021, referencing a FHWA No Adverse Effect to Historic Properties determination transmitted to the State Historic Preservation Officer (SHPO) on October 18, 2016. By letter dated November 3, 2016, the SHPO concurred with FHWA's finding of no adverse effect to historic resources

A GDOT In-House Survey Report and an Addendum to a previous Archaeological Survey Report was submitted to the SHPO on June 23, 2021. A Memorandum of Agreement was executed by the SHPO, FHWA, and GDOT in April 2017 as evidence that the lead federal agency has taken into account the effects of the undertaking on historical properties and afforded the Advisory Council on Historic Preservation (ACHP) an opportunity to comment to mitigate the adverse effects. ACHP concurrence was received on July 12, 2021.

Essential Fish Habitat (EFH): EFH is present within the project survey area. The following resources with tidal influence have been classified as EFH: Canal (CL) 1, Perennial Stream (PS) 2, OW 7, and PS 9. CL 1, PS 2, and PS 9 are considered low quality habitat for estuarine-dependent species of the snapper-grouper complex. OW 7 (Thornhill Creek) is considered low-to-moderate quality habitat for estuarine dependent species of the snapper-grouper complex as well as penaeid shrimp, which includes three species of shrimp in the Panaeidae family. Based on a completed EFH Screening Form and supplemental information, a "no effect" determination to EFH was recommended for EFH in CL 1, PS 2, and PS 9 and an "adverse effect" determination to EFH was made for EFH in OW 7.

A request for EFH determination concurrence was sent to NOAA on June 30, 2016, and May 25, 2017. Via correspondence dated June 16, 2017, NOAA concurred with the determination that the project would have an adverse effect on EFH and provided recommendations pursuant to authorities of the Fish and Wildlife Coordination Act and the Magnuson- Stevens Act. Per the June 2017 correspondence, the proposed avoidance and minimization measures, as well as with the purchase of mitigation credits for salt marsh and freshwater wetland impact, would compensate for impacts to EFH. Proposed impacts to salt marsh and freshwater wetlands remain unchanged since the previous determination.

Endangered Species: A review of the U.S. Fish and Wildlife Service (USFWS) Information for Planning and Consultation tool indicated the following listed species may occur in project area: wood stork (*Mycteria americana*), shortnose sturgeon (*Acipenser brevirostrum*), Atlantic sturgeon (*Acipenser oxyrinchus oxyrinchus*), North Atlantic right whale (*Eubalaena glacialis*), Kemp's Ridley sea turtle (*Lepidochelys kempii*), West Indian manatee (*Trichechus manatus*), loggerhead sea turtle (*Caretta caretta*), green sea turtle (*Chelonia mydas*), hawksbill sea turtle (*Eretmochelys imbricata*), leatherback sea turtle (*Dermochelys coriacea*), and eastern indigo snake (*Drymarchon couperi*) and eastern black rail (*Laterallus jamaicensis jamiaicensis*).

A determination of "No Effect" was made for the West Indian manatee, eastern indigo snake, green sea turtle, Hawksbill sea turtle, Kemp's Ridley sea turtle, leatherback sea turtle, loggerhead sea turtle, shortnose sturgeon, Atlantic right whale, and Atlantic sturgeon. A determination of "may affect, not likely to adversely affect" was made for the wood stork and eastern black rail. Per this effect determination, informal consultation under of the Section 7 of the Endangered Species Act was completed with the USFWS on July 28, 2021.

Pursuant to Section 7(c) of the Endangered Species Act of 1973, as amended (16 U.S.C. § 1531 et seq.), we request information from the U.S. Department of the Interior,

USFWS, the U.S. Department of Commerce, National Oceanic and Atmospheric Administration, NMFS; or, any other interested party, on whether any species listed or proposed for listing may be present in the area.

<u>Public Interest Review</u>: The decision whether to issue a permit will be based on an evaluation of the probable impact including cumulative impacts of the proposed activity on the public interest. That decision will reflect the national concern for both protection and utilization of important resources. The benefit, which reasonably may be expected to accrue from the proposal, must be balanced against its reasonably foreseeable detriments. All factors, which may be relevant to the proposal will be considered including the cumulative effects thereof; among those are conservation, economics, aesthetics, general environmental concerns, wetlands, historic properties, fish and wildlife values, flood hazards, flood plain values, land use, navigation, shoreline erosion and accretion, recreation, water supply and conservation, water quality, energy needs, safety, food and fiber production, mineral needs, considerations of property ownership and in general, the needs and welfare of the people.

<u>Consideration of Public Comments</u>: The Corps is soliciting comments from the public; federal, state, and local agencies and officials; Native American Tribes; and other interested parties in order to consider and evaluate the impacts of this proposed activity. Any comments received will be considered by the Corps to determine whether to issue, modify, condition or deny a permit for this proposal. To make this decision, comments are used to assess impacts on endangered species, historic properties, water quality, general environmental effects, and the other public interest factors listed above. Comments are used in the preparation of an Environmental Assessment and/or an Environmental Impact Statement pursuant to the National Environmental Policy Act. Comments are also used to determine the need for a public hearing and to determine the overall public interest of the proposed activity.

<u>Application of Section 404(b)(1) Guidelines</u>: The proposed activity involves the discharge of dredged or fill material into the waters of the United States. The Savannah District's evaluation of the impact of the activity on the public interest will include application of the guidelines promulgated by the Administrator, Environmental Protection Agency, under the authority of Section 404(b) of the Clean Water Act.

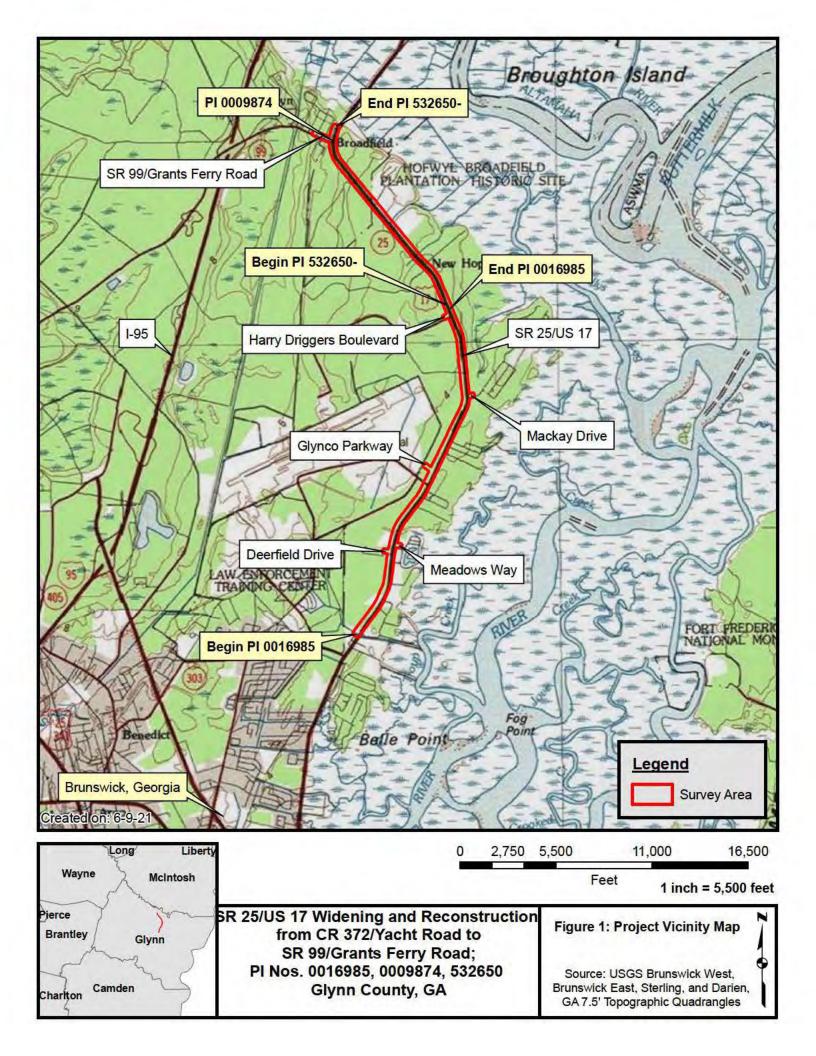
<u>Public Hearing</u>: Any person may request, in writing, within the comment period specified in this notice, that a public hearing be held to consider this application for a Department of the Army permit. Requests for public hearings shall state, with particularity, the reasons for requesting a public hearing. The decision whether to hold a public hearing is at the discretion of the District Engineer, or his designated appointee, based on the need for additional substantial information necessary in evaluating the proposed project.

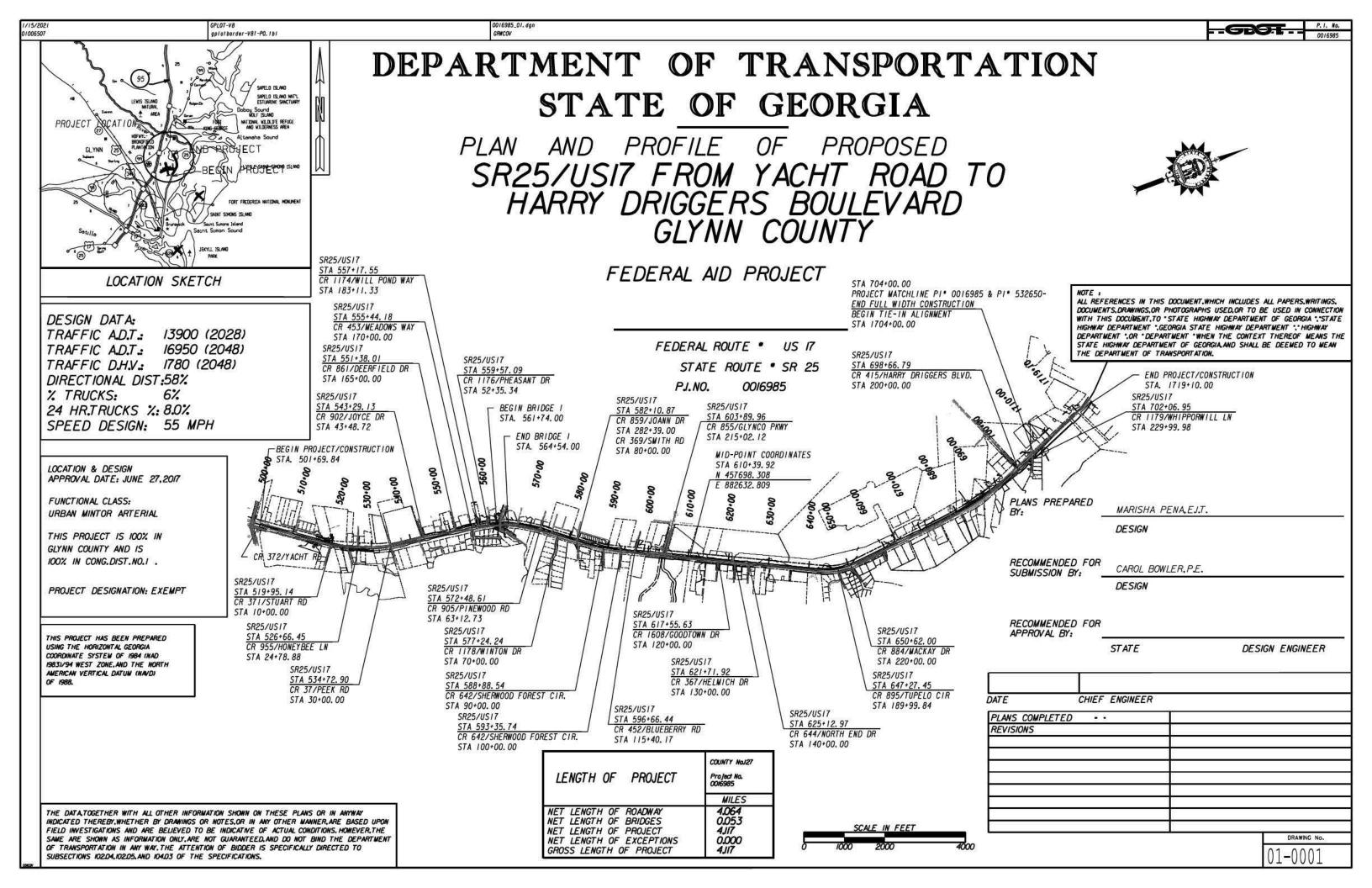
<u>Comment Period</u>: Anyone wishing to comment on this application for a Department of the Army Permit should submit comments by email to <u>brian.moore@usace.army.mil</u>. Alternatively, you may submit comments in writing to the Commander, U.S. Army Corps of Engineers, Savannah District, Attention: Mr. Brian Moore, 100 W. Oglethorpe Avenue, Savannah, Georgia 31401-3604, within 30 days from the date of this notice. Please refer to the applicant's name and the application number in your comments.

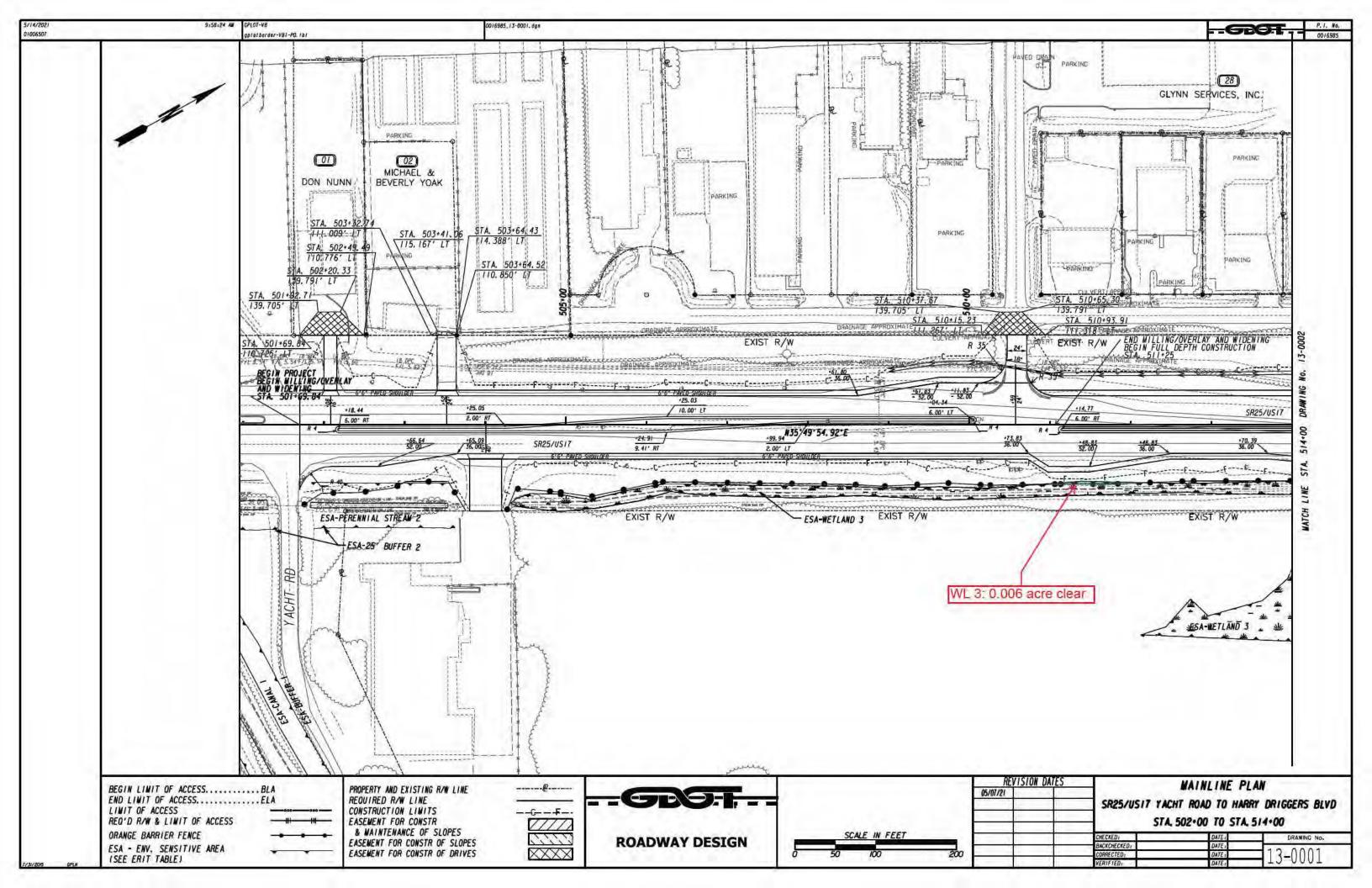
If you have any further questions concerning this matter, please contact Mr. Brian Moore, Project Manager, Management Branch at 912-652-5349.

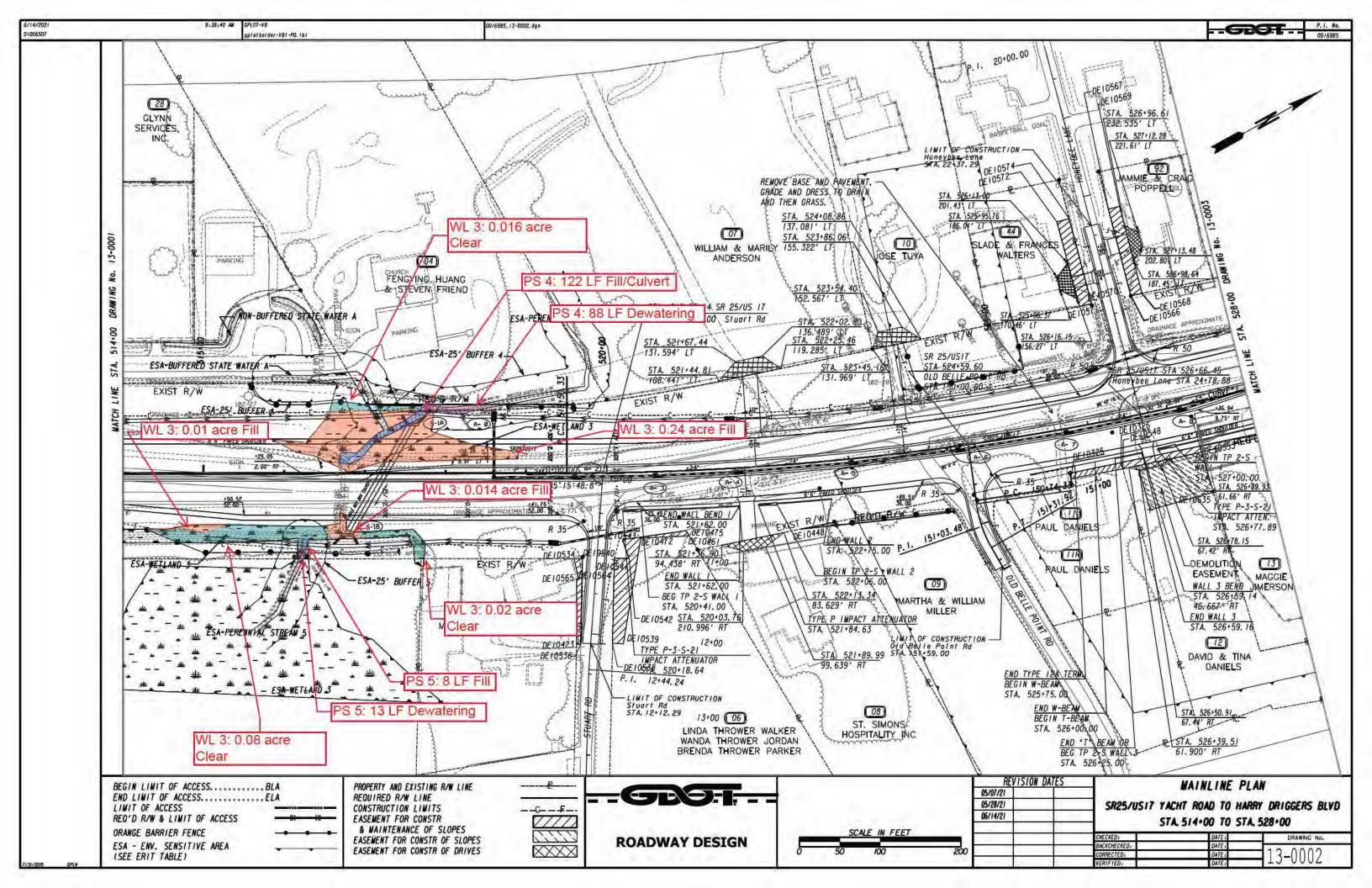
Enclosures

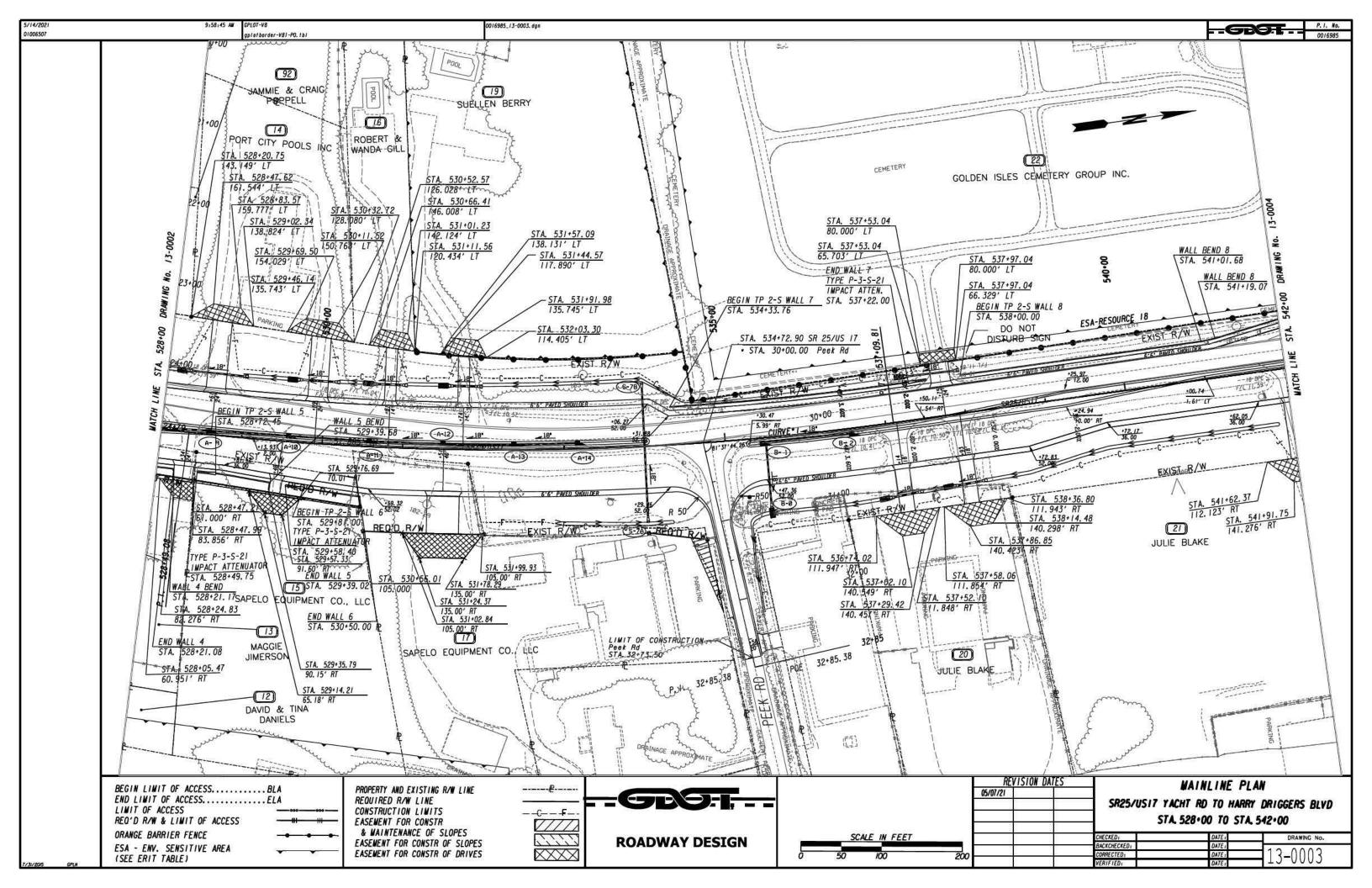
- 1. Project Vicinity Map
- 2. Construction Plans with Wetland Impacts

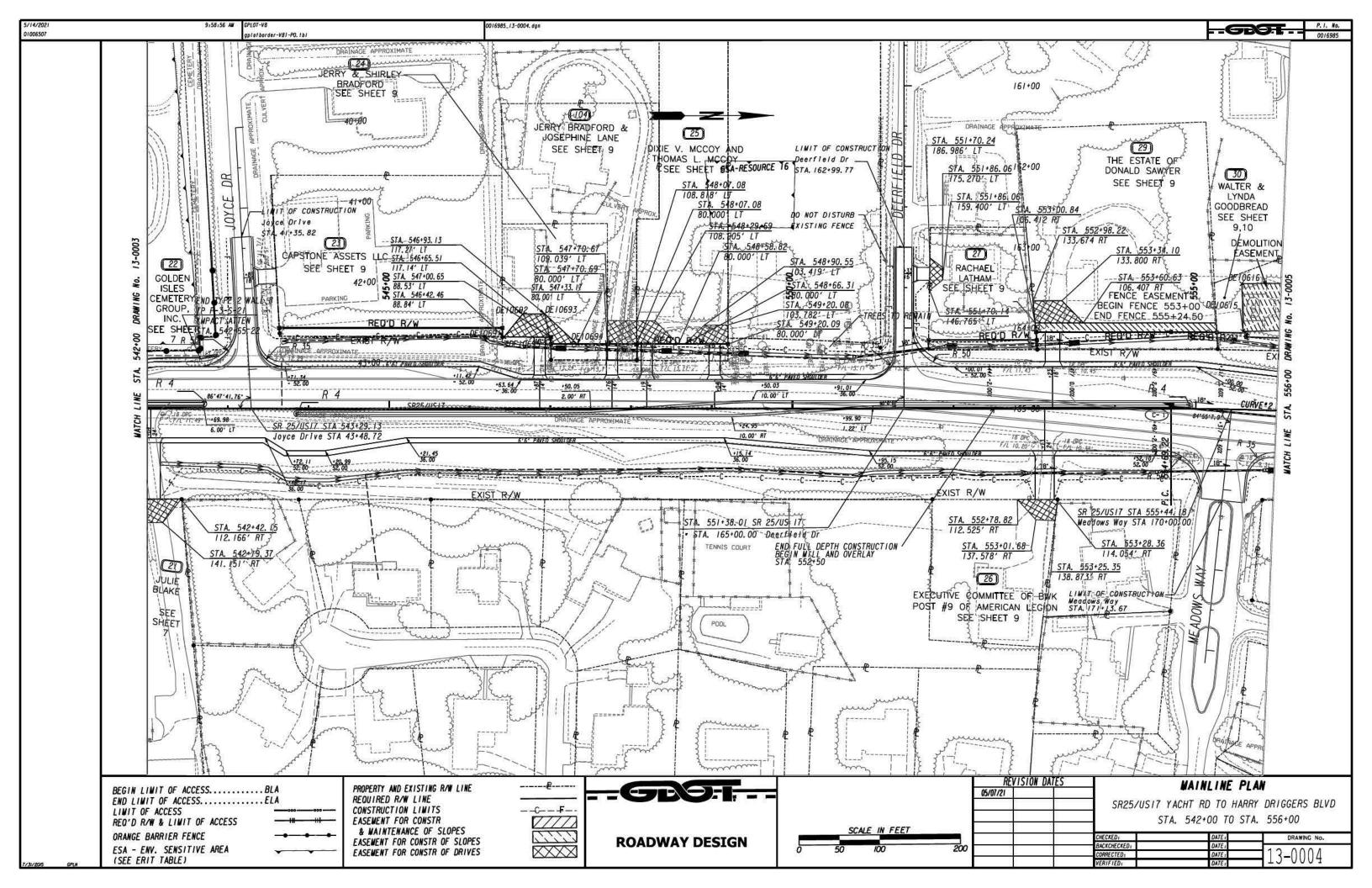


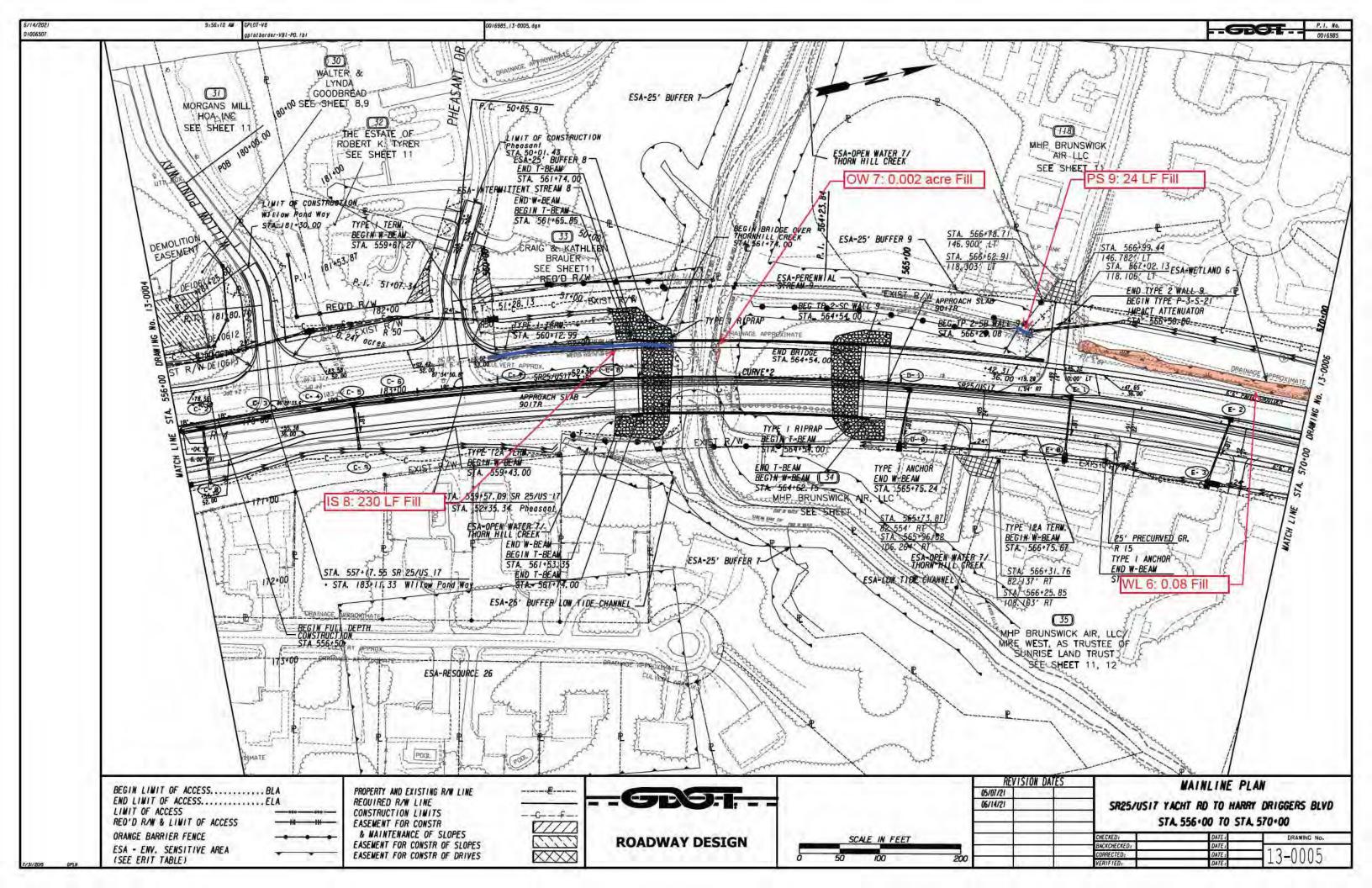


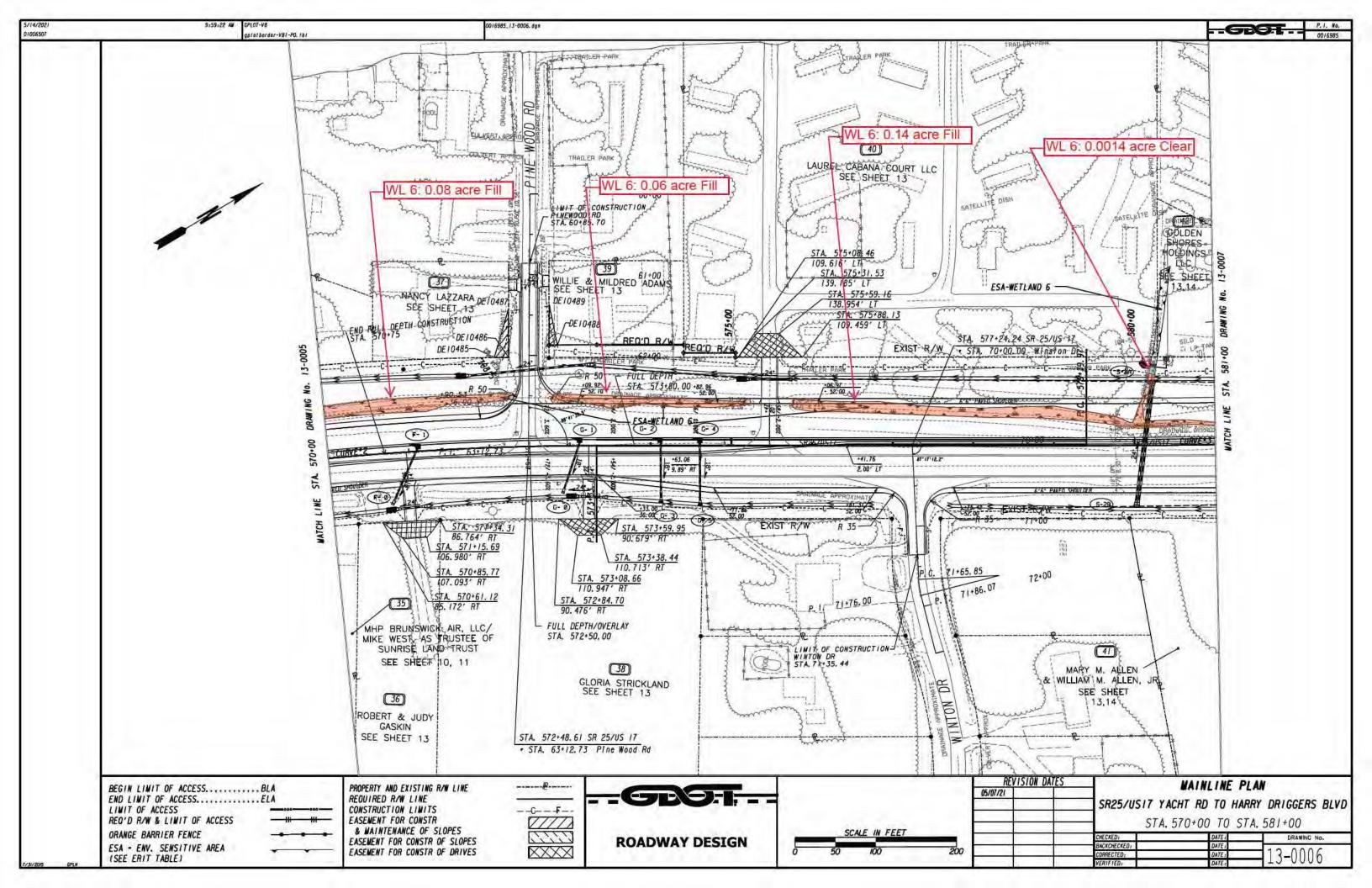


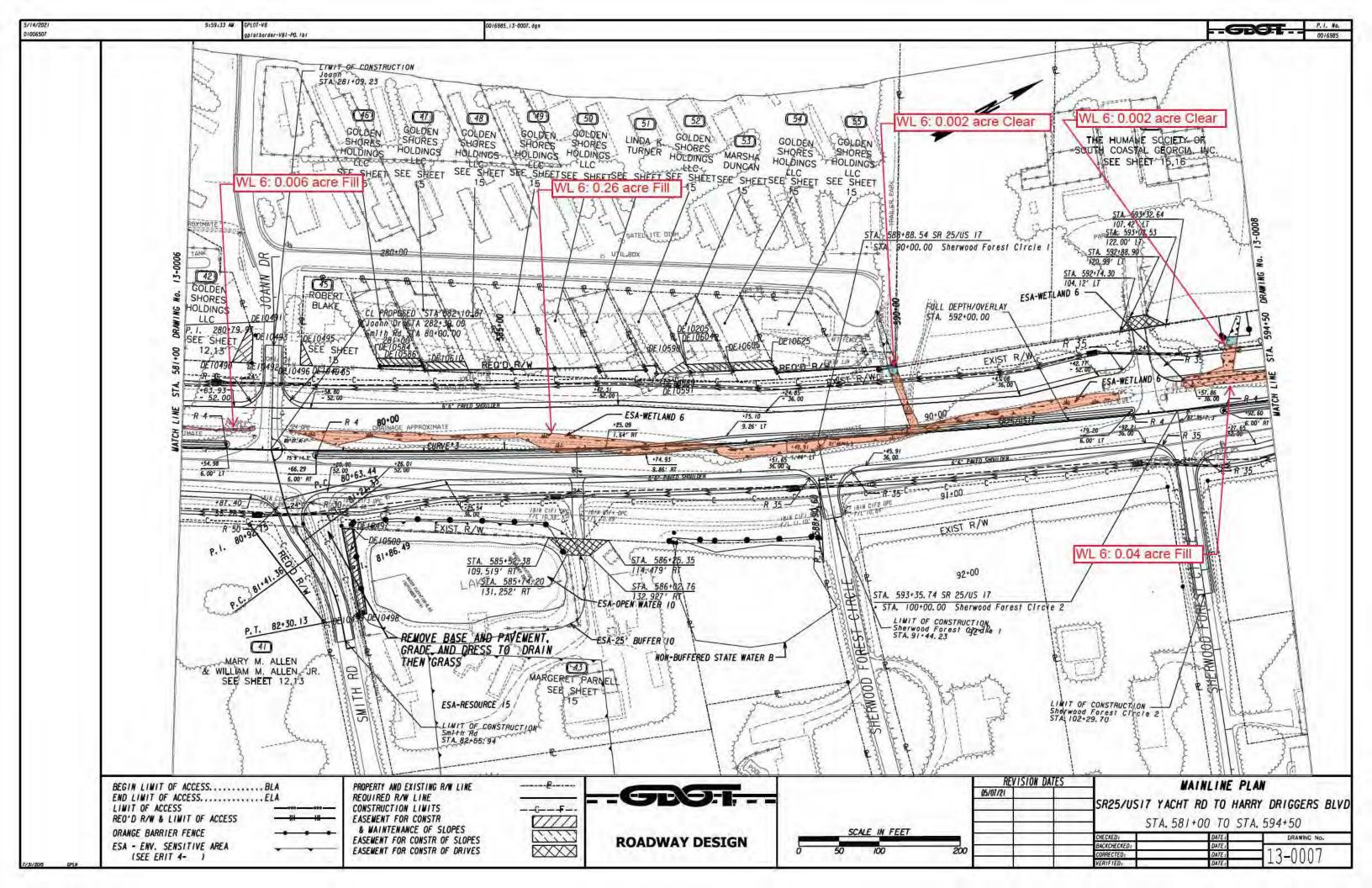


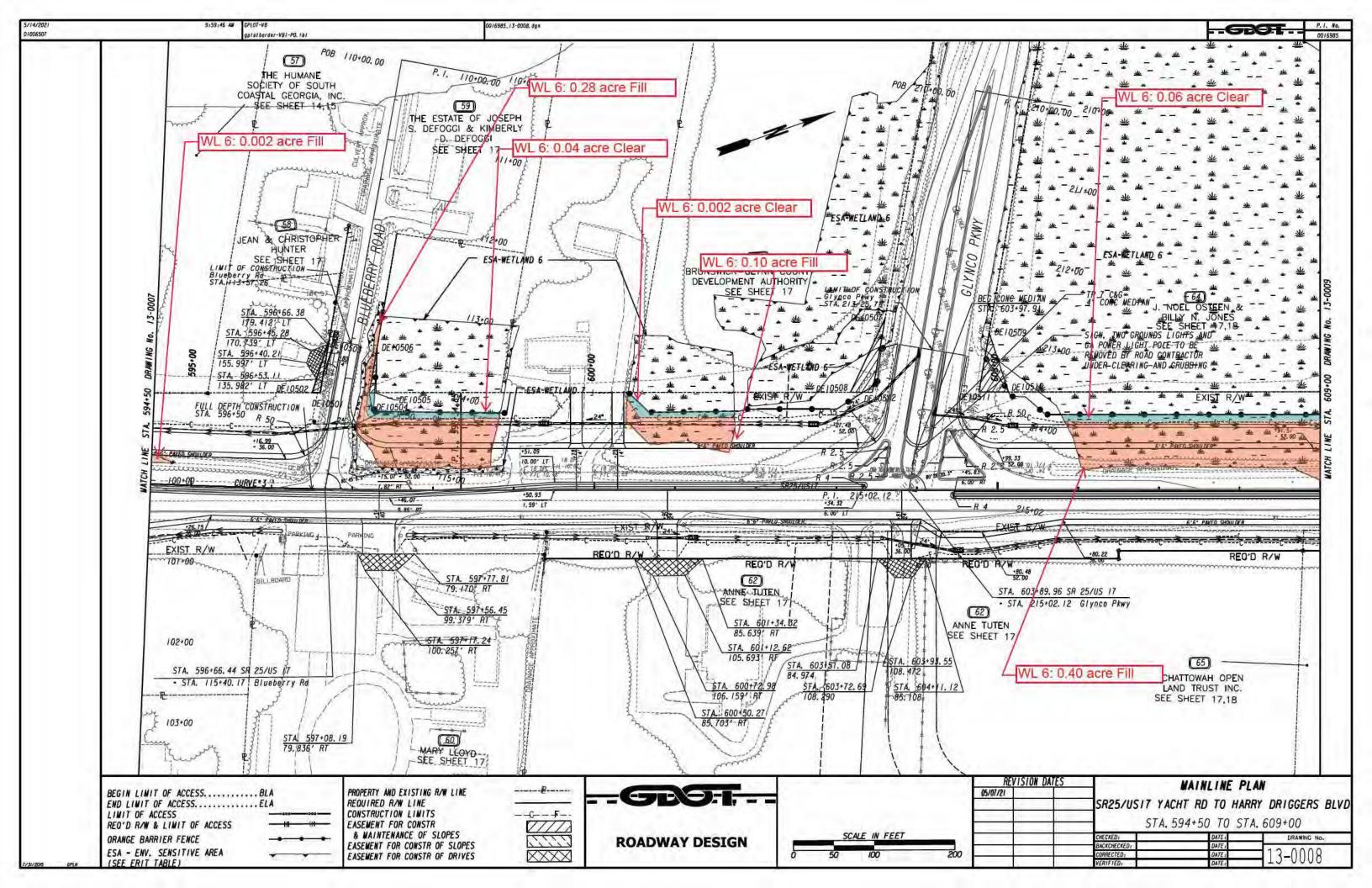


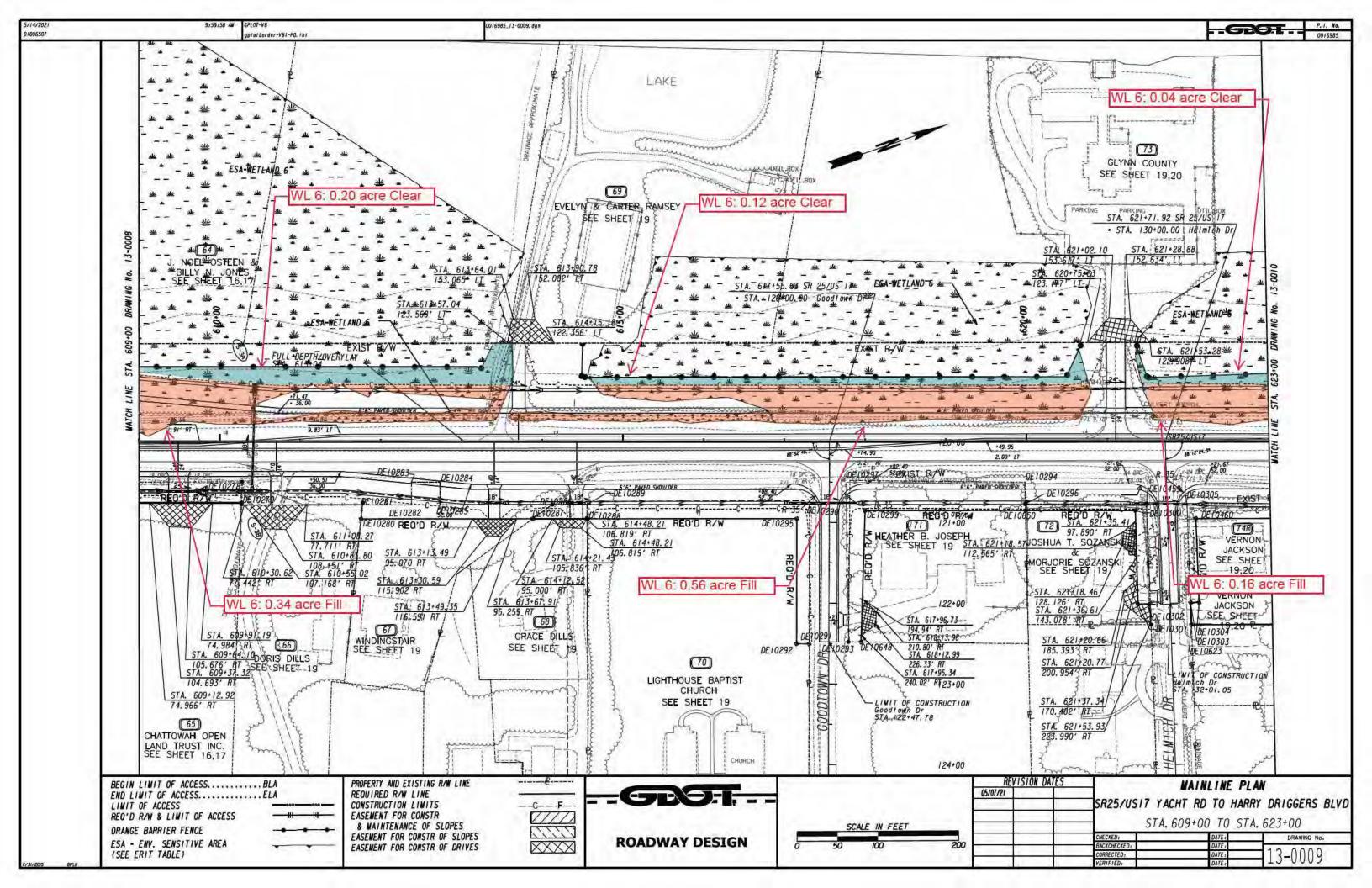


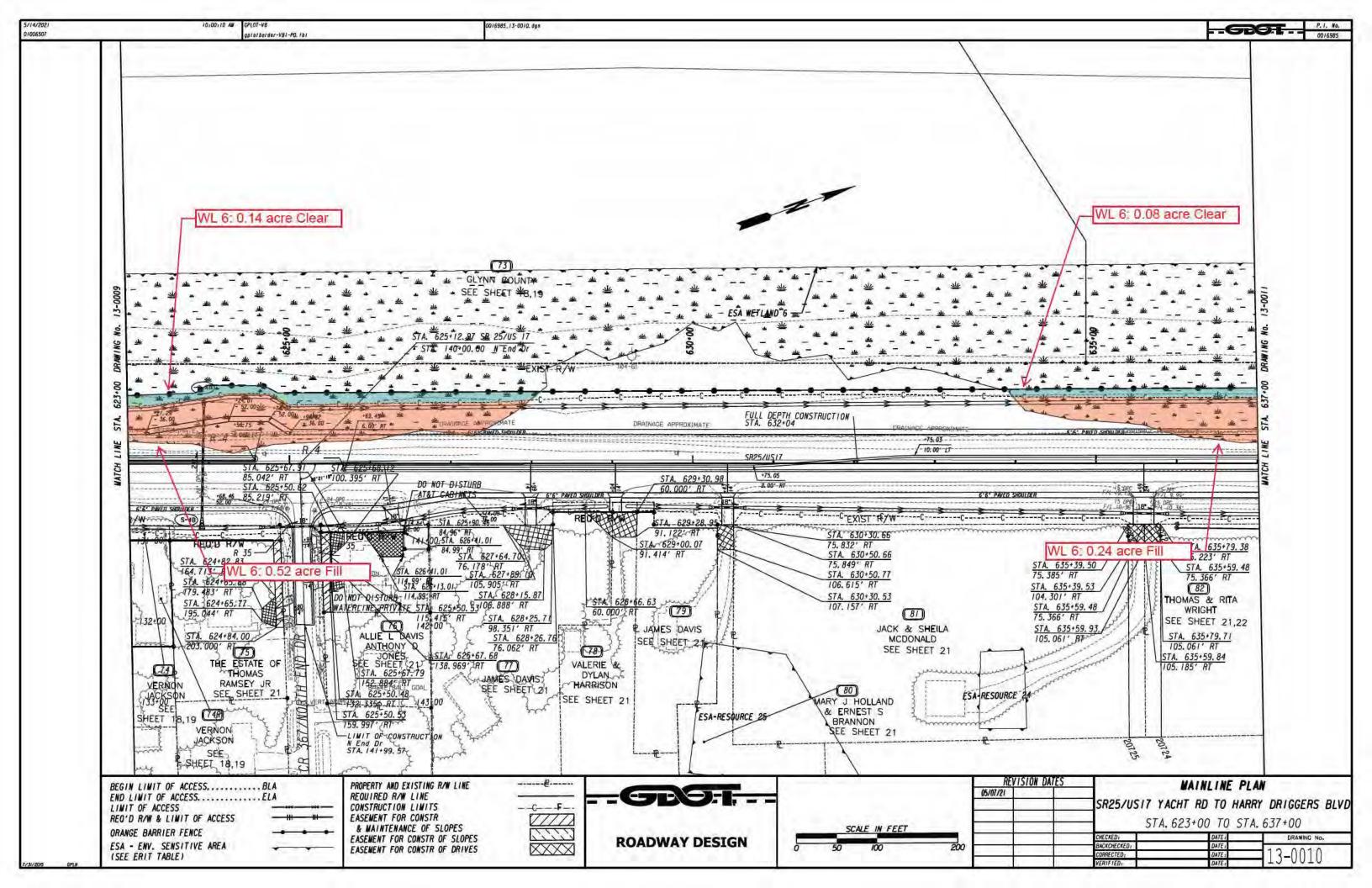


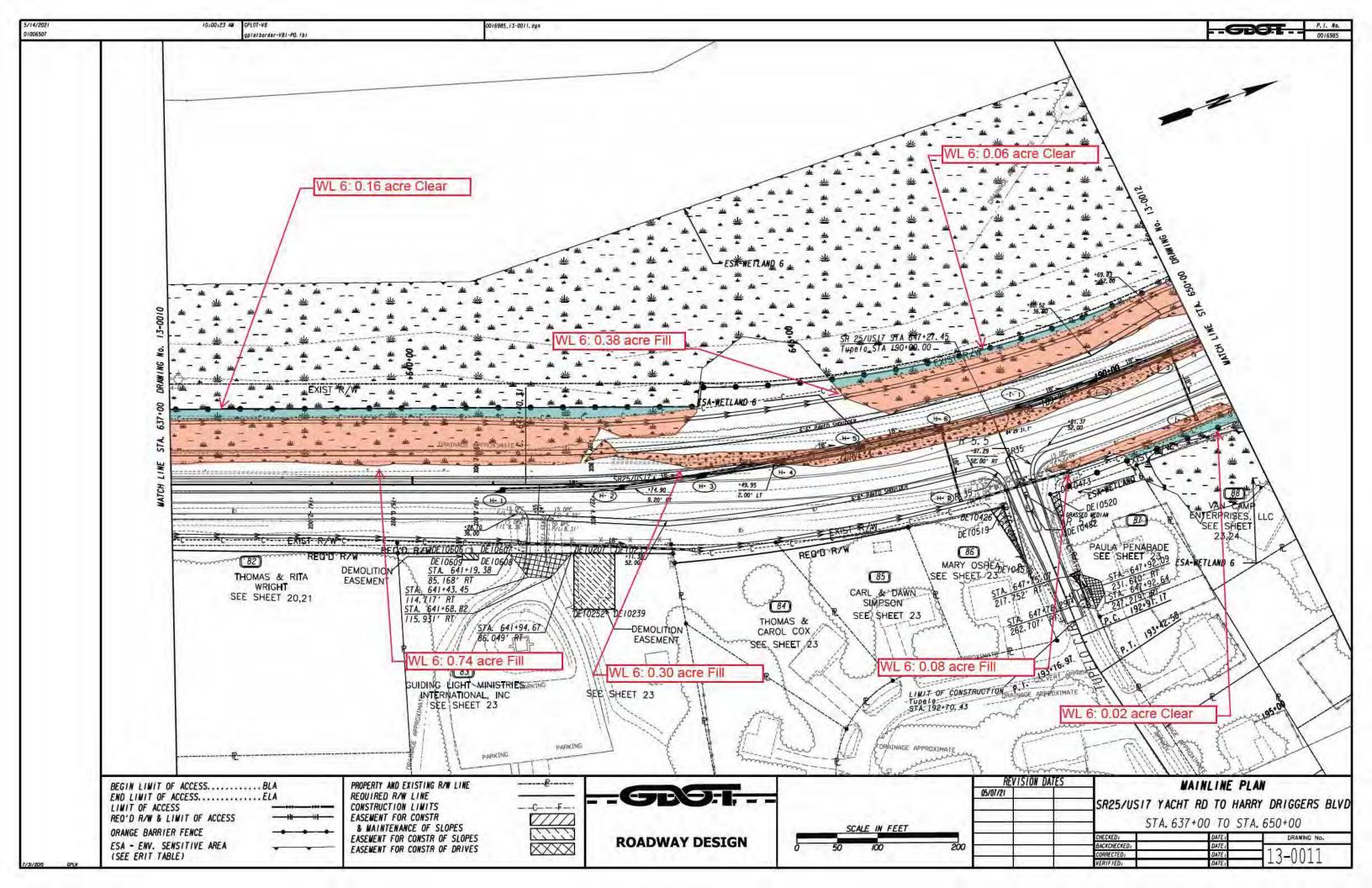


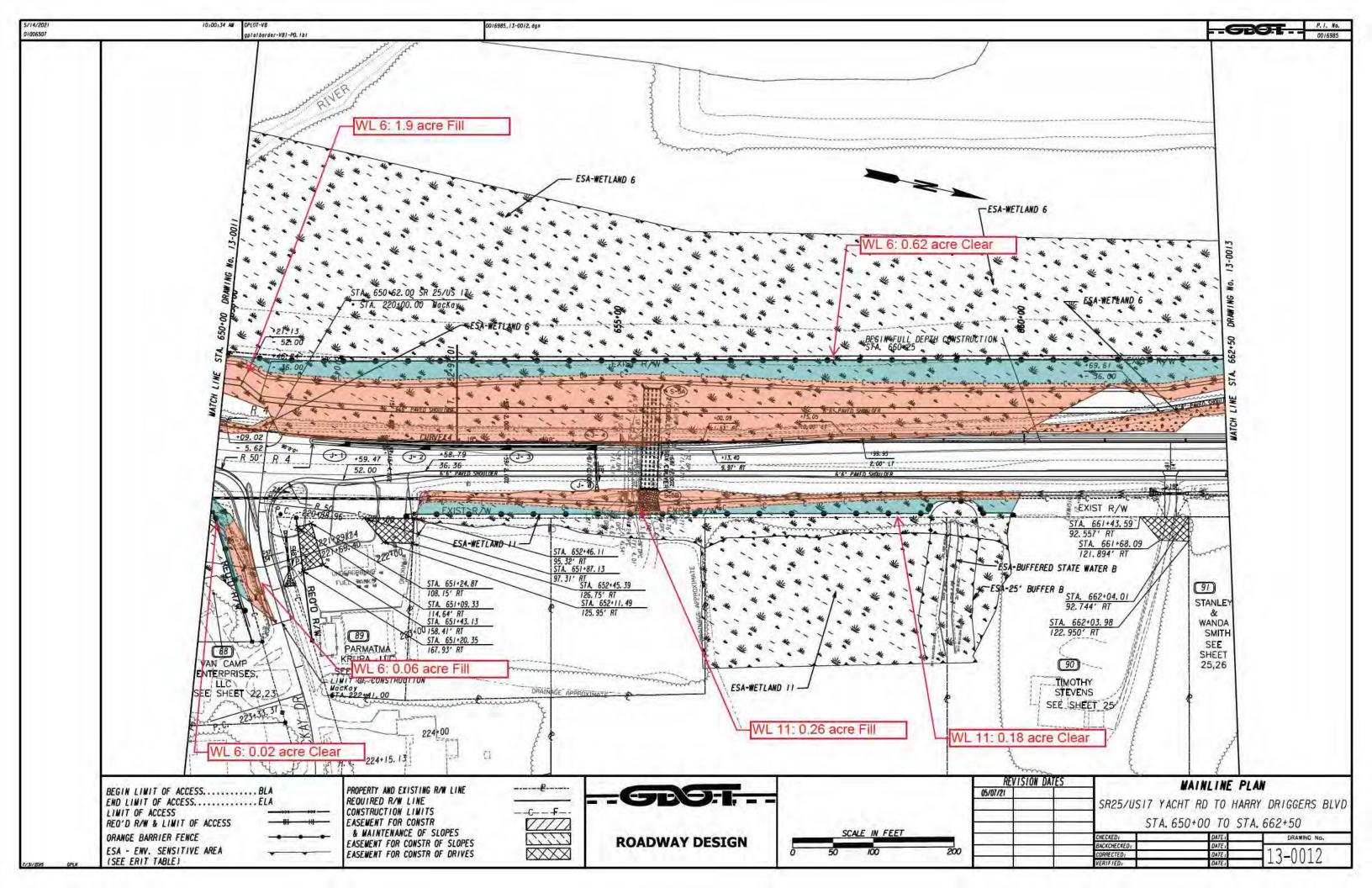


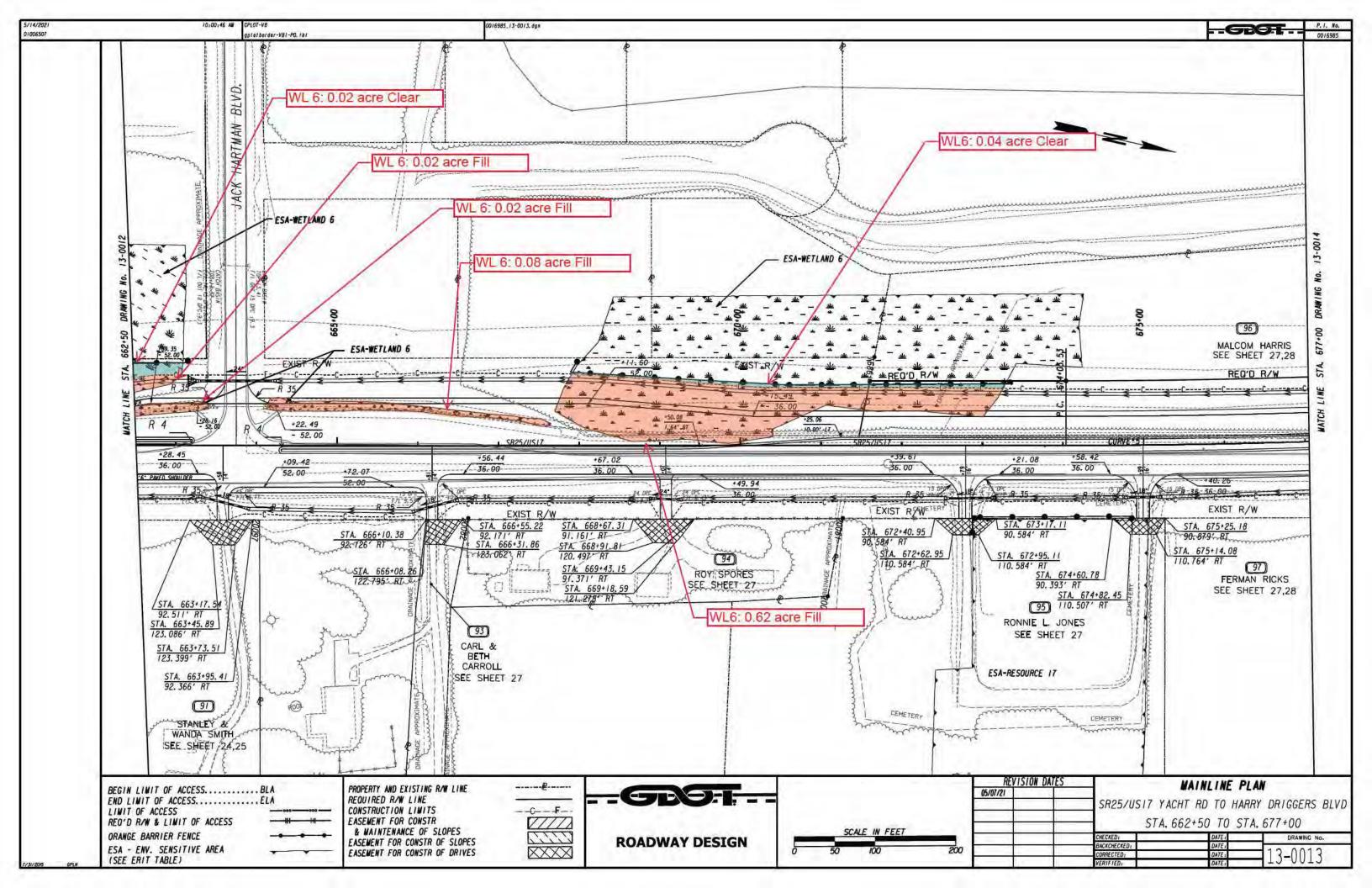


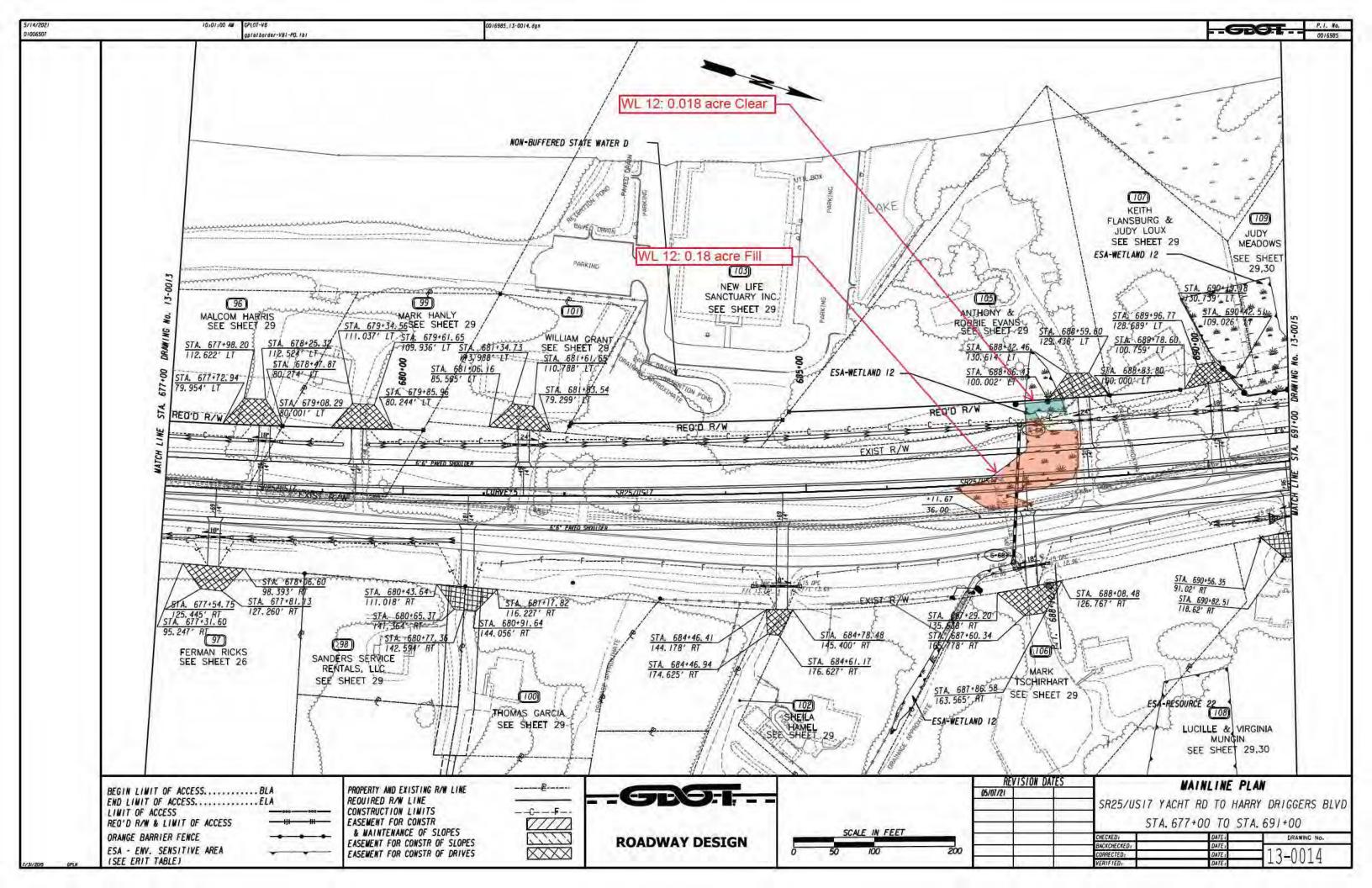


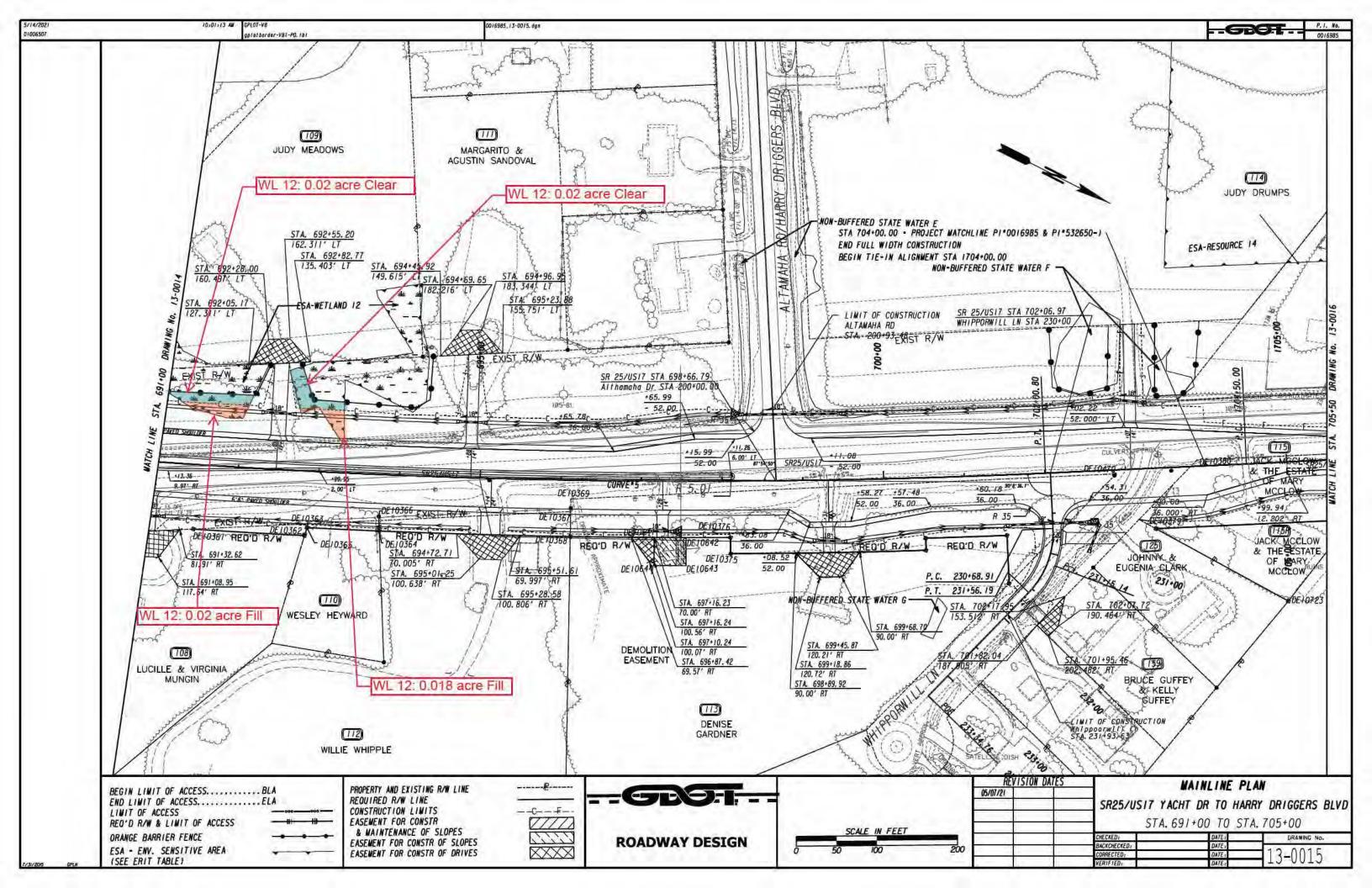


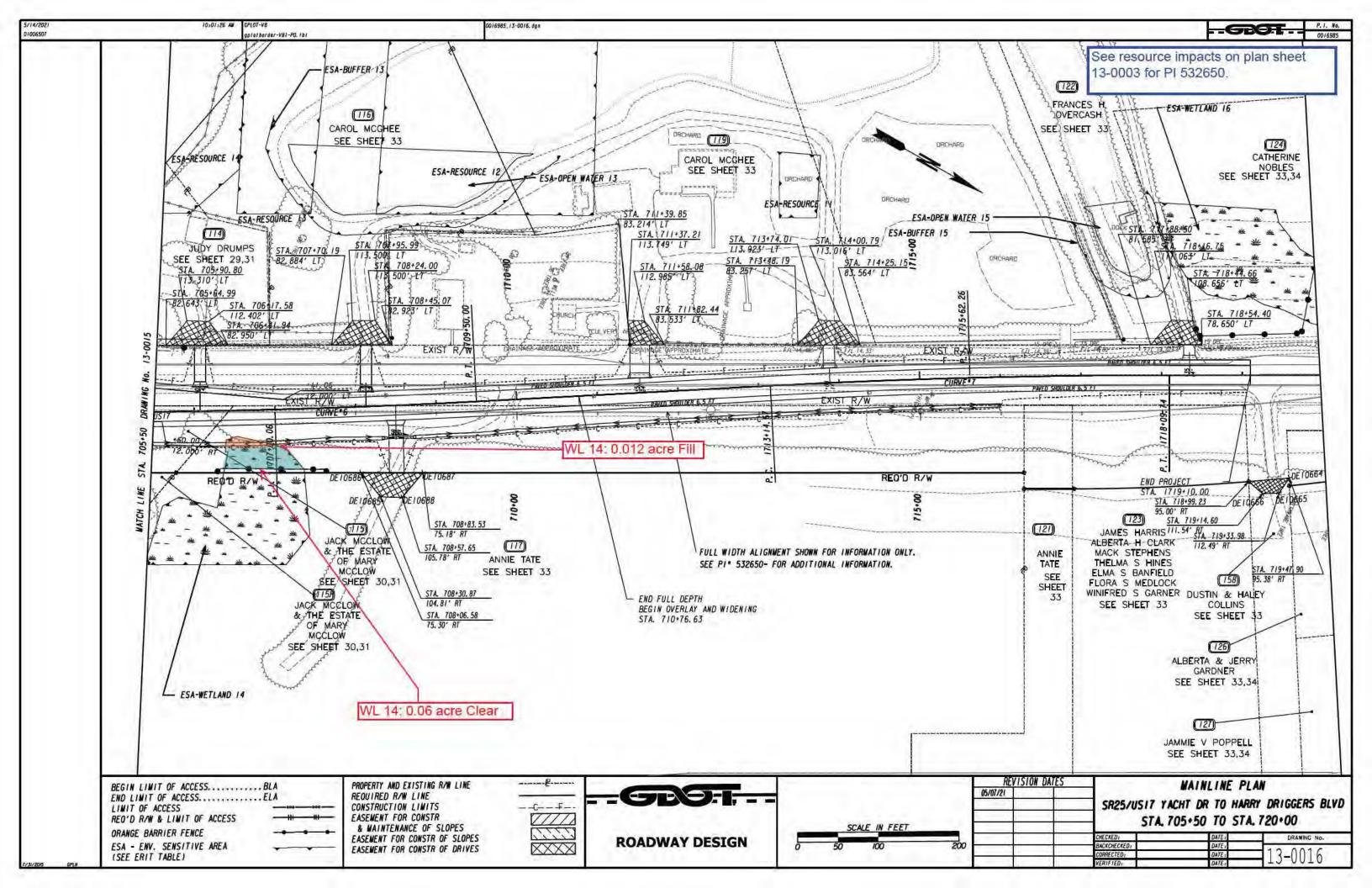


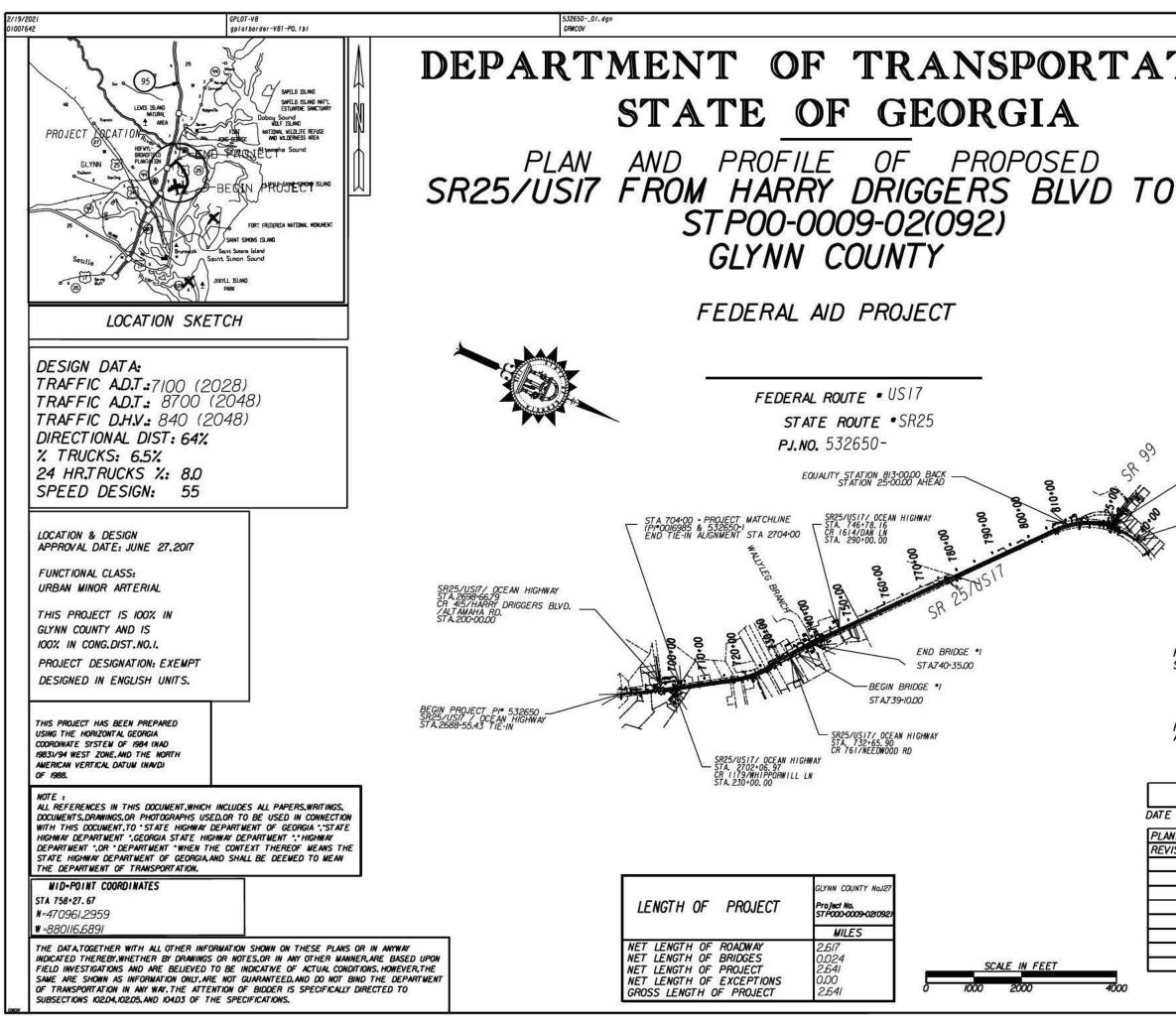




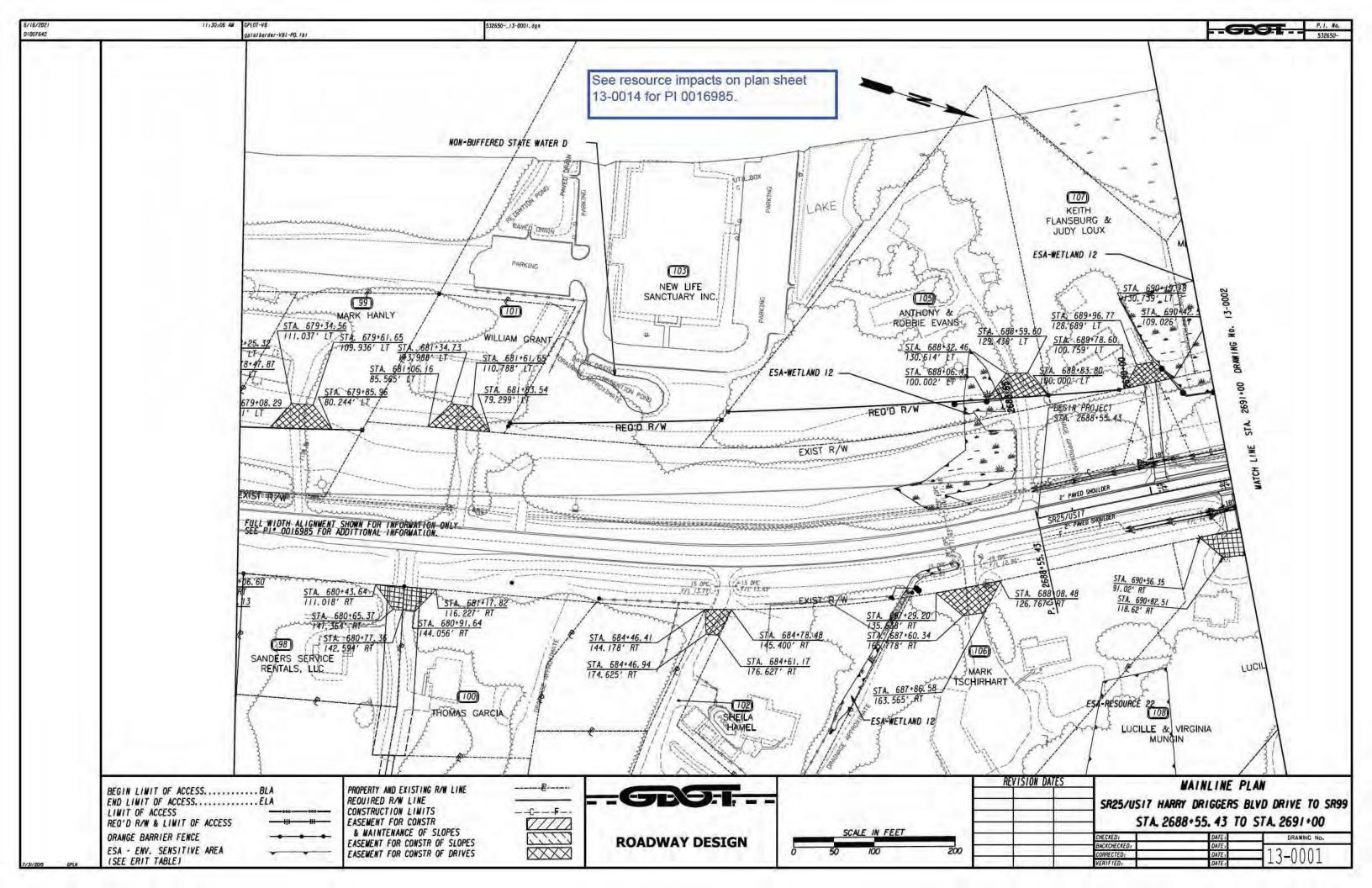


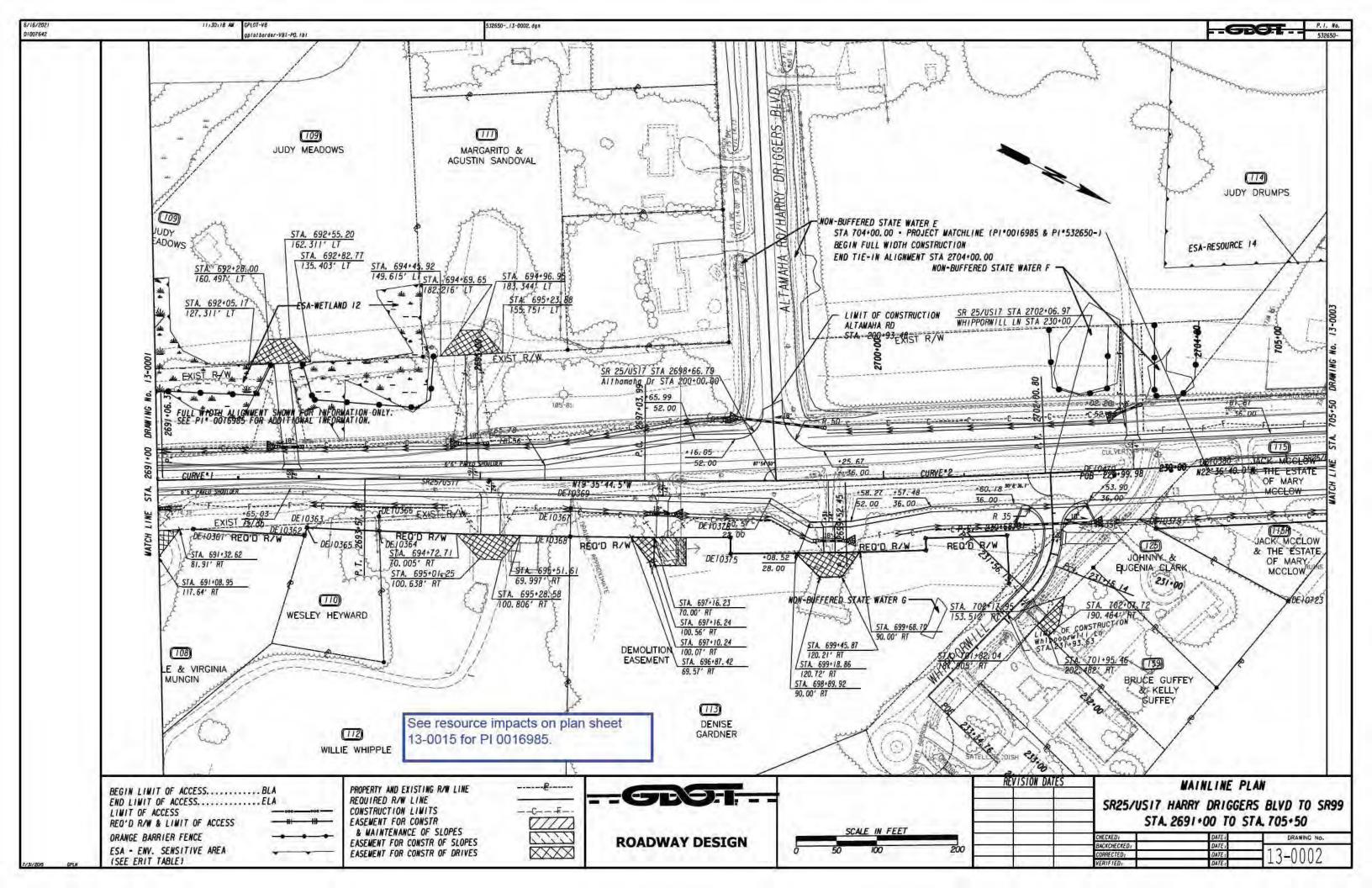


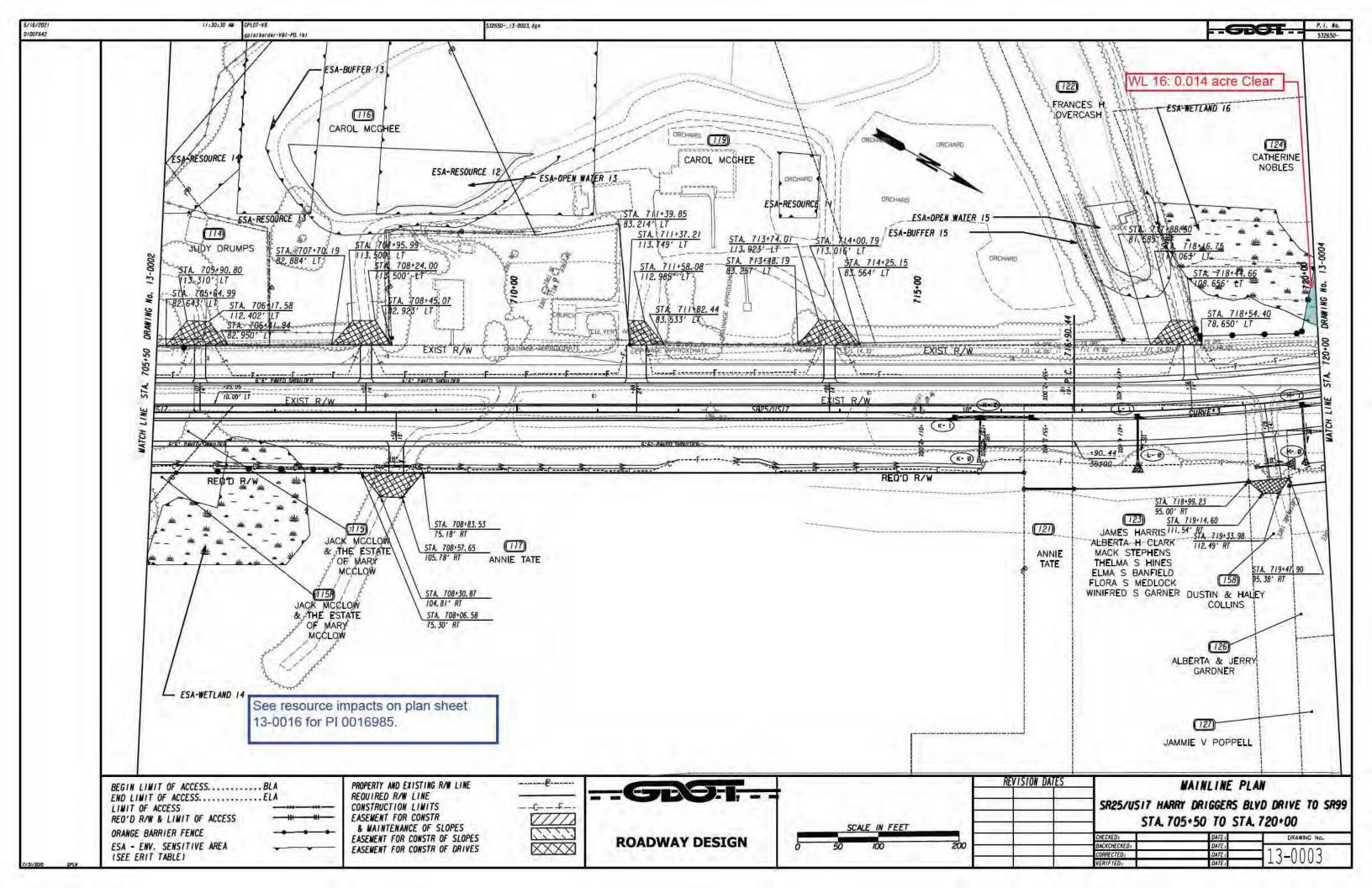


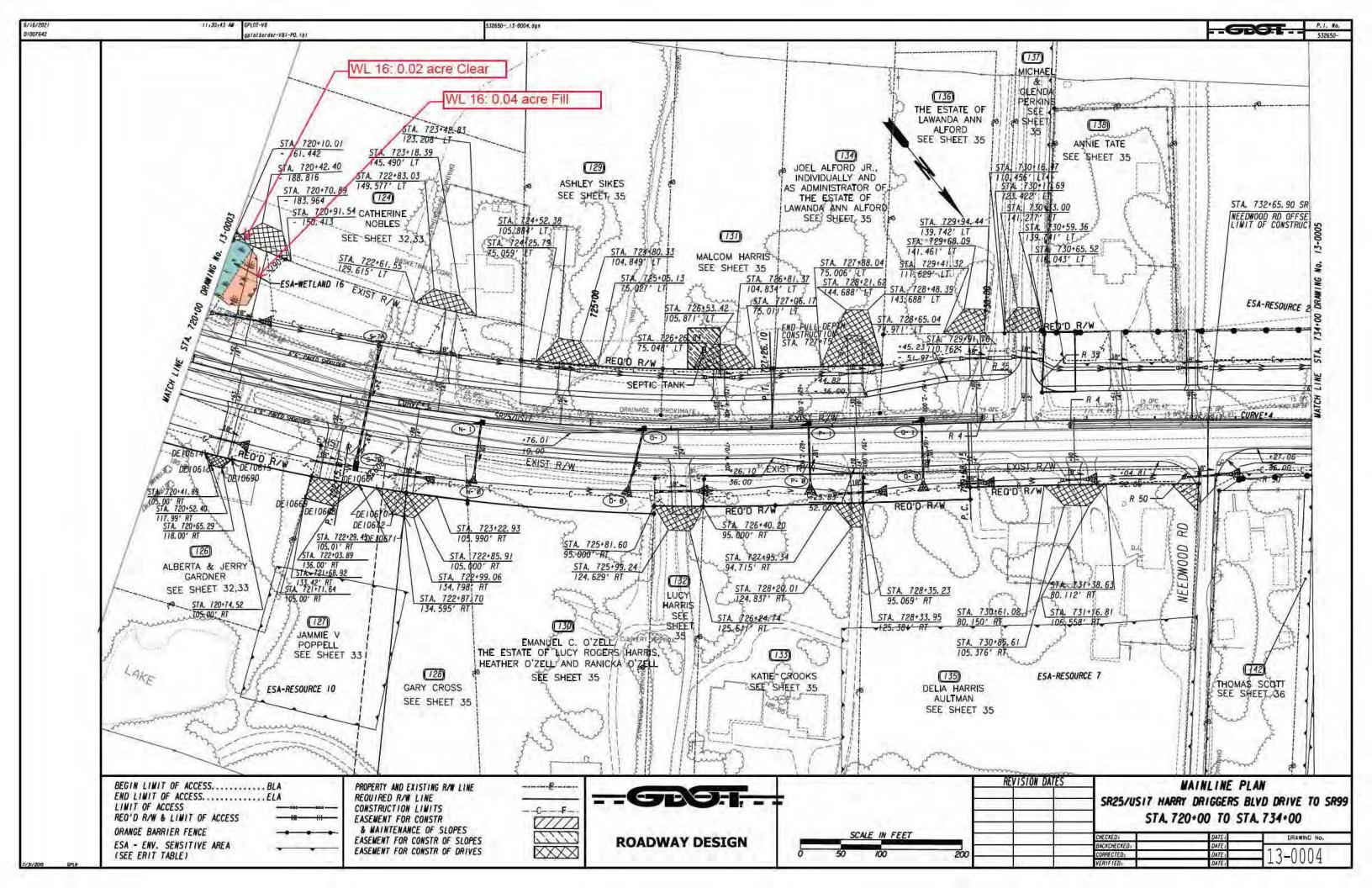


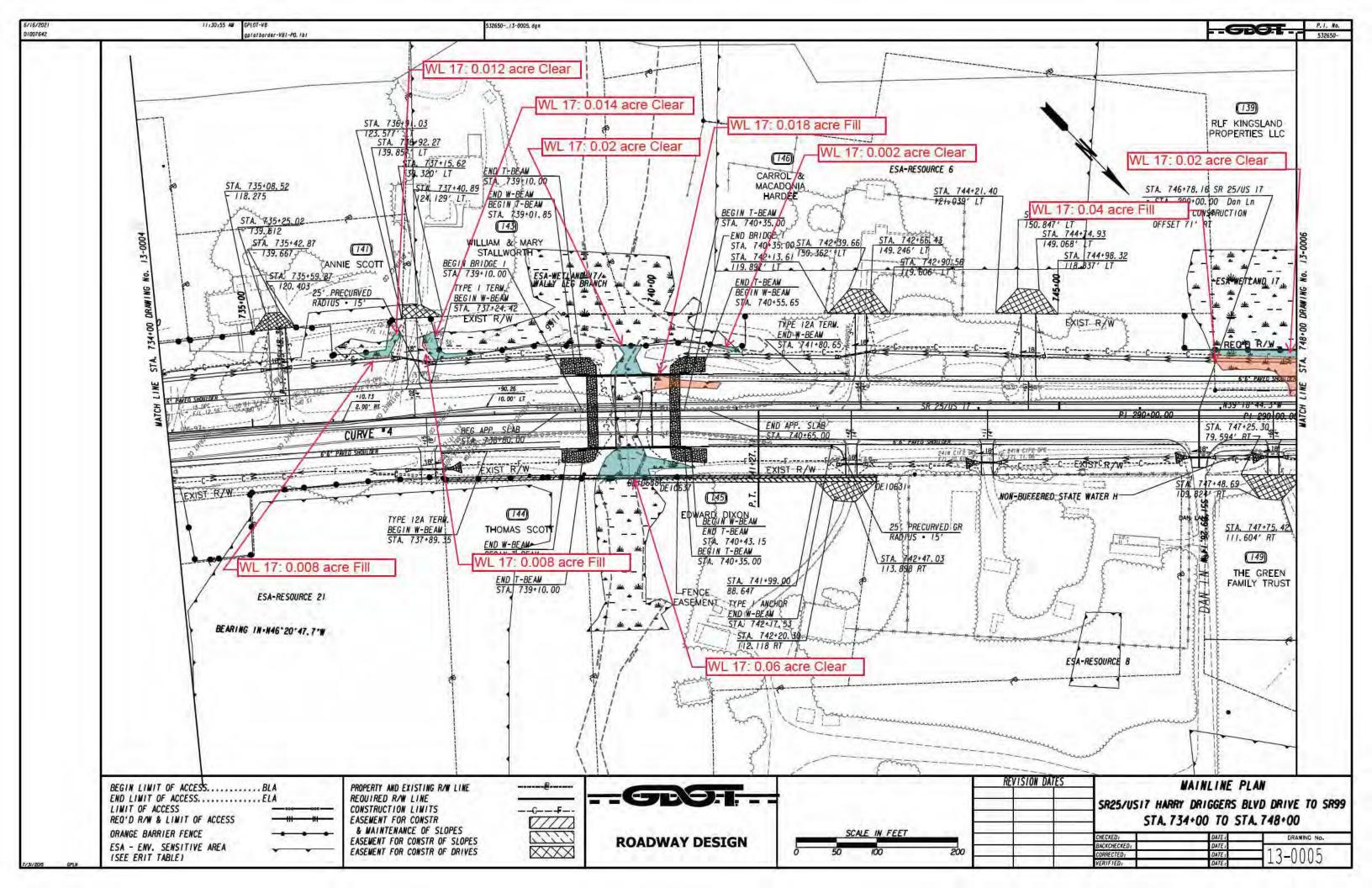
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PLANS MARISHA PENA, E.I.T.
PLANS MARISHA PENA, E.I.I. PREPARED BY: DESIGN ENGINEER
RECOMMENDED FOR SUBMISSION BY: CAROL BOWLER, P.E. DESIGN ENGINEER GROUP MANAGER
DESIGN ENGINEER GROOF MANAGER
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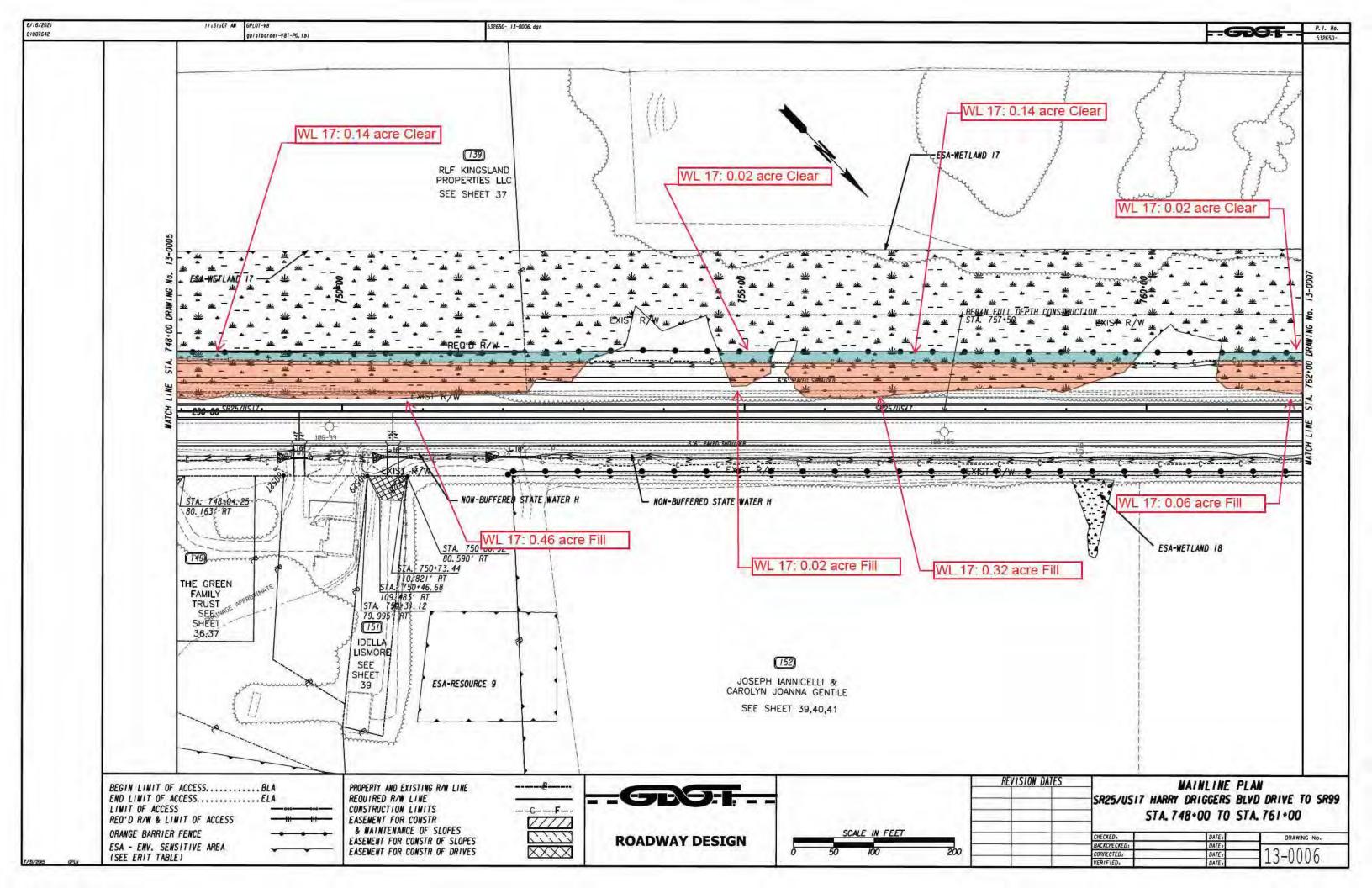


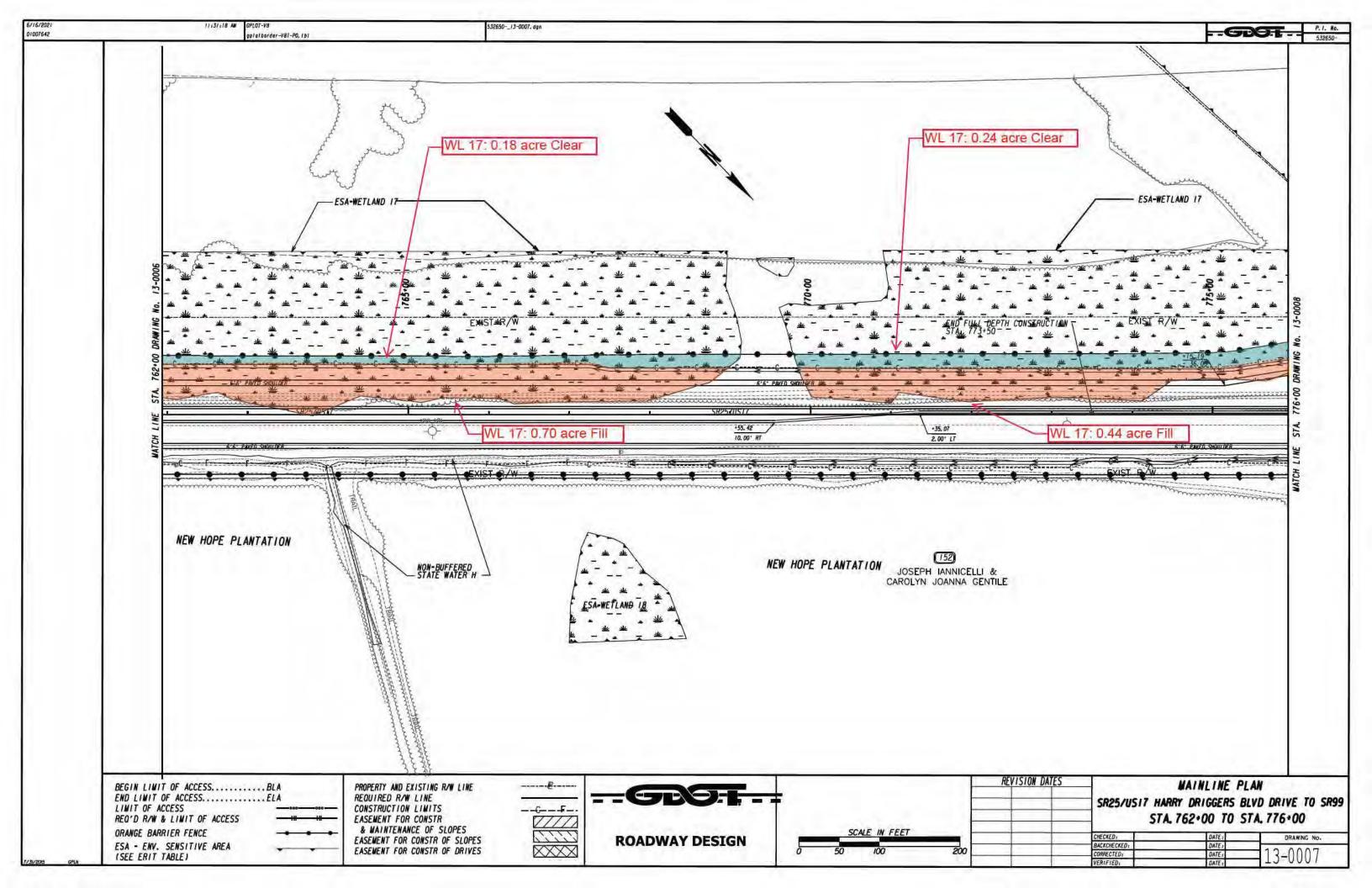


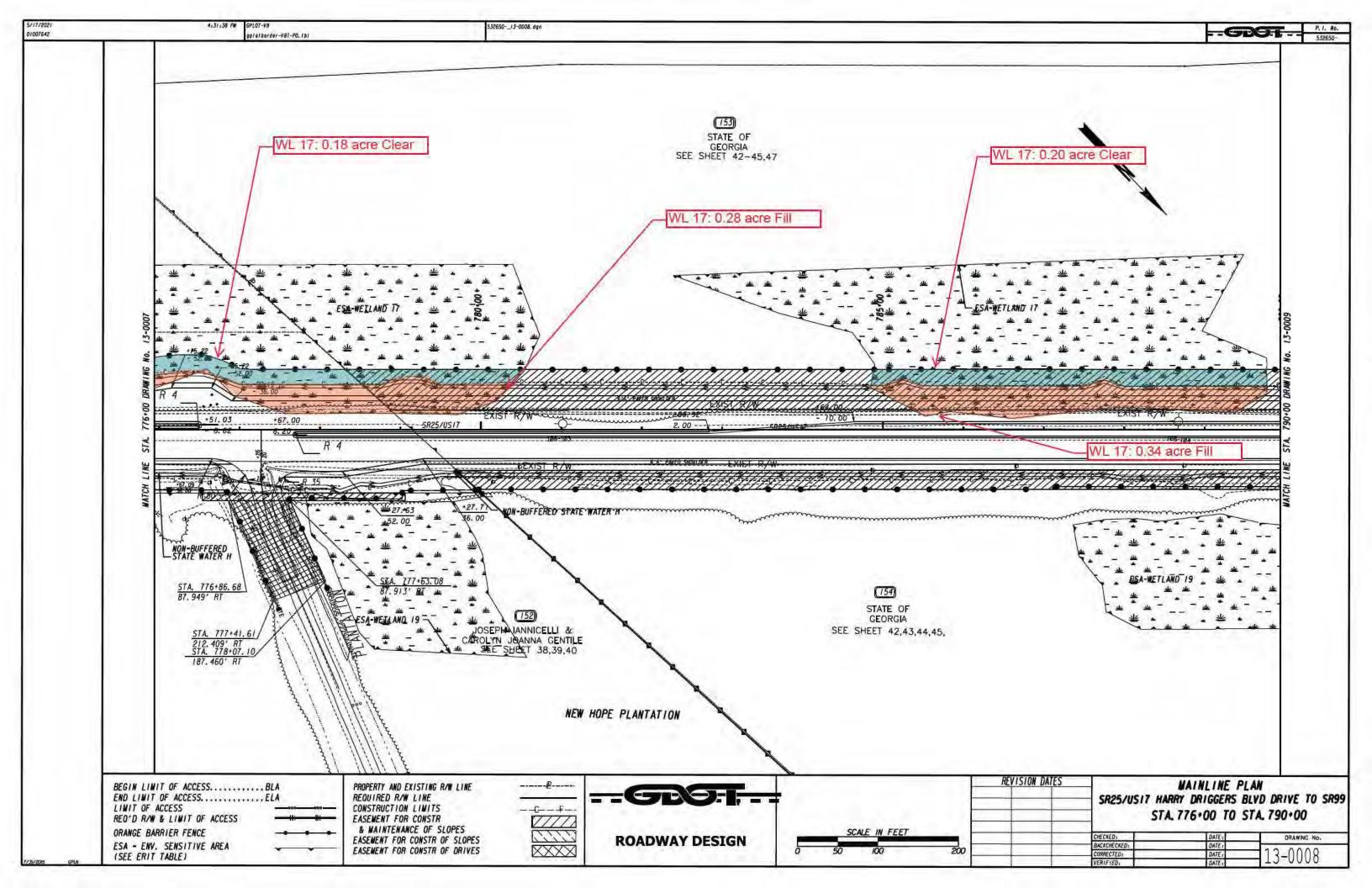


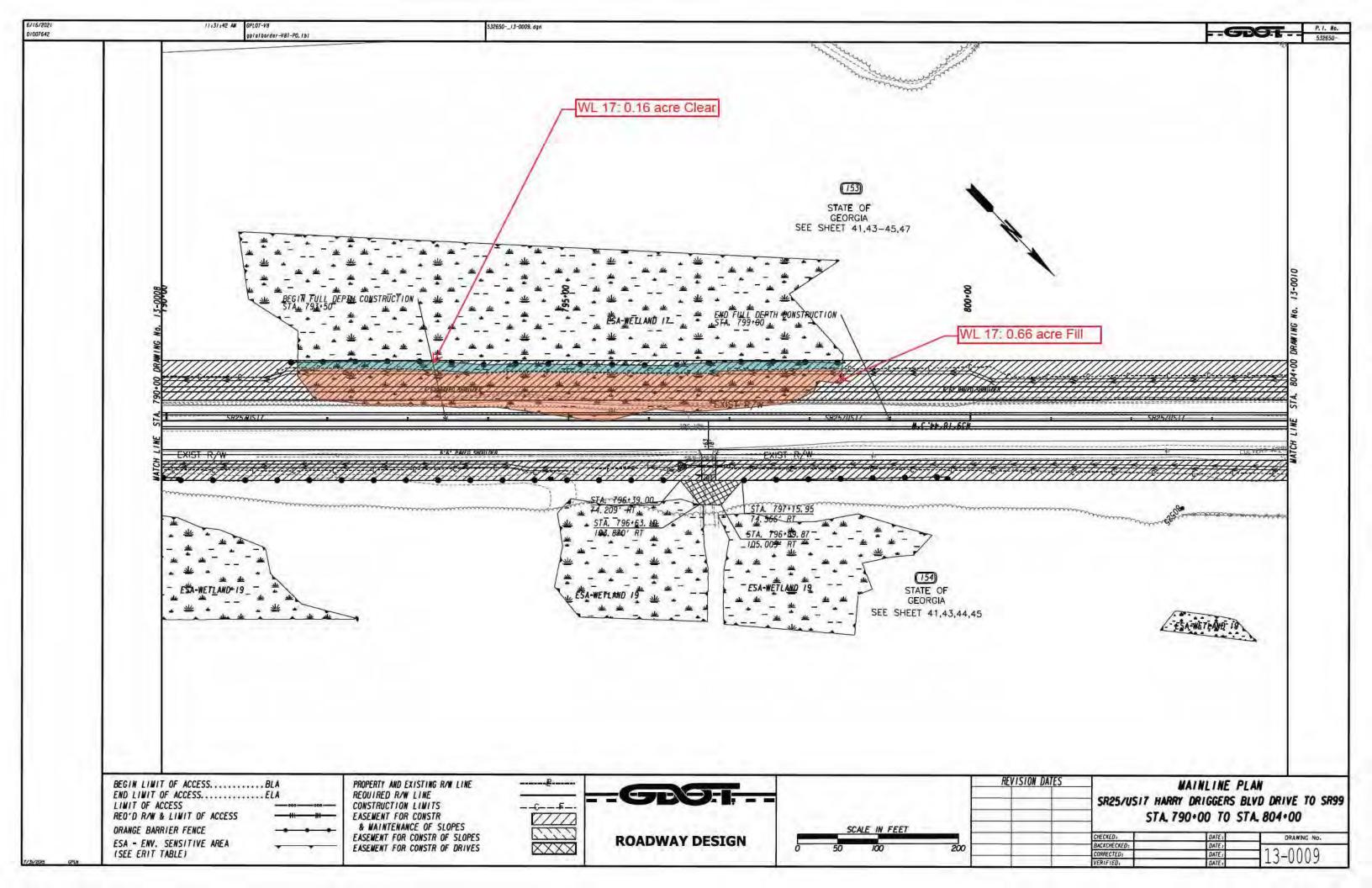


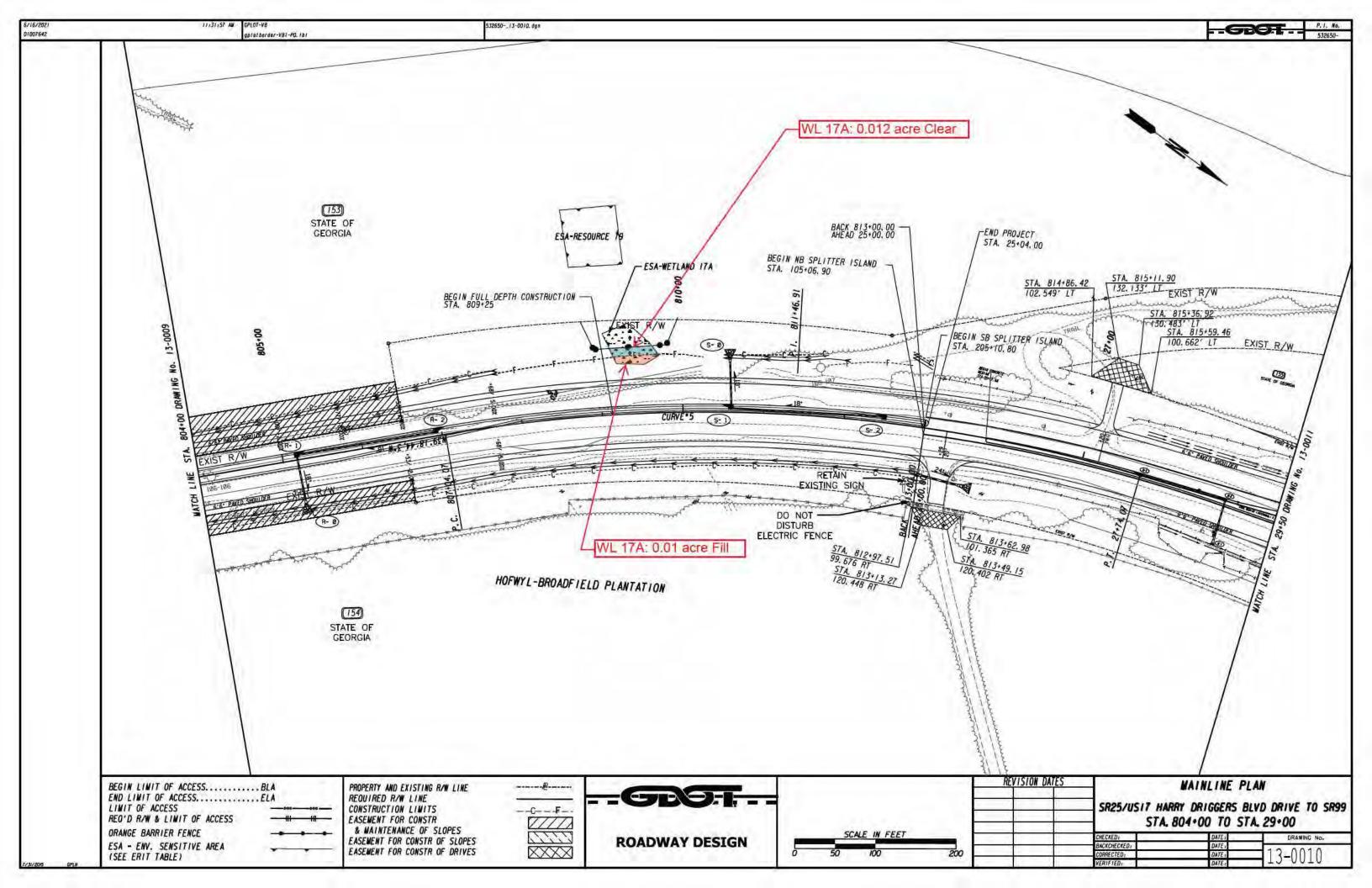


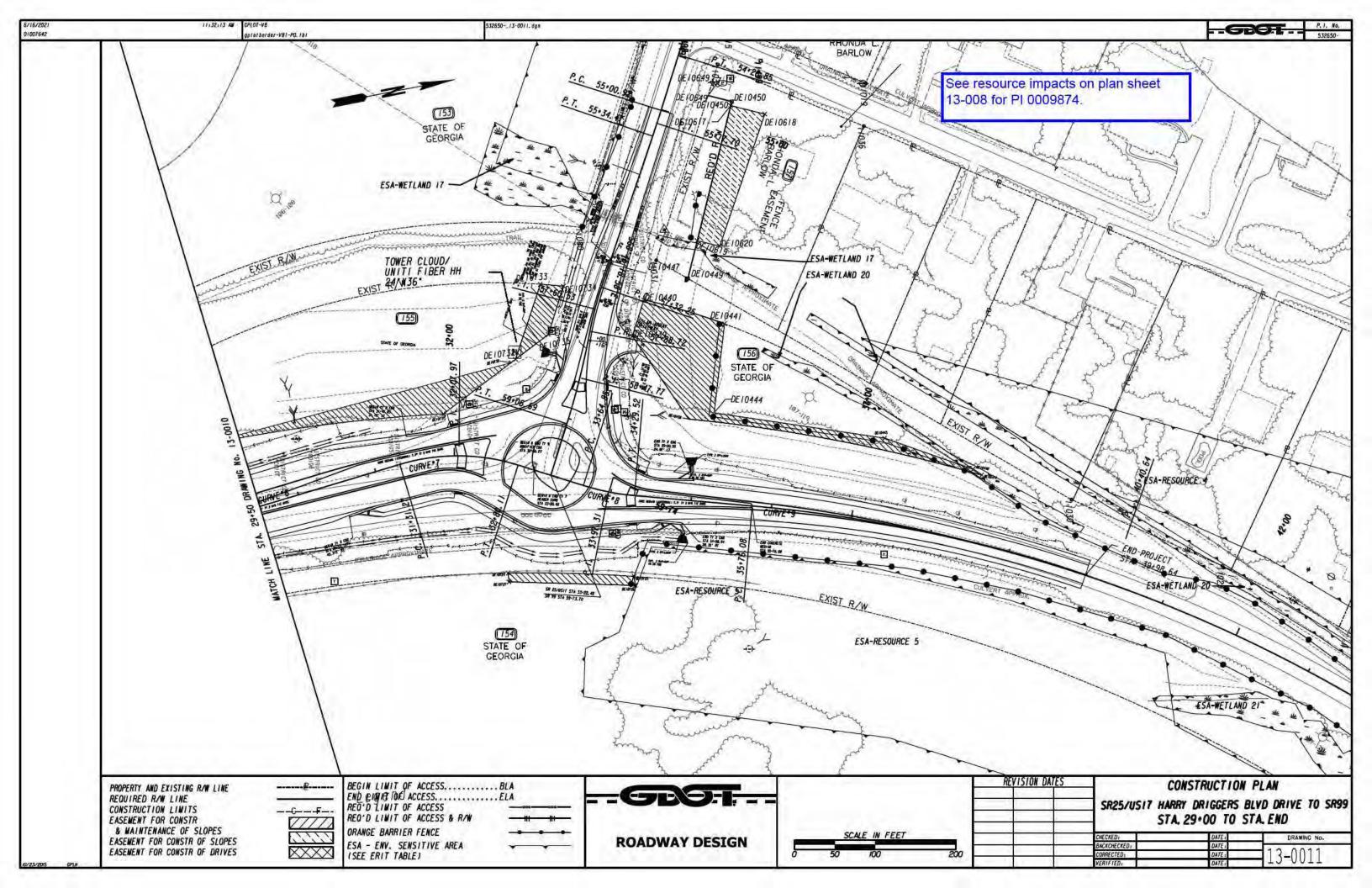


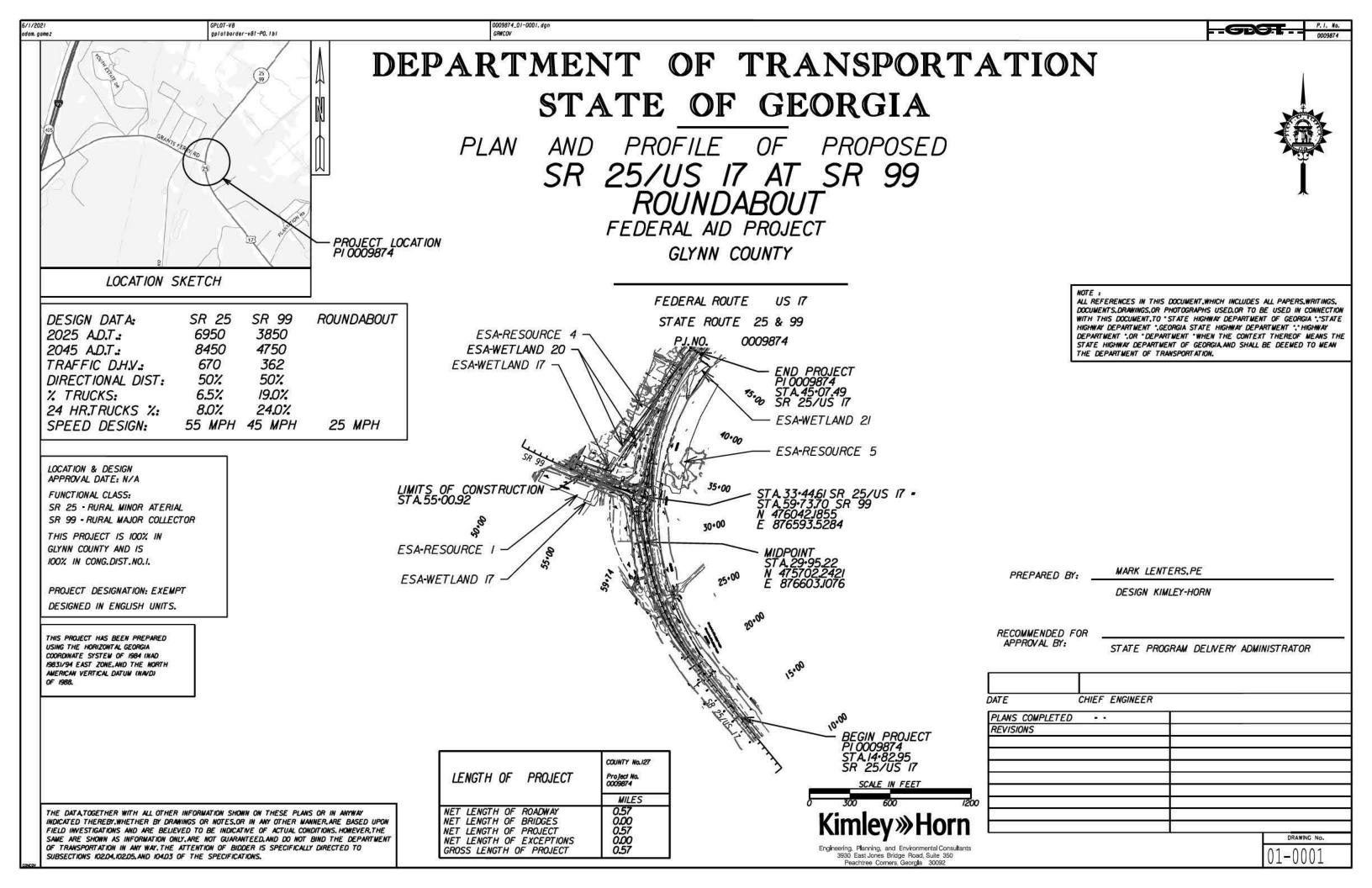


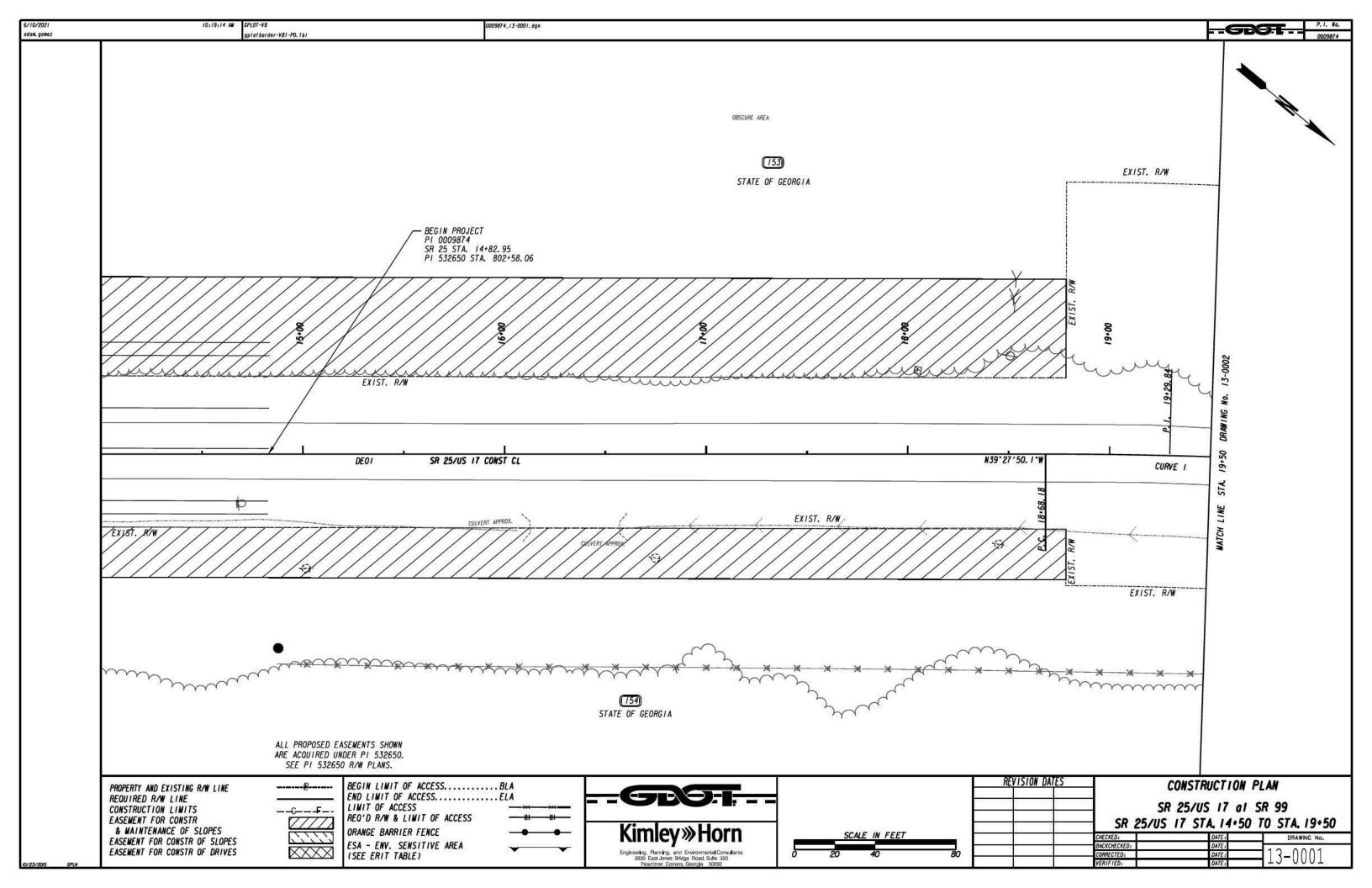


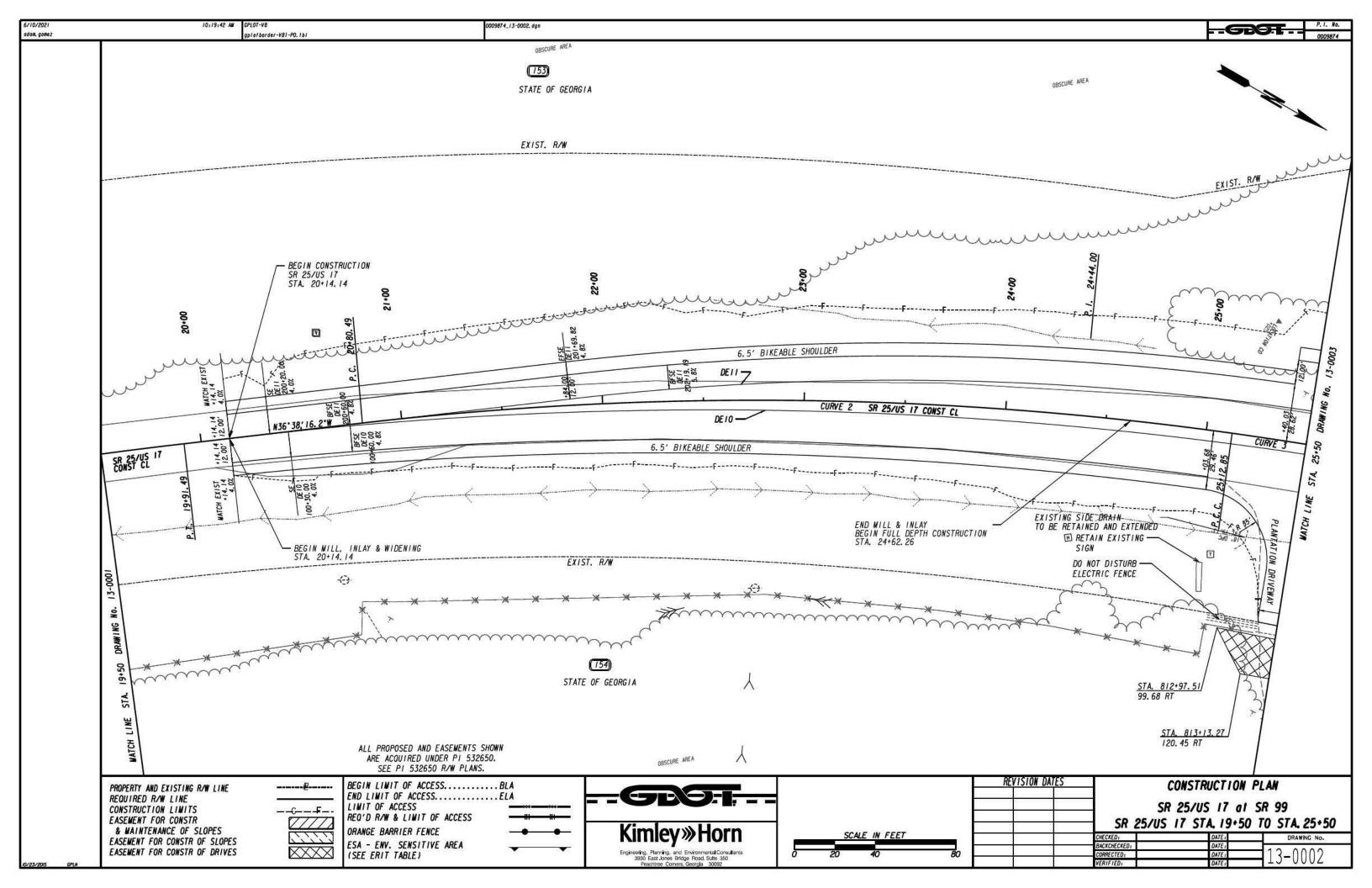


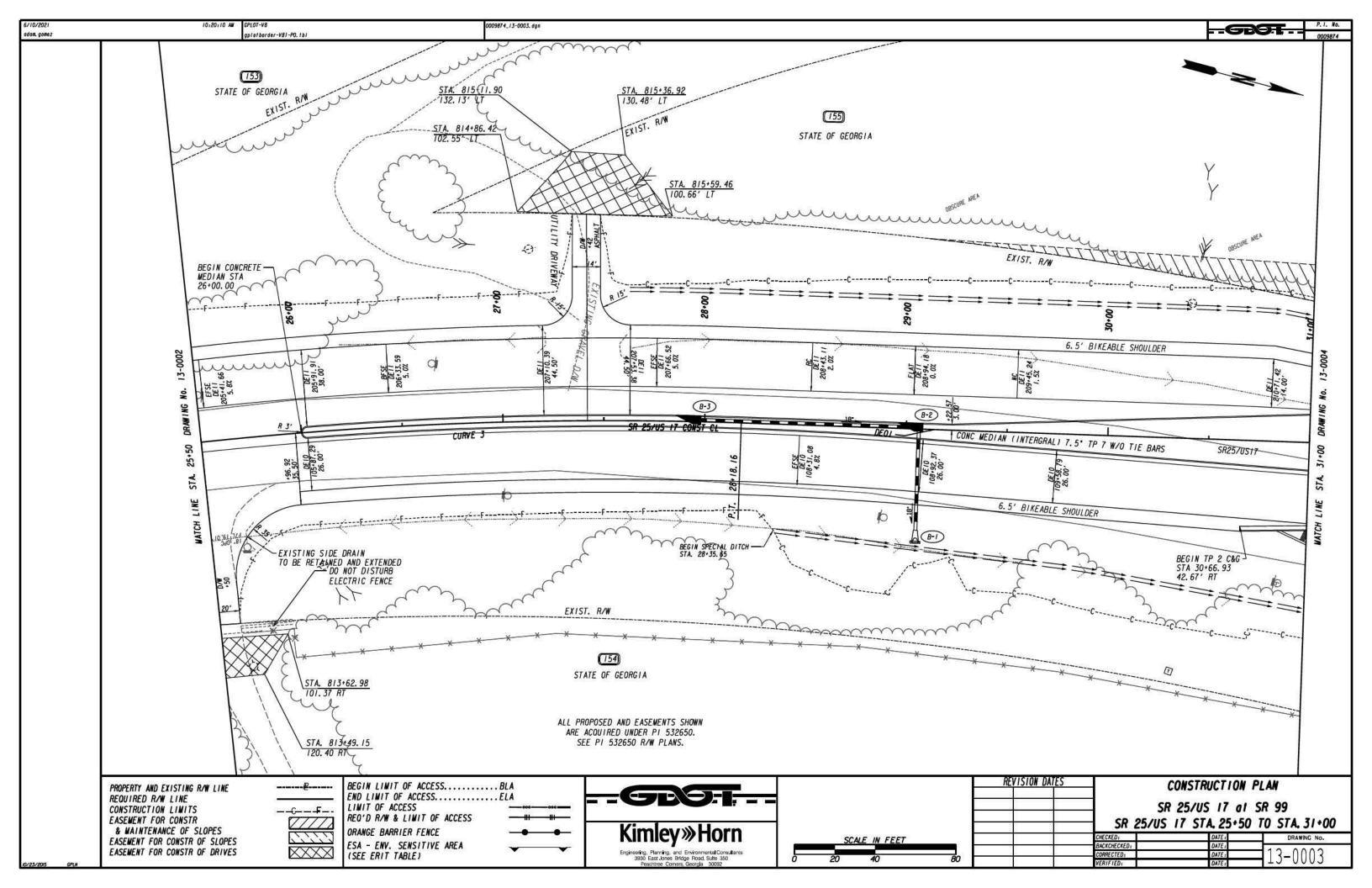


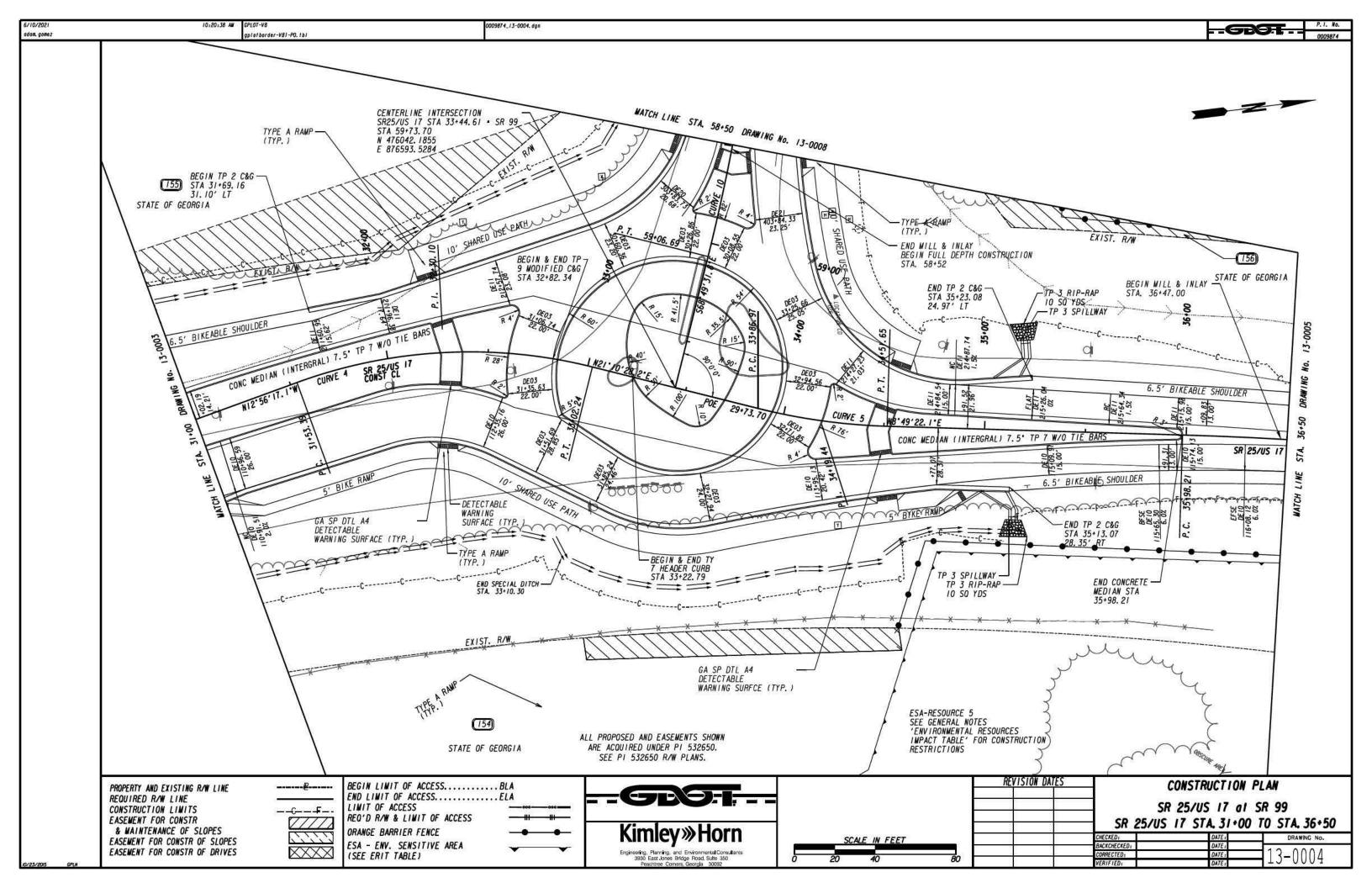


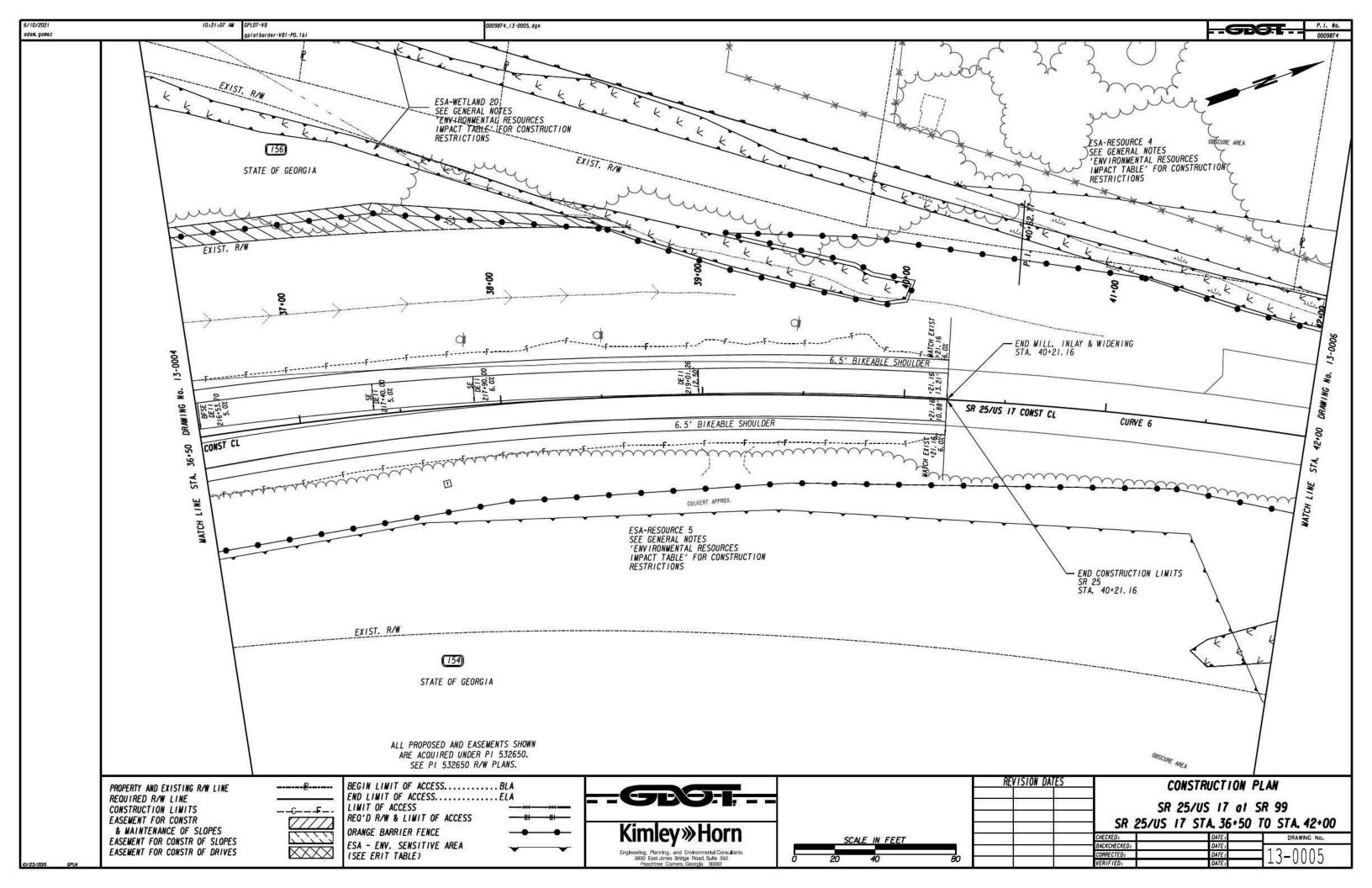


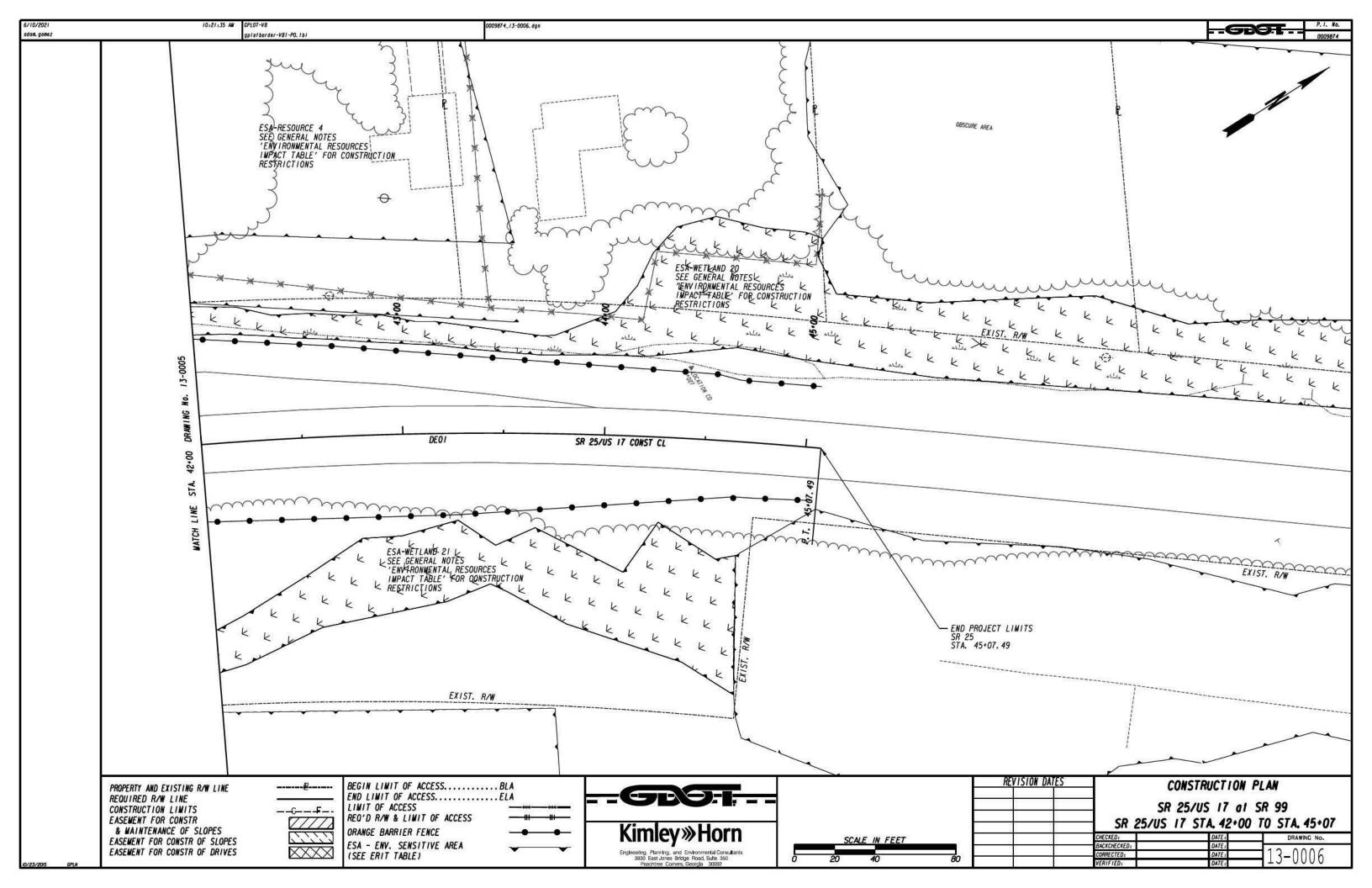


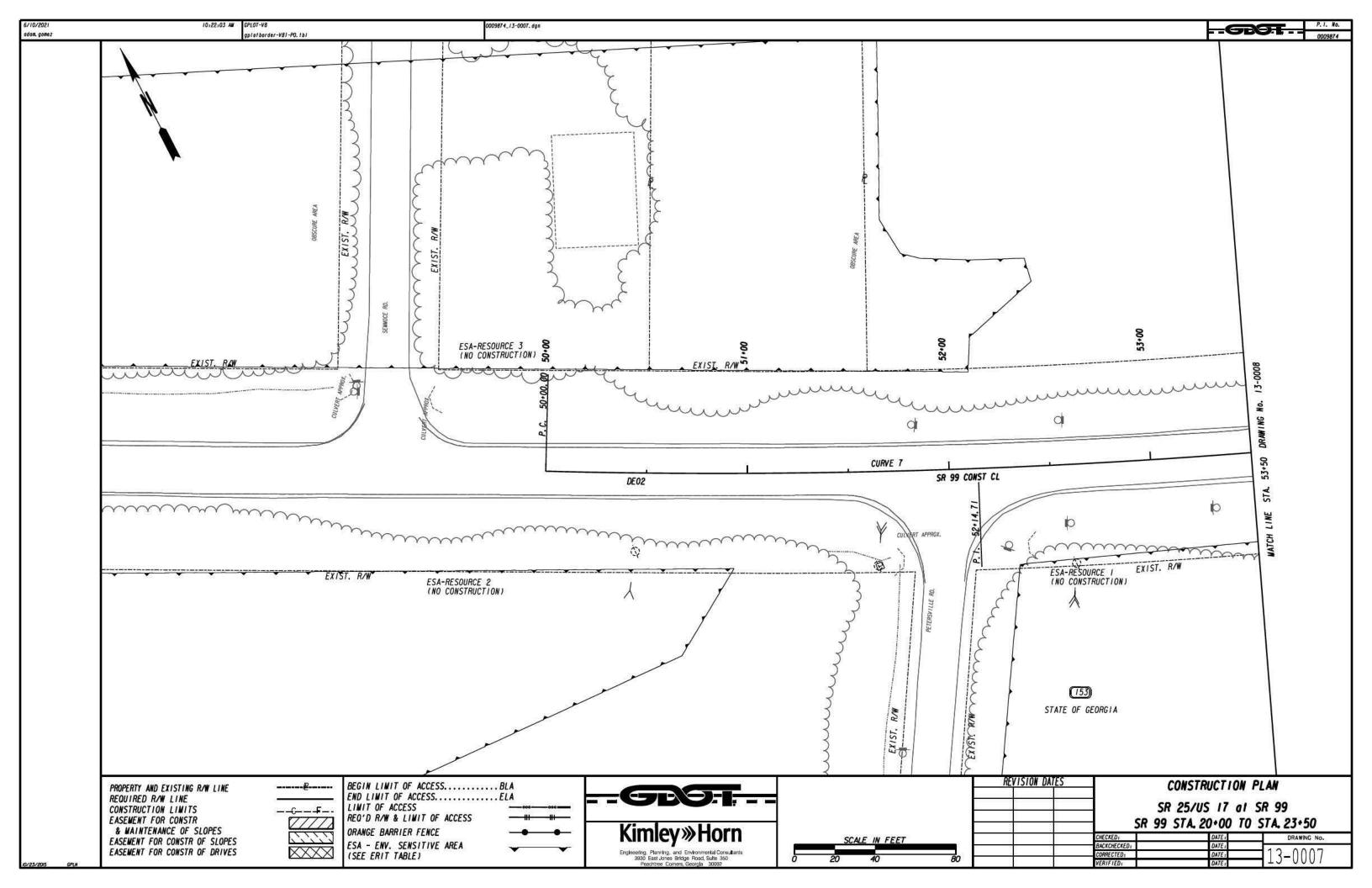


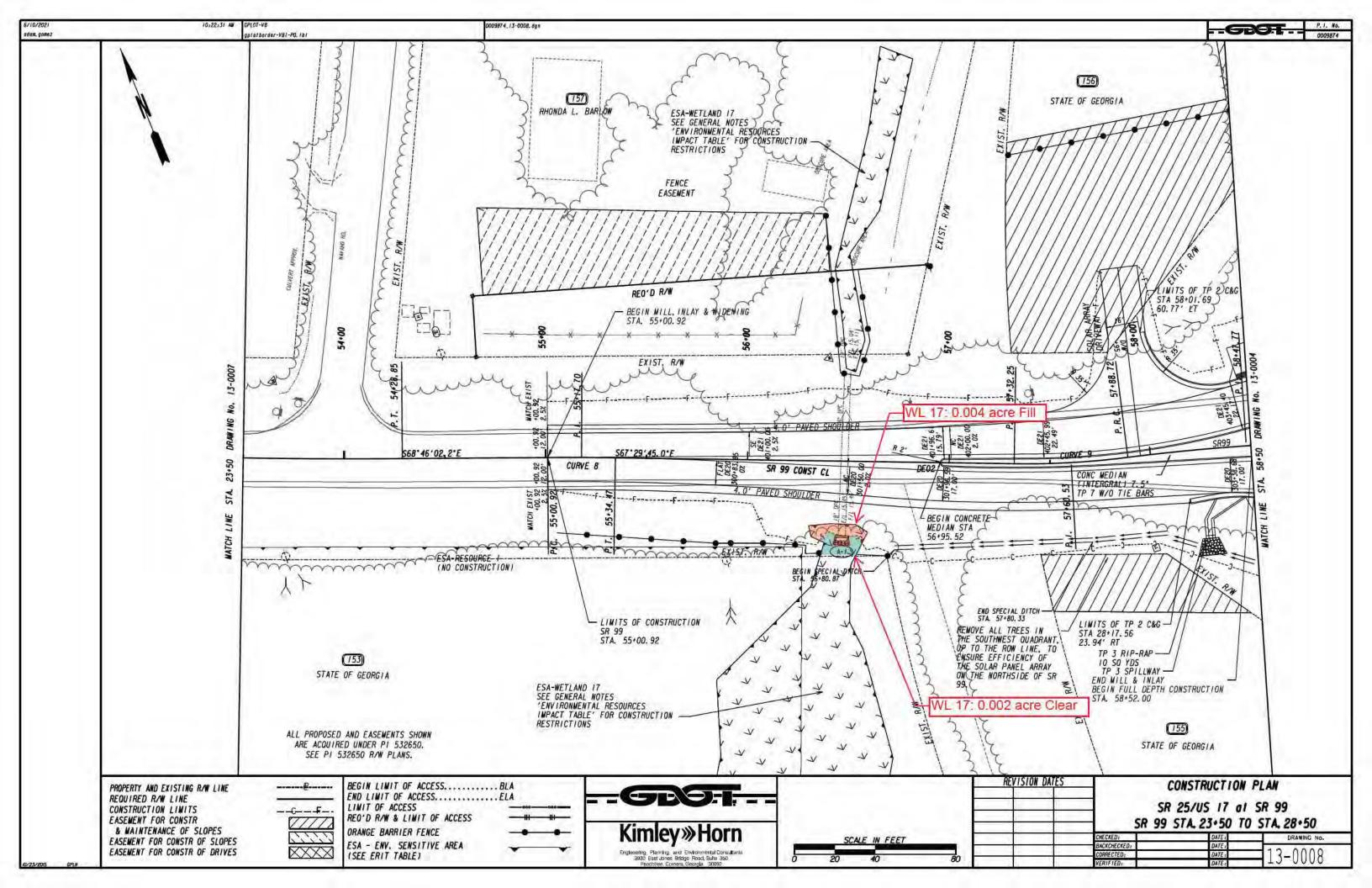


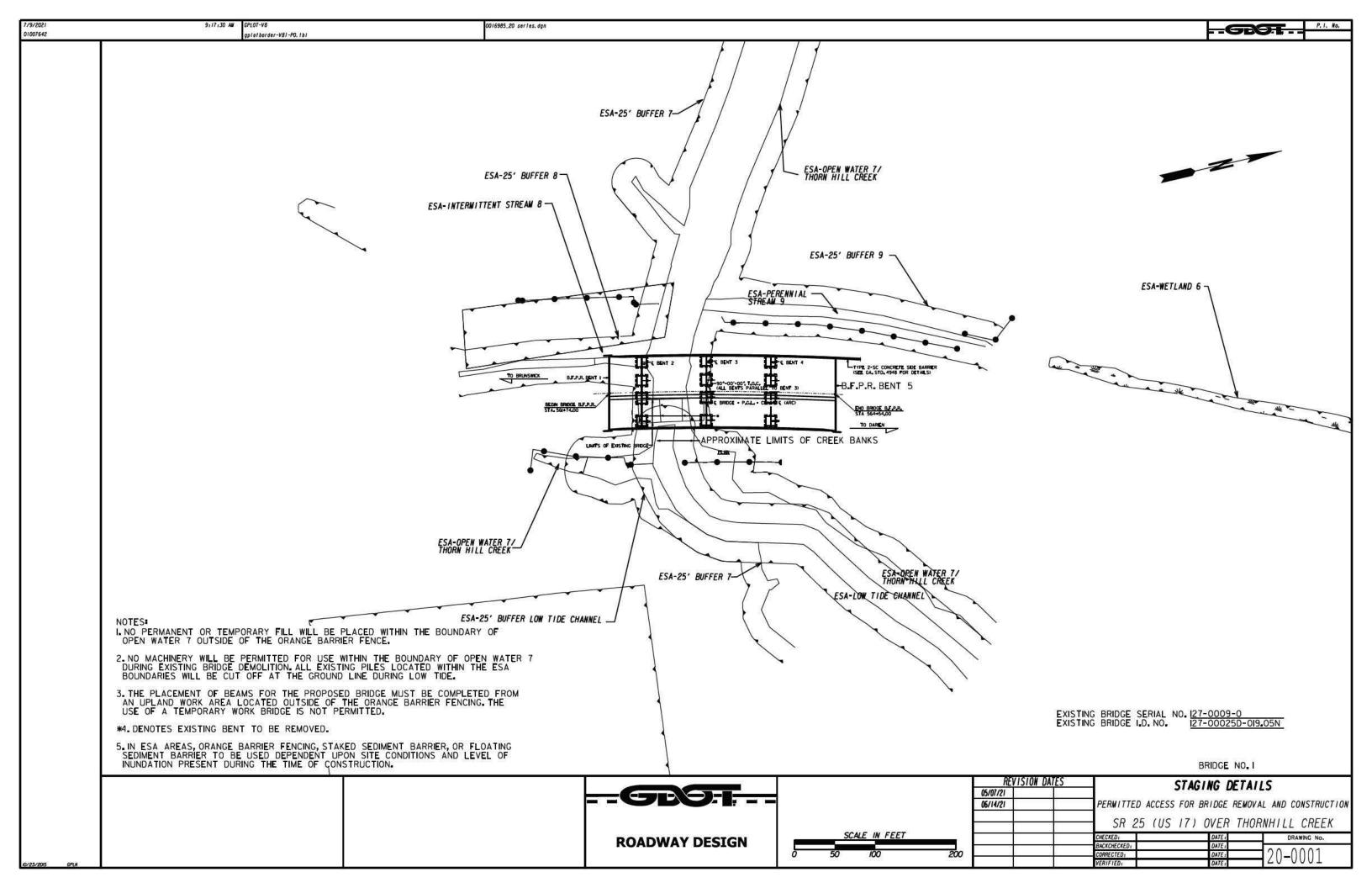












Anticipated Temporary Access Construction Method Analysis

P.I No. 0016985, Glynn County SR 25 (US 17) @ THORNHILL CREEK

08/31/2020 Anticipated Construction Method for Temporary Access – Based on Construction Plans Dated

No In-water Access Required (Activity able to take place from streambank)

Barge (Minimum water depth ≥ 7 feet; Predictable water level @ Project site)

Work Bridge (\geq 10 feet of stable substrate above bedrock for pile driving)

Cofferdam or Sediment Containment Unit (\geq 10 feet of stable substrate above bedrock for sheet pile driving)

Bulkhead (Uneven terrain requires flattening of streambank from which to operate equipment)

Jetty (< 10 feet of stable substrate above bedrock for pile driving)

Other

Stream & Construction Method Details

- 41 Approximate Stream Width at Project Site – Linear Feet (LF)
- 41 Open Stream Width @ Maximum Estimated Restriction (LF) - Section 20 Plans denote restriction limits

N/A Estimated Duration @ Max Stream Restriction (Months)

N/A Total Estimated Stream Restriction Duration (Months)

N/A Return to Regular Flow from Max Stream Restriction (LF downstream)

N/A Return to Regular Flow from Max Stream Restriction (LF upstream)

Stream Geomorphology Analysis

Bank: Banks are low laying and are marshy.

Substrate: Channel bottom composed of mostly sand.

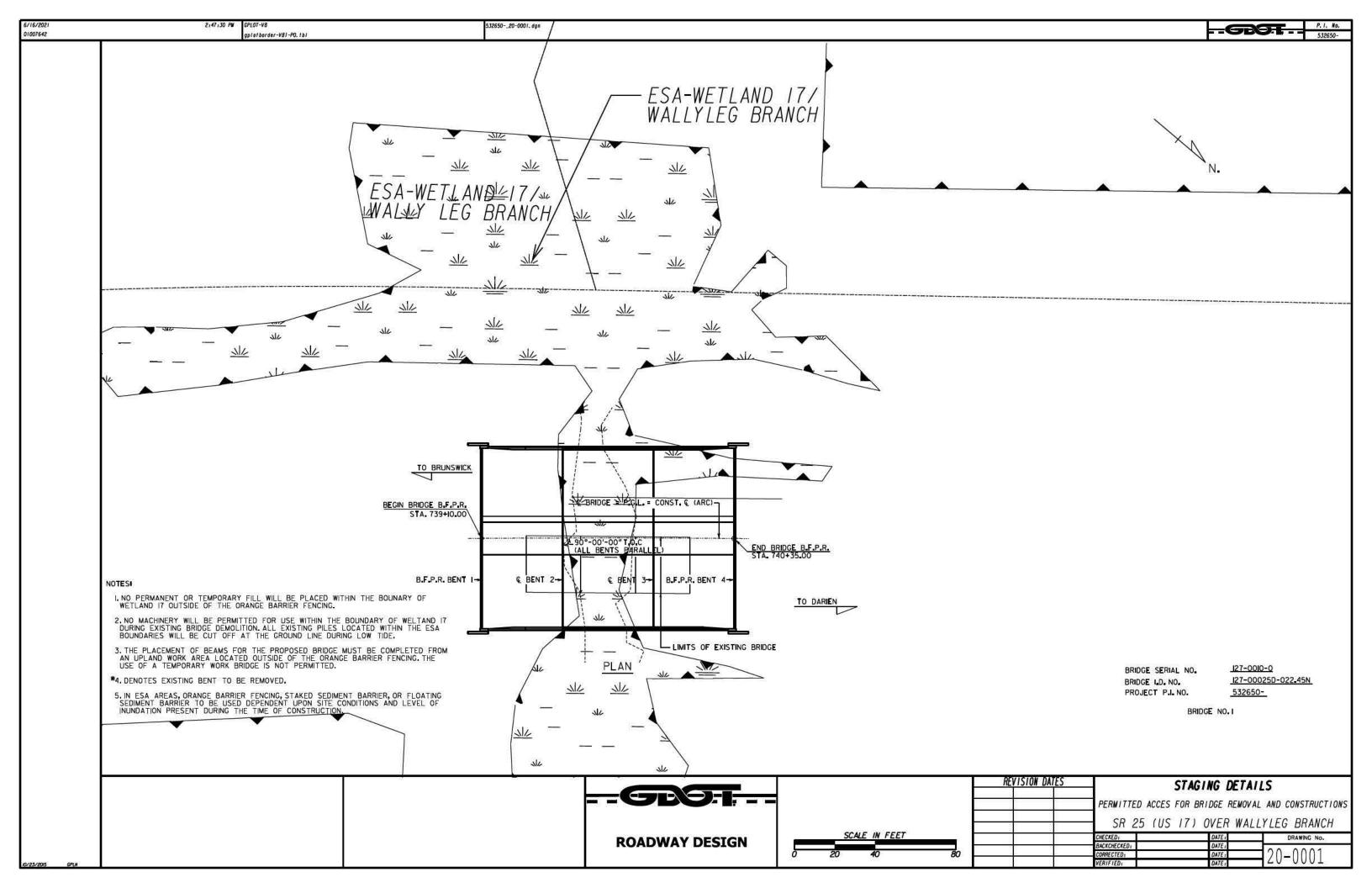
	Base Flo	w	Flow Immediate Overtopping Re	1.10			
Flow within Channel Limits	N/A	cfs	N/A	cfs	N/A	cfs	
Channel Velocity through Bridge (No Restriction)	N/A	fps	N/A	fps	N/A	fps	
Channel Velocity through Bridge (With Restriction)	N/A	fps	N/A	fps	N/A	fps	
% Increase in Channel Velocity	N/A	%	N/A	%	N/A	%	
Contraction Scour in Channel (No Restriction)	N/A	ft	N/A	ft	N/A	ft	
Contraction Scour in Channel (With Restriction)	N/A	ft	N/A	ft	N/A	ft	

Flow Analysis – NOTE: Only required when restriction of the stream will occur (e.g., bulkhead, jetty, etc.)

Key: ft = feet; cfs = cubic feet per second; fps = feet per second

Conclusion

No machinery will be permitted for use within the boundary of the wetland during demolition. All existing piles located within the ESA boundaries will be cut off at the ground line during low tide. The proposed beam placement will be complete from an upstream area located outside of the orange barrier fencing. No work bridge permitted. No dredging anticipated. No wetland crossing anticipated during construction. Proposed concrete intermediate bent 2, 3 and 4 will required cofferdams. Cofferdams are outside the limits of the stream bank and water.



Anticipated Temporary Access Construction Method Analysis

P.I No. 532650 ,Glynn County SR 25 (US 17) over WALLYLEG BRANCH

08/31/2020 Anticipated Construction Method for Temporary Access – Based on Construction Plans Dated

No In-water Access Required (Activity able to take place from streambank)

Barge (Minimum water depth ≥ 7 feet; Predictable water level @ Project site)

Work Bridge (\geq 10 feet of stable substrate above bedrock for pile driving)

Cofferdam or Sediment Containment Unit (\geq 10 feet of stable substrate above bedrock for sheet pile driving)

Bulkhead (Uneven terrain requires flattening of streambank from which to operate equipment)

Jetty (< 10 feet of stable substrate above bedrock for pile driving)

Other

1

Stream & Construction Method Details

- 35 Approximate Stream Width at Project Site – Linear Feet (LF)
- 35 Open Stream Width @ Maximum Estimated Restriction (LF) - Section 20 Plans denote restriction limits

N/A Estimated Duration @ Max Stream Restriction (Months)

N/A Total Estimated Stream Restriction Duration (Months)

N/A Return to Regular Flow from Max Stream Restriction (LF downstream)

N/A Return to Regular Flow from Max Stream Restriction (LF upstream)

Stream Geomorphology Analysis

Bank: Banks are low laying and are marshy.

Substrate: Channel bottom composed of mostly sand.

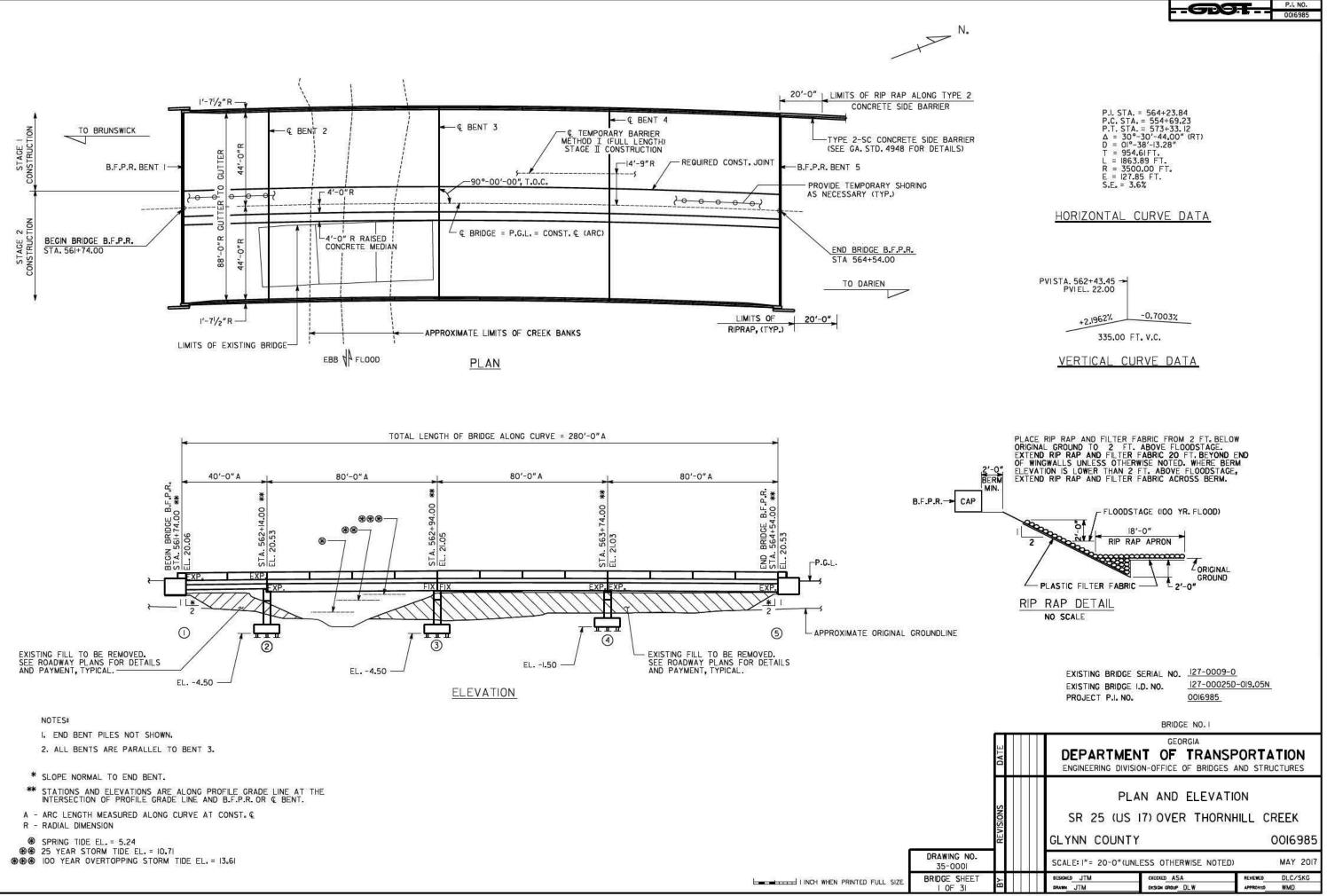
	Base Flo	w	Flow Immediate Overtopping Re		Flow @ 2 Year Stor Event	
Flow within Channel Limits	N/A	cfs	N/A	cfs	N/A	cfs
Channel Velocity through Bridge (No Restriction)	N/A	fps	N/A	fps	N/A	fps
Channel Velocity through Bridge (With Restriction)	N/A	fps	N/A	fps	N/A	fps
% Increase in Channel Velocity	N/A	%	N/A	%	N/A	%
Contraction Scour in Channel (No Restriction)	N/A	ft	N/A	ft	N/A	ft
Contraction Scour in Channel (With Restriction)	N/A	ft	N/A	ft	N/A	ft

Flow Analysis – NOTE: Only required when restriction of the stream will occur (e.g., bulkhead, jetty, etc.)

Key: ft = feet; cfs = cubic feet per second; fps = feet per second

Conclusion

No permanent or temporary fill will be placed within the wetland boundaries. No machinery will be permitted for use within the boundary of the wetland during demolition. All existing PSC piles located within the ESA boundaries will be cut off at the ground line during low tide. The proposed beam placement will be complete from an upstream area located outside of the orange barrier fencing. No work bridge permitted. No dredging anticipated. No wetland crossing anticipated during construction. Proposed PSC piles can be driven from the banks and outside of the wetlands.



BRIDGE CONSISTS OF

- I 40'-0" TYPE I MOD PSC BEAM SPANS ------ SPECIAL DESIGN
- 3 80'-0" TYPE T PSC BEAM SPANS ------ SPECIAL DESIGN
- 2 METAL SHELL PILE END BENTS ------ SPECIAL DESIGN
- 3 CONCRETE INTERMEDIATE BENTS ------ SPECIAL DESIGN
- 3 END POST AND GUARDRAIL ATTACHMENT DETAIL ----- GA, STD. 3054 (9-30-02) (L = 4' - 0''; W = |' - |''; H = 3' - 6'')
 - BAR BENDING DETAILS ----- GA. STD. 3901 (8-69)

CONCRETE BARRIERS - TEMPORARY ----- GA. STD. 4960 (5-10-07)

- DETAILS OF PRECAST TEMPORARY BARRIERS ----- GA. STD. 4961 (9-8-06)
- TYPICAL FILL DETAIL AT END OF BRIDGE ----- GA. STD. 9037 (9-99)

DRAINAGE DATA

DRAINAGE AREA			2.6 SQ MILES
FLOOD FREQUENCY	TOTAL DISCHARGE	MEAN VELOCITY	AREA OF OPENING UNDER FLOODSTAGE
25 YEAR	834 CFS	2.30 FPS	362 SQ FT
100 YEAR (OVER TOPPING)	7,448 CFS	4.57 FPS	1,630 SQ FT

TRAFFIC DATA

TRAFFIC	ADT	=	7,350	(20)22)
	ADT	ine:	8,250	(20)42)
DESIGN SPEED	0101010	<u>1997</u>		55	MPH
TRUCKS					2 %
24 HR TRUCKS	014070	0907		- 3.	,5 %
DIRECTIONAL				5	57 %

UTILITIES

NO UTILITIES ON BRIDGE

EXISTING UTILITIES

TELEPHONE CONDUITS ----- AT&T

GENERAL NOTES

- SPECIFICATIONS GEORGIA STANDARD SPECIFICATIONS, 2021 EDITION, AS MODIFIED BY CONTRACT DOCUMENTS.
- REINFORCING STEEL PLACE AND TIE ALL REINFORCING STEEL IN ACCORDANCE WITH THE GEORGIA DOT SPECIFICATIONS. DO NOT WELD REINFORCING STEEL. MAINTAIN 2" CLEARANCE ON ALL REINFORCEMENT UNLESS OTHERWISE NOTED.

CHAMFER - CHAMFER ALL EXPOSED CONCRETE EDGES 3/4" UNLESS OTHERWISE NOTED.

TEMPORARY SHORING - INCLUDE THE COST OF TEMPORARY SHORING AS NECESSARY FOR BRIDGE CONSTRUCTION IN THE OVERALL BID SUBMITTED.

- TEMPORARY BARRIERS, METHOD I PLACE TEMPORARY BARRIERS AS SHOWN ON THE PLANS AND GEORGIA STANDARD NOS. 4960 AND 4961 TO PROVIDE FOR 2 - 12'-0" TRAFFIC LANES. SUPPLY AND USE THE BARRIER IN ACCORDANCE WITH SECTION 620 OF THE GEORGIA DOT SPECIFICATIONS.
- TRAFFIC CONTROLS SEE ROADWAY PLANS FOR TRAFFIC CONTROLS AND TRAFFIC CONTROL PAYMENT.
- EXISTING BRIDGE PLANS ORIGINAL BRIDGE PLANS MAY BE OBTAINED ON THE GEORGIA DOT WEBSITE AT:

HTTP://WWW.DOT.GA.GOV/BS/PROJECTS/PROJECTSEARCH

THE ORIGINAL BRIDGE WAS BUILT UNDER PROJECT NUMBER BA(2)1791-A(15) (PROJECT ID NO. H007492).

- DIMENSIONS AND ELEVATIONS VERIFY ALL DIMENSIONS AND ELEVATIONS IN THE FIELD PRIOR TO ORDERING MATERIALS OR BUILDING FORMS. MEASURE CAMBER OF STAGE II BEAMS AND ADJUST "D" DIMENSION AND CAP ELEVATIONS AS NECESSARY FOR MEASURED CAMBER.
- EPOXY RESIN ADHESIVE APPLY EPOXY RESIN ADHESIVE TYPE I TO ALL HARDENED CONCRETE SURFACES JUST PRIOR TO POURING THE CONCRETE FOR THE NEXT STAGE OF CONSTRUCTION. SEE SECTION 886 OF THE GEORGIA DOT SPECIFICATIONS. INCLUDE THE COST OF EPOXY ADHESIVE AND ITS APPLICATION IN THE OVERALL BID SUBMITTED.
- WAITING PERIOD NONE REQUIRED.

COFFERDAMS - PROVIDE COFFERDAMS AT BENTS 2, 3 AND 4.

FOUNDATION BACKFILL MATERIAL - PLACE I'-O" OF TYPE I FOUNDATION BACKFILL MATERIAL UNDER EACH FOOTING AT BENTS 2, 3 AND 4. THE QUANTITY IS BASED ON THE PLAN FOOTING DIMENSIONS PLUS 2'-0"

PLAN DRIVING OBJECTIVE - SEE SUBSTRUCTURE DETAILS.

- DRIVING RESISTANCE DETERMINE DRIVING RESISTANCE FOR PILES USING DYNAMIC PILE TESTING IN ACCORDANCE WITH SUB-SECTION 520, 3.05.D.2 OF THE GEORGIA DOT SPECIFICATIONS. DYNAMIC PILE TESTING SHALL BE REQUIRED AT BENTS 3 RIGHT AND 5 | FET.
- DYNAMIC PILE TESTING PERFORM PILE TESTING USING THE PILE DRIVING ANALYZER (PDA) IN ACCORDANCE WITH SPECIAL PROVISION SECTION 523. NOTIFY THE GEOTECHNICAL BUREAU OF THE GEORGIA DOT OFFICE OF MATERIALS AND TESTING AT 404-608-4720 TWO WEEKS PRIOR TO DRIVING PILES.
- WAVE EQUATION PERFORM WAVE EQUATION ANALYSIS (WEAP) IN ACCORDANCE WITH SUB-SECTION 520.3.05.D.2 OF THE GEORGIA DOT SPECIFICATIONS. PROVIDE RESULTS OF THE WEAP TO THE GEOTECHNICAL BUREAU OF THE GEORGIA DOT OFFICE OF MATERIALS AND TESTING FOR REVIEW AND APPROVAL TWO WEEKS PRIOR TO DRIVING PILES.
- PILE DRIVING SHOULD PILES FAIL TO OBTAIN DRIVING RESISTANCE AFTER ACHIEVING THE PILE TIP ELEVATIONS SHOWN, ALLOW PILES TO FREEZE A MINIMUM OF 24 HOURS AND RESTRIKE WITH A WARM HAMMER.

BENT NUMBER	PILE TIP ELEVATION
L	-57.00
2	-66.00
3	-68.00
4	-65.00
5	-58.00

- METAL SHELL PILES USE A MINIMUM SHELL THICKNESS OF 5/16" FOR ALL MS PILES. USE THESE SHELL THICKNESSES IN LIEU OF THOSE CALLED FOR IN SUB-SECTION 520.3.05.M AND SUB-SECTION 855.2.01.A.1 OF THE GEORGIA DOT SPECIFICATIONS.
- PILE CLOSURE PLATE DETAIL USE CLOSURE PLATE OPTION 2 AT THIS SITE IN ACCORDANCE WITH SUB-SECTION 520.3.05.M OF THE GEORGIA DOT SPECIFICATIONS.
- SMOOTH DOWEL BARS PLACE SMOOTH DOWEL BARS IN FORMED 3" DIAMETER X 12" DEEP HOLES AND GROUT IN PLACE SIMILAR TO ANCHOR BOLTS, SEE SUB-SECTION 501.3.05.B.3 OF THE GEORGIA DOT SPECIFICATIONS. STIRRUPS MAY BE SHIFTED SLIGHTLY TO CLEAR FORMED HOLES.

STANDARD PLAN MODIFICATION - MODIFY THE APPROACH SLAB STANDARD TO INCREASE THE 3/4" EXPANSION JOINT SHOWN BETWEEN THE APPROACH SLAB AND THE BACK FACE PAVING REST AND END POST TO I" AT BENTS I AND 5. KEEP THE EXPANSION JOINT AT END BENT CONTINUOUS AROUND END POST. INCLUDE COST OF JOINT IN APPROACH SLAB PAYMENT. SEE ROADWAY PLANS FOR APPROACH SLAB PAYMENT.

GROOVED CONCRETE - GROOVE THE ENTIRE LENGTH OF THE BRIDGE TRANSVERSELY AS PER SUB-SECTION 500.3.05.T.9.C OF THE GEORGIA DOT SPECIFICATIONS. DO NOT GROOVE UNDER SIDEWALK. DO NOT GROOVE UNDER RAISED MEDIAN.

EXTERIOR BEAM BRACING - THE CONTRACTOR SHALL PROVIDE BRACING IN STAGE I BETWEEN EXTERIOR BEAM I AND INTERIOR BEAM 2 AND BETWEEN EXTERIOR BEAM 5 AND INTERIOR BEAM 4 UNTIL THE DECK HAS BEEN POURED AND THE OVERHANG FORMS HAVE BEEN REMOVED. THE CONTRACTOR SHALL PROVIDE BRACING IN STAGE 2 BETWEEN EXTERIOR BEAM II AND INTERIOR BEAM IO UNTIL THE DECK HAS BEEN POURED AND THE OVERHANG FORMS HAVE BEEN REMOVED. ALL COST FOR DESIGNING, PROVIDING, INSTALLING AND REMOVING BRACING SHALL BE INCLUDED IN PRICE BID FOR LUMP SUPERSTRUCTURE CONCRETE.

WELDING - ALL WELDING ON GEORGIA DOT PROJECTS SHALL BE PERFORMED BY CERTIFIED WELDERS THAT HAVE IN THEIR POSSESSION A CURRENT WELDING CERTIFICATION CARD ISSUED BY THE OFFICE OF MATERIALS AND TESTING. USE ONLY E70XX (EXCLUDING E7014 AND E7024) LOW HYDROGEN ELECTRODES FOR MANUAL SHIELDED METAL ARC WELDING.

BRIDGE REMOVAL - REMOVE EXISTING BRIDGE AS PER SUB-SECTION 540.3.05 OF THE GEORGIA DOT SPECIFICATIONS.

INCIDENTAL ITEMS - INCLUDE THE COST INCIDENTAL TO THE WORK THAT IS NOT SPECIFICALLY COVERED BY THE GEORGIA STANDARD SPECIFICATIONS, SUPPLEMENTAL SPECIFICATIONS AND/OR SPECIAL PROVISIONS IN THE OVERALL BID SUBMITTED. THIS INCLUDES THE COST OF WATERPROOFING, JOINT FILLERS AND OTHER INCIDENTAL ITEMS NECESSARY TO COMPLETE THE WORK.

PROJECT.

DRAWING

35-00 BRIDGE S

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SALVAGE MATERIAL - NO MATERIAL REMOVED FROM THE EXISTING STRUCTURE SHALL BE SALVAGED FOR USE BY THE GEORGIA DOT.

STEEL DIAPHRAGMS - SUBSTITUTION FOR STEEL DIAPRHAGMS IS NOT ALLOWED FOR THIS

	236	BRIDGE NO. I
	DATE	GEORGIA DEPARTMENT OF TRANSPORTATION ENGINEERING DIVISION-OFFICE OF BRIDGES AND STRUCTURES
	REVISIONS	GENERAL NOTES SR 25 (US I7) OVER THORNHILL CREEK GLYNN COUNTY 0016985
NO. 02		NO SCALE MARCH 202
HEET 3I	BΥ	DESIGNED JTM CHECKED ASA REVENED DLC/SKG DRAWN JTM DESIGN GROUP DLW APPROVED WMD

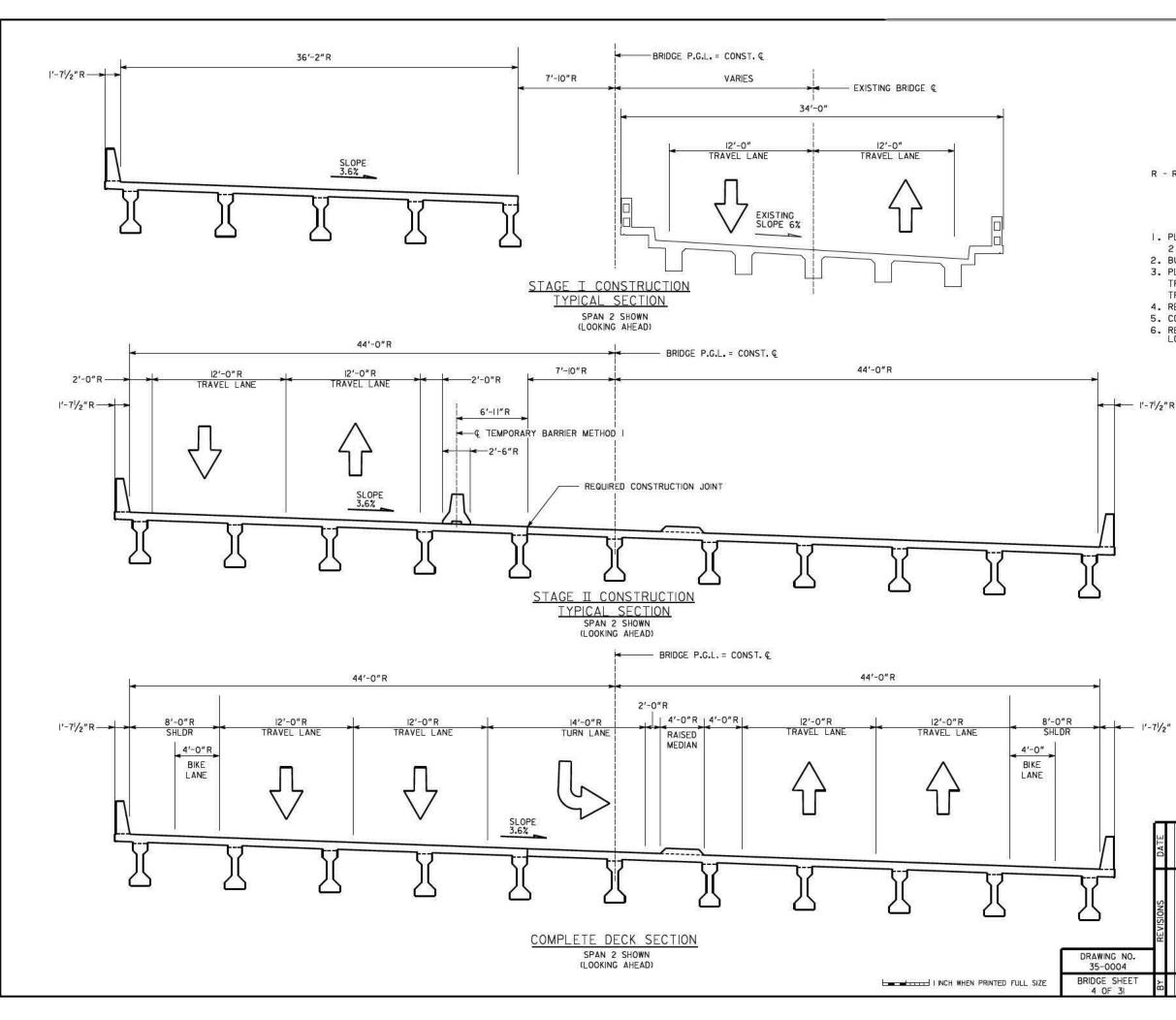
<u>DESIGN DATA</u>

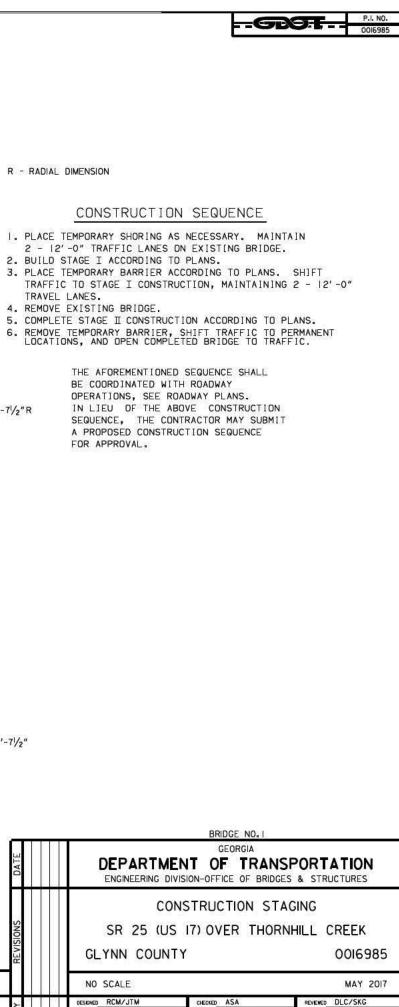
SPECIFICATIONS (DESIGNED FOR SEISMIC PERFORMANCE ZONE 2, SDI = 0.163	
DESIGN VEHICLE LIVE LOAD	13
FUTURE PAVING ALLOWANCE 30 LBS PER SO F	Т
CONCRETE: SUPERSTRUCTURE CLASS D, fc = 4,000 PS BARRIER CLASS D, fc = 4,000 PS PSC BEAMS CLASS AAA, fc = SEE BEAM SHEET PSC BEAM ALLOWABLE TENSION CLASS AAA, fc = SEE BEAM SHEET SUBSTRUCTURE CLASS AA, fc = 3,500 PS	SI S S
REINFORCEMENT STEEL: GRADE 60, fy = 60,000 PS	I
PRETENSIONING STRANDS: f' = 270,000 PS	; I
METAL SHELL PILES: GRADE 3, fy = 45,000 PS	ι

SUMMARY OF QUANTITIES

PAY ITEM NUMBER	QUANTITY	<u>UNIT</u>	PAY ITEM
207-0203	87	CY	FOUND BKFILL MATL, TP II
211-0300	600	CY	BRIDGE EXCAVATION, STREAM CROSSING
500-0100	255	SY	GRODVED CONCRETE
500-1011	LUMP	LS	SUPERSTR CONCRETE, CL D, BR NO - I (756)
500-2100	55	LF	CONCRETE BARRIER
500-3002	520	CY	CLASS AA CONCRETE
507-8900	424	LF	PSC BEAMS, AASHTO TYPE I MOD, BR NO - I
507-9003	2604	LF	PSC BEAMS, AASHTO TYPE III, BR NO - I
511-1000	85357	LB	BAR REINF STEEL
511-3000	LUMP	LS	SUPERSTR REINF STEEL, BR NO - I (186245)
520-1316	5570	LF	PILING IN PLACE, METAL SHELL, 16 IN OD
520-4316	Ê	EA	LOAD TEST, METAL SHELL, 16 IN OD (IF REQD)
523-1100	2	EA	DYNAMIC PILE TEST
525-1000	12	EA	COFFERDAM
540-1101	LUMP	LS	REMOVAL OF EXISTING BR, STA NO - 562+54
603-2024	1605	SY	STN DUMPED RIP RAP, TP 1, 24 IN
603-7000	1605	SY	PLASTIC FILTER FABRIC
620-0100	602	LF	TEMPORARY BARRIER, METHOD NO. I

				629	P.I. NO. 0016985
				BRIDGE NO. I	
	DATE		DEPARTMENT		
	H		ENGINEERING DIVISION-	NERAL NOTES	NU STRUCTURES
	REVISIONS		SR 25 (US 17)	OVER THORNH	
NO.	RE		GLYNN COUNTY		OOI6985
03 HEET 3I	ВΥ	4-14 9-19	designed JTM ci	HECKED ASA ESKON CROUP DLW	MARCH 2021 REVEWED DLC/SKG APPROVED WMD
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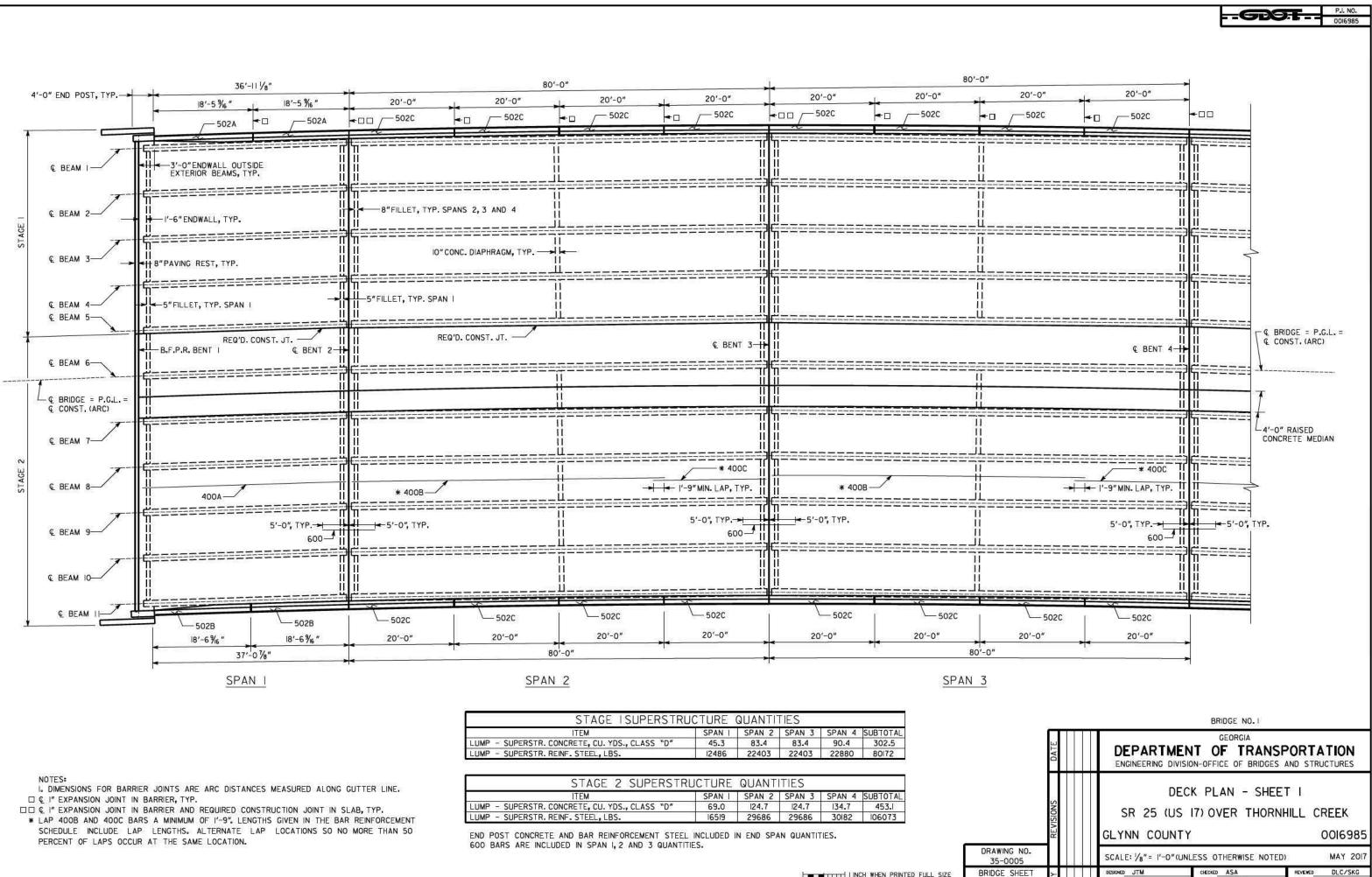




DRAWN RCM/JTM

DESIGN GROUP DLW

APPROVED WMD



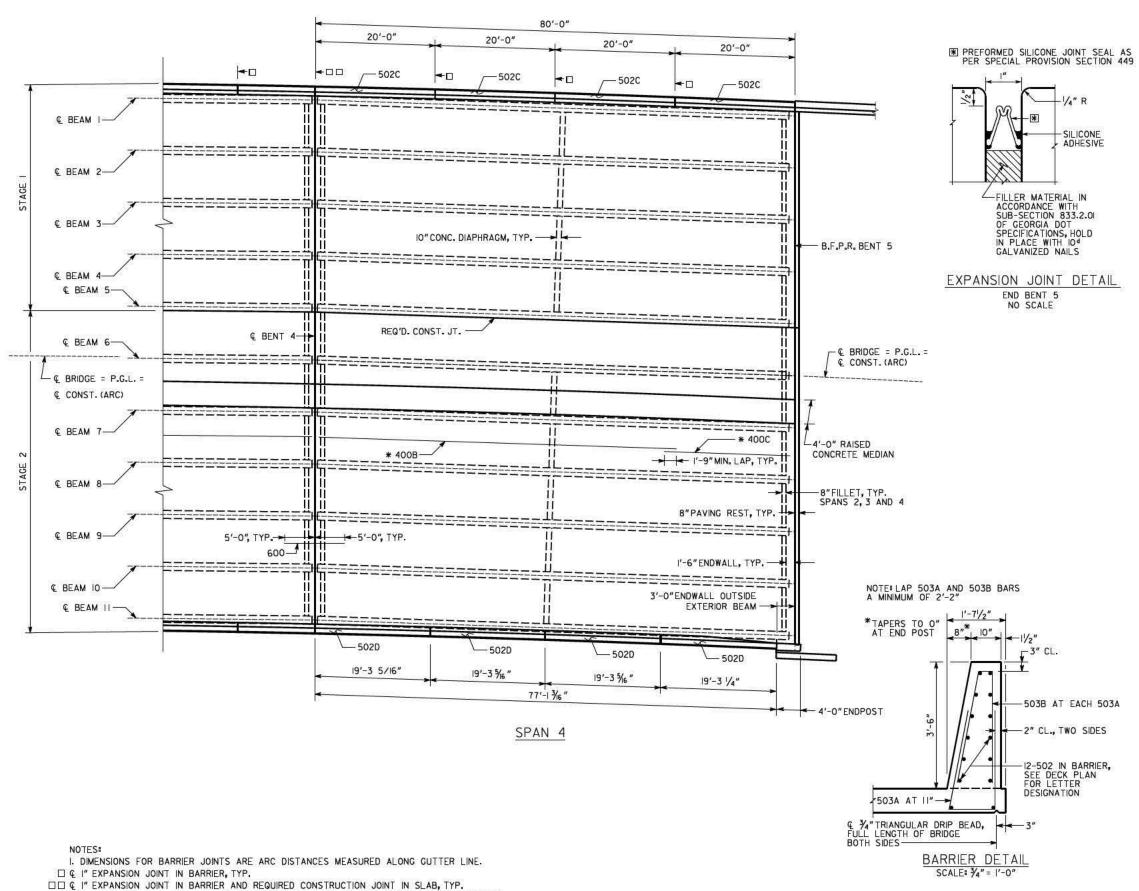
STAGE SUPERSTRL	ICTURE	QUANTI	TIES		
ITEM	SPAN I	SPAN 2	SPAN 3	SPAN 4	SUBTOTAL
LUMP - SUPERSTR. CONCRETE, CU. YDS., CLASS "D"	45.3	83.4	83.4	90.4	302.5
LUMP - SUPERSTR. REINF. STEEL, LBS.	12486	22403	22403	22880	80172

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ESIGN GROUP DI

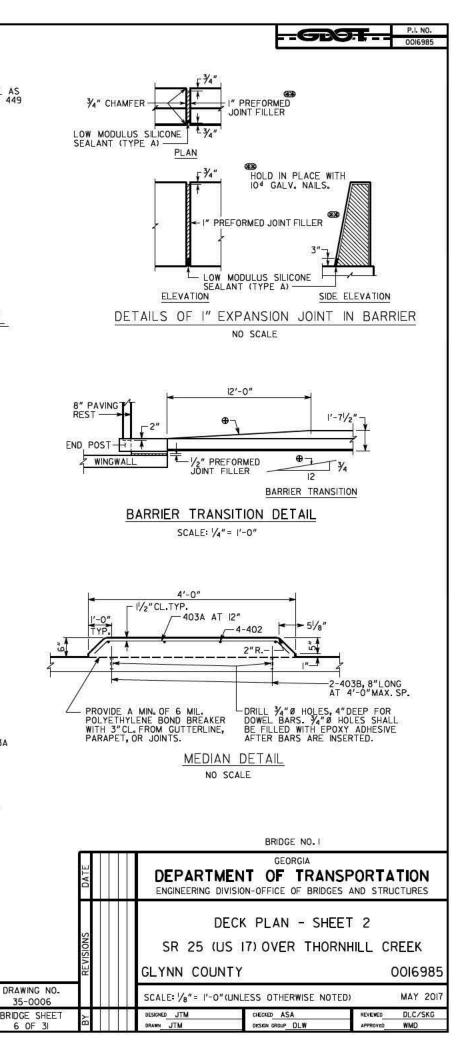
JTM

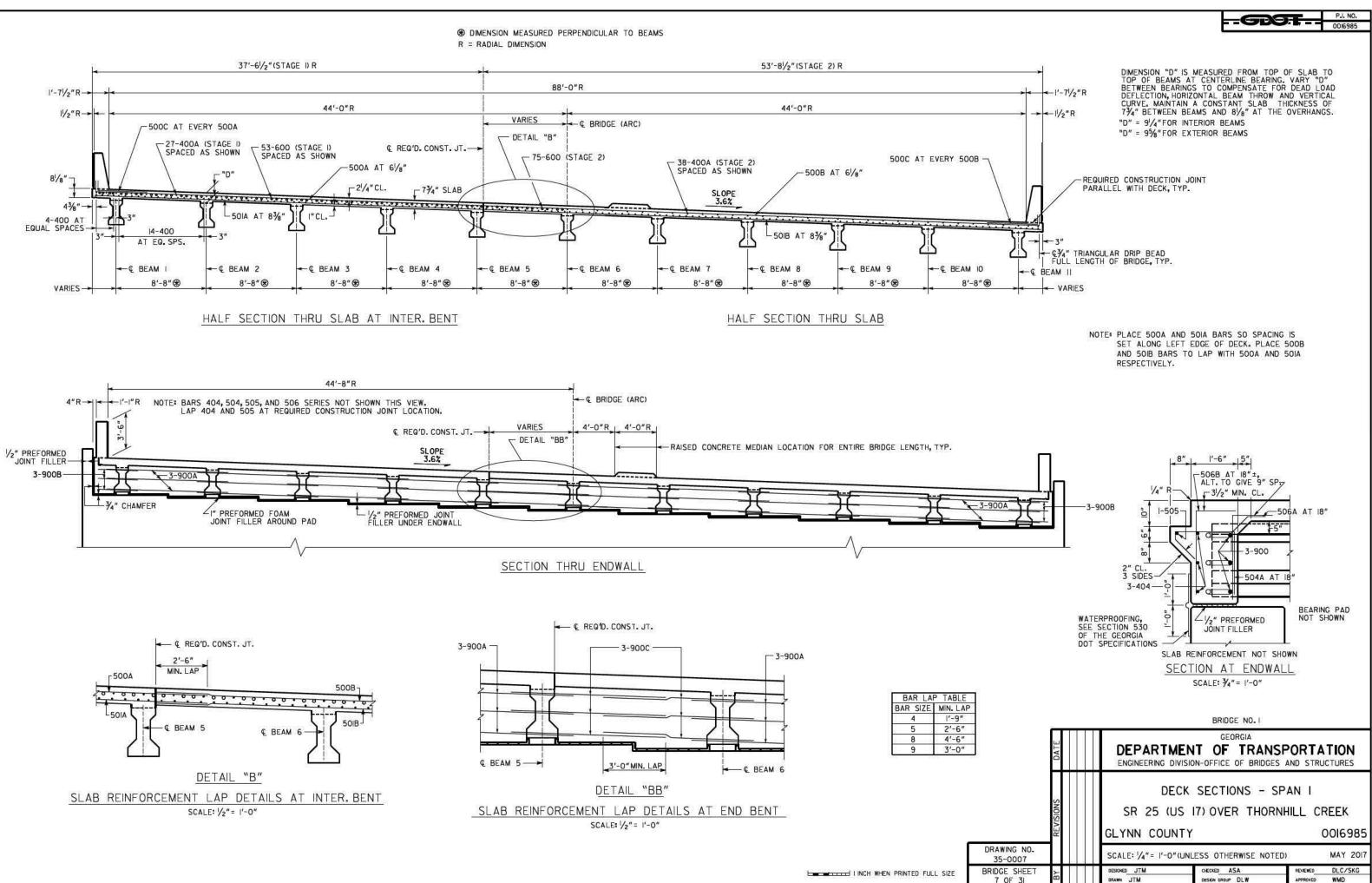


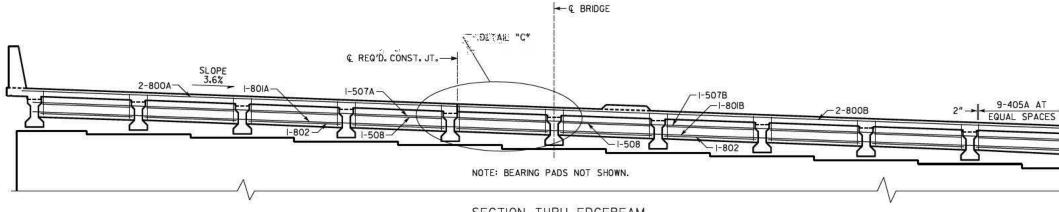
* LAP 400B AND 400C BARS A MINIMUM OF 1'-9". LENGTHS GIVEN IN THE BAR REINFORCEMENT SCHEDULE INCLUDE LAP LENGTHS. ALTERNATE LAP LOCATIONS SO NO MORE THAN 50

PERCENT OF LAPS OCCUR AT THE SAME LOCATION.

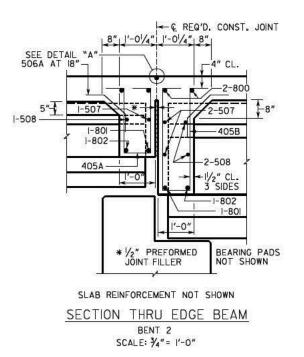
35-0006 BRIDGE SHEET 6 OF 31

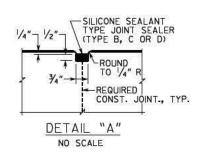


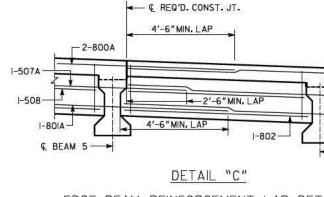








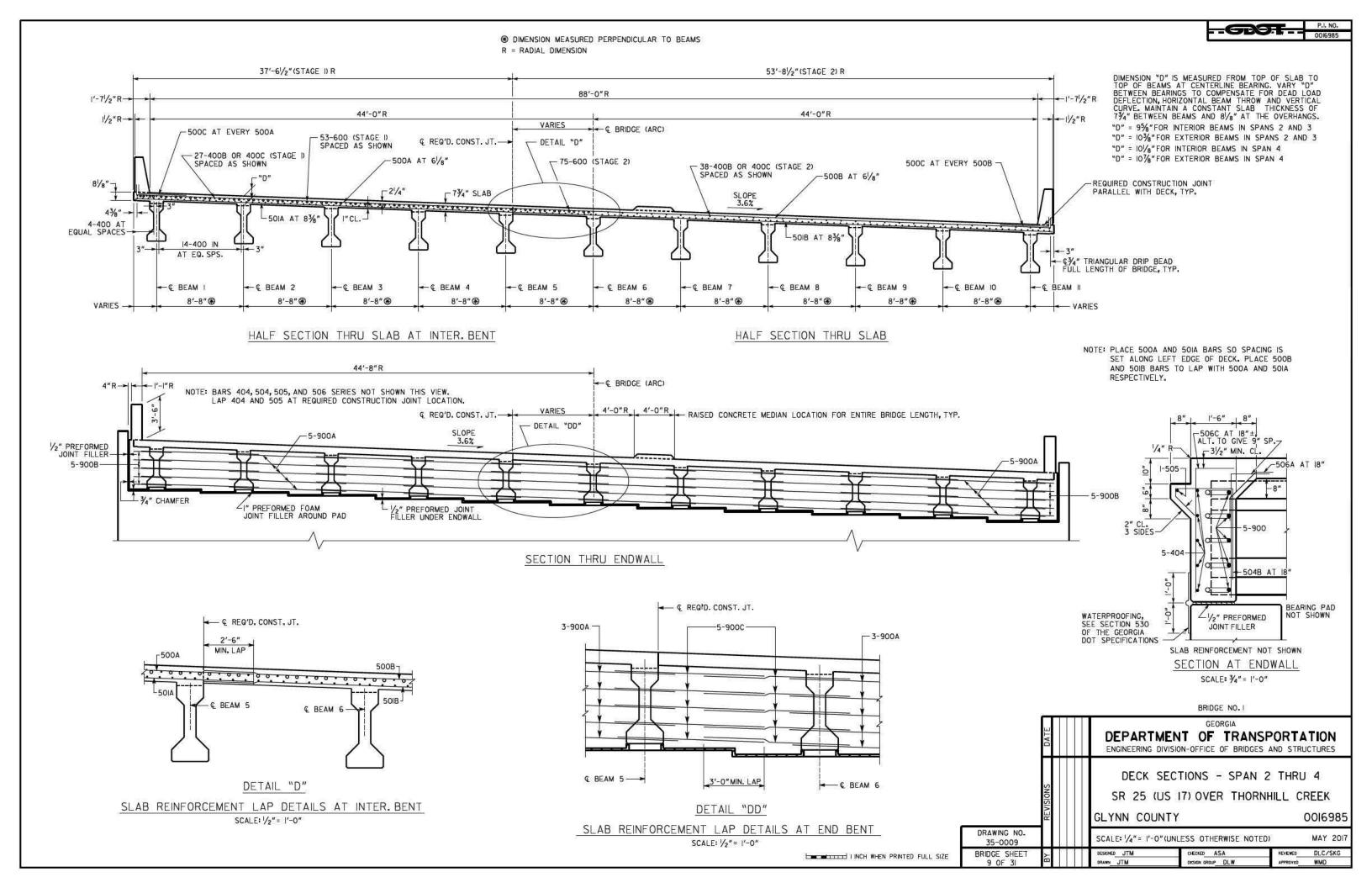




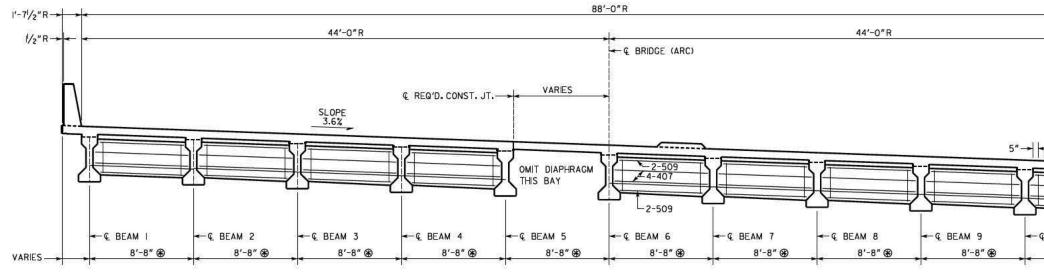
EDGE BEAM REINFORCEMENT LAP DET



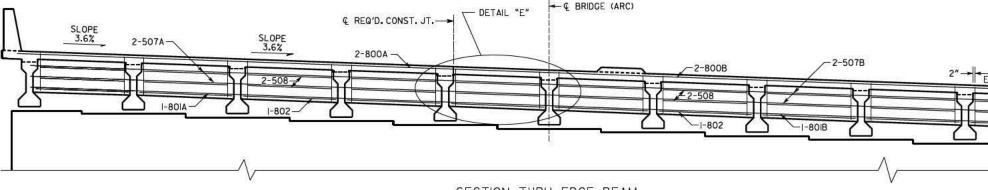
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<u>-</u> 2"			
	GE BEAM FLUSH WITH FACE OF BEAM, TYP-		
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040.4 3 HOLE ROMARKAMINGCOS			
TAILS			
		BRIDGE NO. I	
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	DECK	SECTIONS - SP	AN I
SNO		17) OVER THORNE	
REVISIONS	GLYNN COUNT		0016985
NO-	10842310304 N2052660985	LESS OTHERWISE NOTED)	
HEET 🚡	DESIGNED JTM	CHECKED ASA	REVEWED DLC/SKG
31 🕮	DRAWN_JTM	DESIGN GROUP DLW	APPROVED WMD

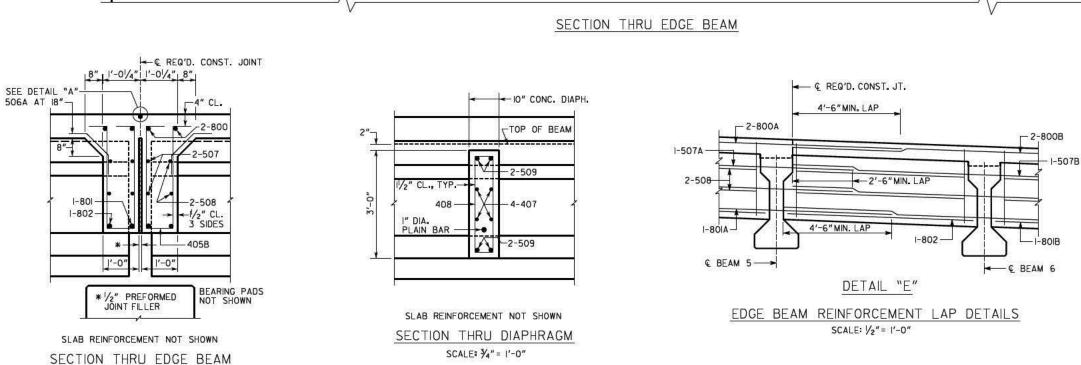


❀ DIMENSION MEASURED PERPENDICULAR TO BEAMS R = RADIAL DIMENSION



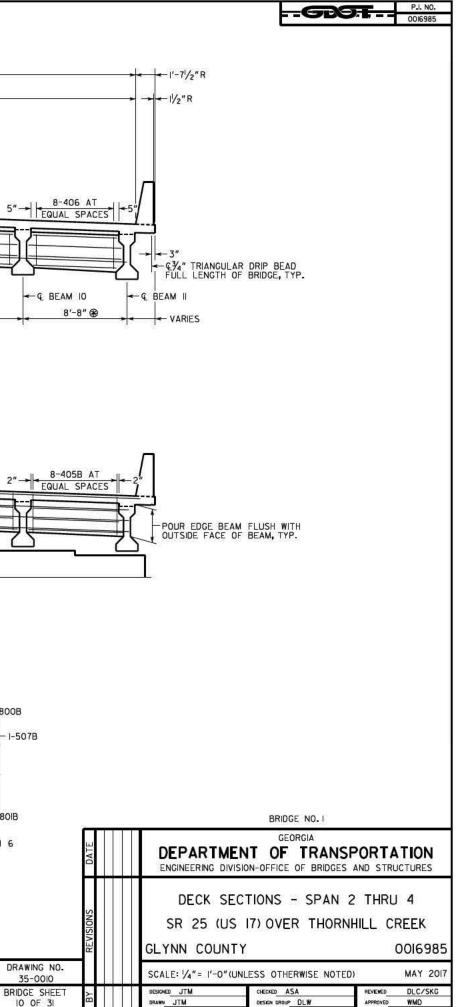
SECTION THRU DIAPHRAGM

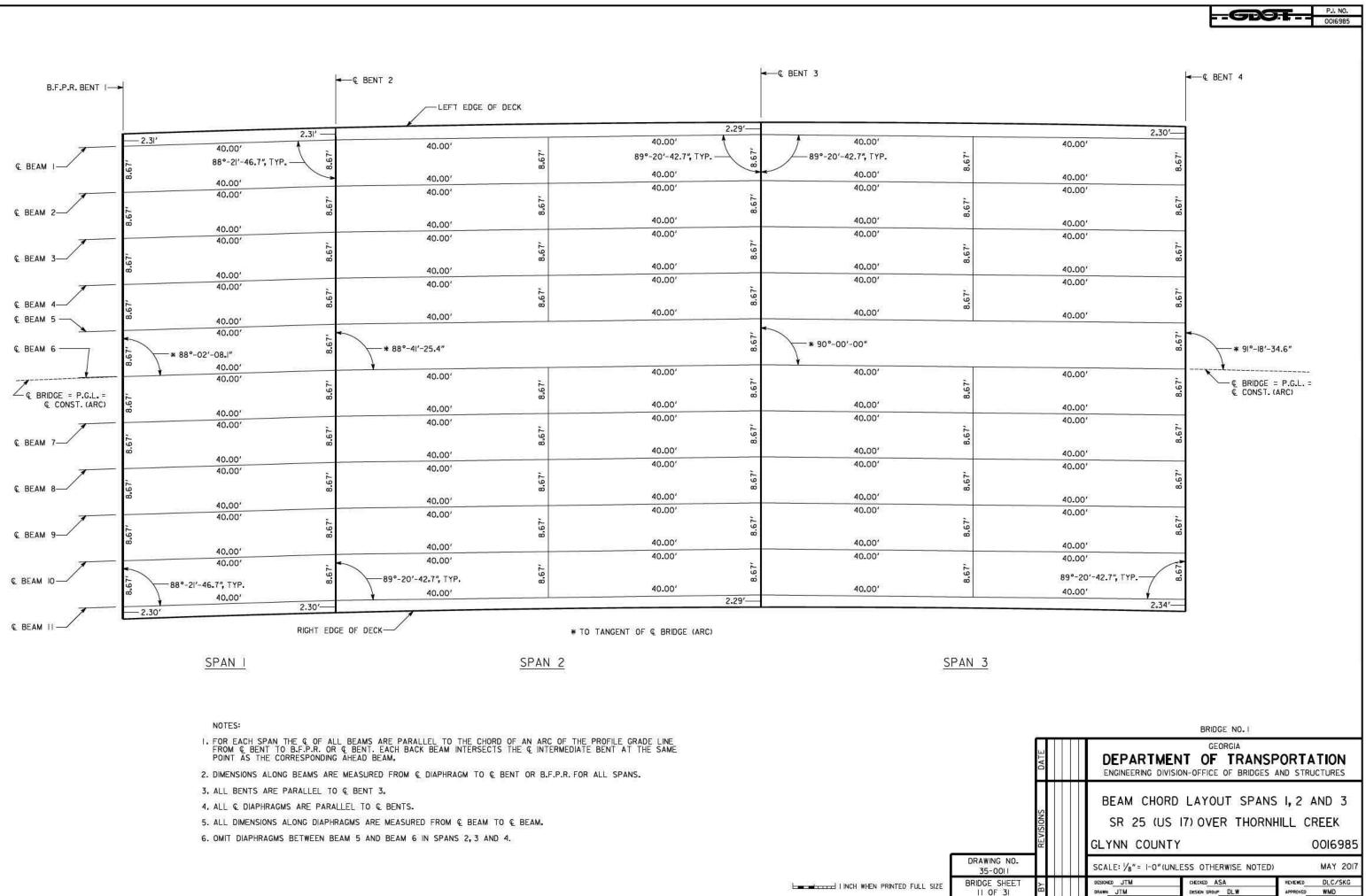


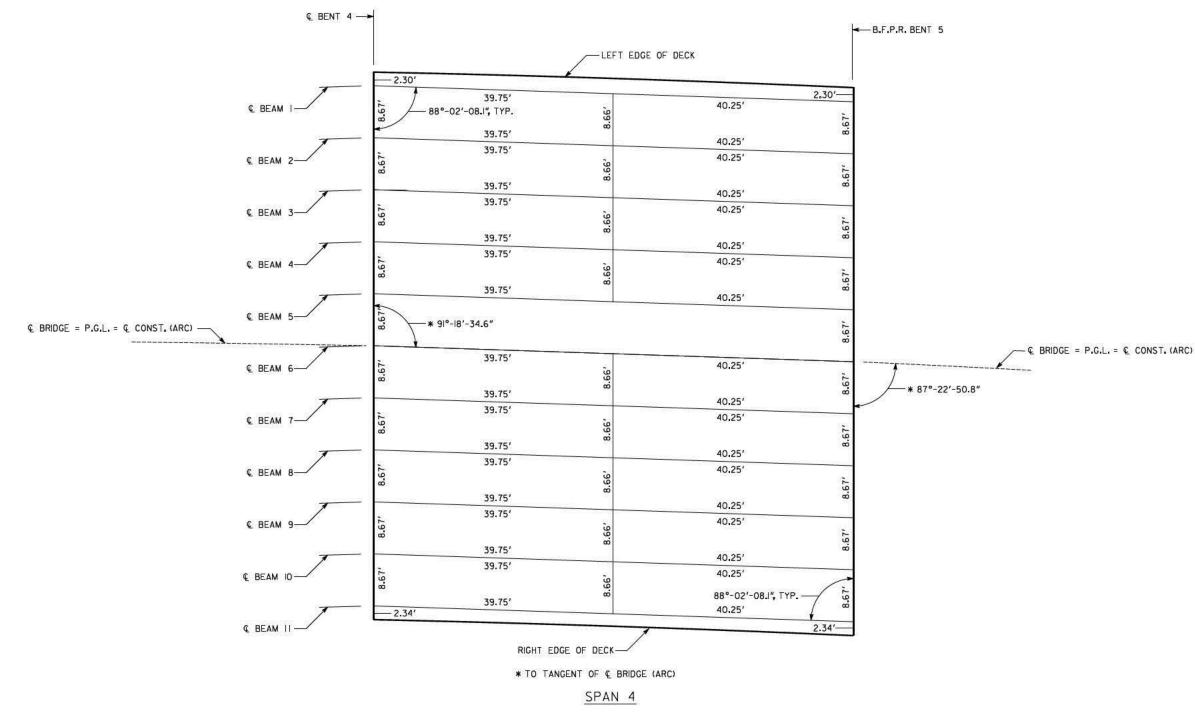


BENT 3 AND 4 SCALE: 3/4"= 1'-0"

BRIDGE SHEET IO OF 31







NOTES:

1. FOR EACH SPAN THE € OF ALL BEAMS ARE PARALLEL TO THE CHORD OF AN ARC OF THE PROFILE GRADE LINE FROM € BENT TO B.F.P.R. OR € BENT. EACH BACK BEAM INTERSECTS THE € INTERMEDIATE BENT AT THE SAME POINT AS THE CORRESPONDING AHEAD BEAM.

2. DIMENSIONS ALONG BEAMS ARE MEASURED FROM & DIAPHRAGM TO & BENT OR B.F.P.R. FOR ALL SPANS.

3. ALL BENTS ARE PARALLEL TO & BENT 3.

4. ALL & DIAPHRAGMS ARE PARALLEL TO & BENTS.

5. ALL DIMENSIONS ALONG DIAPHRAGMS ARE MEASURED FROM & BEAM TO & BEAM.

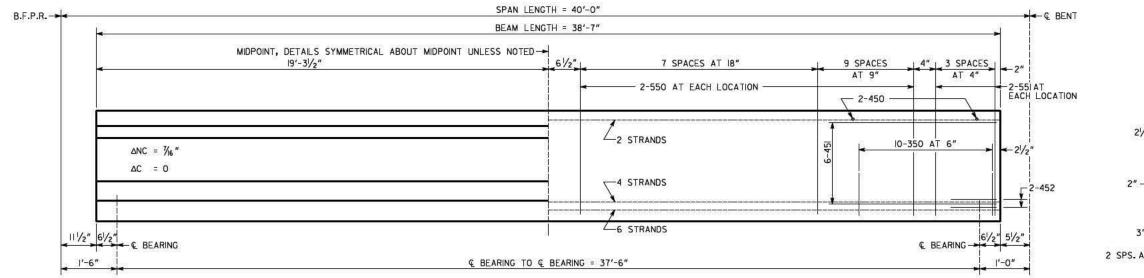
6. OMIT DIAPHRAGMS BETWEEN BEAM 5 AND BEAM 6 IN SPANS 2, 3 AND 4.

			BRIDGE NO. I	
	DATE		GEORGIA ENT OF TRAN VISION-OFFICE OF BRIDGE	양양 전 것 못 못 여러 있다. 말 방송 것 같은 것 같
	REVISIONS		CHORD LAYOUT S 17) OVER THOR TY	
3 NO. 012		SCALE 1/8"= 1'-0"	UNLESS OTHERWISE NO	TED) MAY 2017
SHEET 3I	BΥ	designed JTM drawn JTM	CHECKED ASA DESIGN GROUP DLW	REVEWED DLC/SKG APPROVED WMD

- GDJ--

P.I. NO.

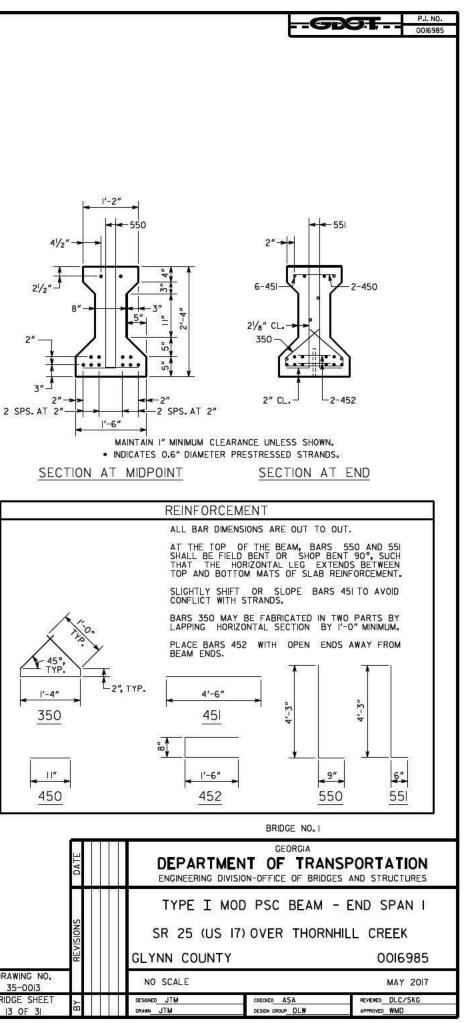
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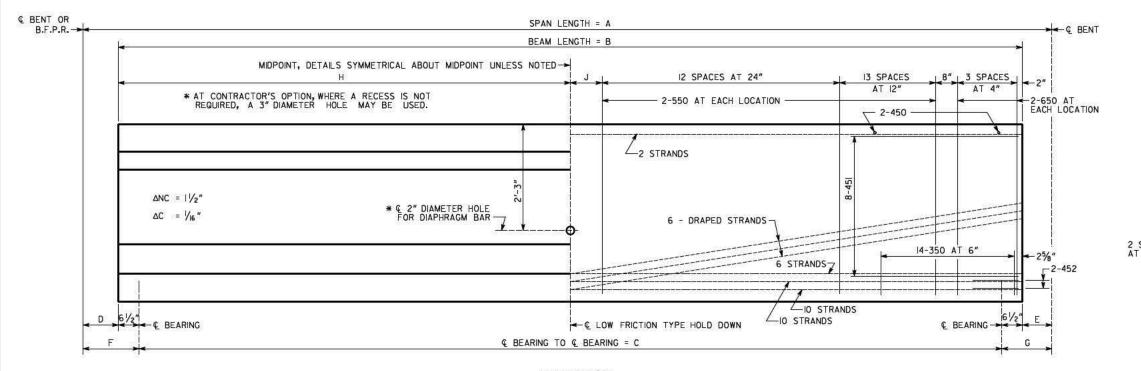
ELEVATION

- I. BEAMS SHALL BE MAINTAINED IN AN UPRIGHT POSITION AT ALL TIMES AND SHALL BE PICKED UP WITHIN 3'-6" FROM THEIR ENDS. DISREGARDING THIS REQUIREMENT COULD LEAD TO COLLAPSE OF THE BEAM. PICK-UPS SHALL BE EMBEDDED TO WITHIN 4" OF THE BOTTOM OF THE BEAM. DETAILS OF PICK-UPS SHALL BE INCLUDED IN THE SHOP DRAWINGS.
- 2. CHAMFER EDGES OF BEAMS 1/2" OR 3/4".
- 3. HORIZONTAL DIMENSIONS ARE IN PLACE DIMENSIONS. THE BEAM LENGTH INCLUDES THE $J_{8}^{\prime\prime}$ EPOXY MORTAR AT EACH END. SHOP DRAWINGS SHALL ADJUST HORIZONTAL DIMENSIONS FOR GRADE AND FABRICATION EFFECTS SUCH AS SHRINKAGE AND ELASTIC SHORTENING.
- 4. AT & BEARING, FORM A 134" DIAMETER X 7" DEEP HOLE AT THE FIXED ENDS AND A 4" X 1 $\frac{3}{4}$ " X 7" DEEP SLOT AT THE EXPANSION ENDS FOR A 1 $\frac{1}{2}$ " DIAMETER SMOOTH DOWEL. SEE PLAN AND ELEVATION SHEET FOR LOCATION OF FIXED AND EXPANSION ENDS.
- 5. TOPS OF BEAMS SHALL BE ROUGH FLOATED AT APPROXIMATELY THE TIME OF INITIAL SET. ENTIRE TOP SHALL BE SCRUBBED TRANSVERSELY WITH A COARSE BRUSH TO REMOVE ALL LAITANCE AND TO PRODUCE A ROUGHENED SURFACE FOR BONDING TO THE SLAB. ROUGHENED SURFACE SHALL HAVE AN AMPLITUDE OF APPROXIMATELY 1/4". CONCRETE FINS OR PROJECTIONS SHALL BE REMOVED TO PRODUCE A VERTICAL FACE AT THE EDGE OF THE BEAM.
- 6. NON-COMPOSITE DEAD LOAD DEFLECTION (ANC) AT THE MIDPOINT IS DUE TO THE WEIGHT OF THE SLAB AND COPING.
- 7. COMPOSITE DEAD LOAD DEFLECTION (AC) AT THE MIDPOINT IS DUE TO THE WEIGHT OF BARRIER AND RAISED MEDIAN.
- 8. STRANDS SHALL MEET ALL REQUIREMENTS OF ASTM A 416 GRADE 270.
- 9. PRESTRESSING DATA IS AS FOLLOWS:
 - A. USE 12 0.6" DIAMETER LOW-RELAXATION (A = 0.217 SQ IN) STRANDS. PRETENSION STRANDS TO 43,943 LBS EACH.
 - B. PRETENSIONED STRANDS SHALL BE RELEASED AFTER THE CONCRETE HAS REACHED A MINIMUM STRENGTH (fc1) OF 4,500 PSI.
 - C. INCLUDING THE TOP STRANDS, THE TOTAL JACKING FORCE OF PRETENSIONING IS 527,316 LBS.
 - D. INCLUDING THE TOP STRANDS, THE NET PRESTRESSING FORCE OF THE STRANDS AFTER ALL LOSSES IS 419,075 LBS.
- IO. CONCRETE STRENGTH (fc) = 5,000 PSI.
- II. ALLOWABLE PSC BEAM TENSION = 425 PSI.



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-	13 OF 31

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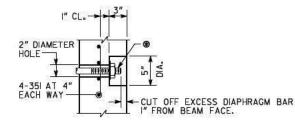


NOTES

ELEVATION

- I. BEAMS SHALL BE MAINTAINED IN AN UPRIGHT POSITION AT ALL TIMES AND SHALL BE PICKED UP WITHIN 5'-6" FROM THEIR ENDS. DISREGARDING THIS REQUIREMENT COULD LEAD TO COLLAPSE OF THE BEAM. PICK-UPS SHALL BE EMBEDDED TO WITHIN 4" OF THE BOTTOM OF THE BEAM. DETAILS OF PICK-UPS SHALL BE INCLUDED IN THE SHOP DRAWINGS.
- 2. CHAMFER EDGES OF BEAMS 1/2" OR 1/4".
- 3. HORIZONTAL DIMENSIONS ARE IN PLACE DIMENSIONS. THE BEAM LENGTH INCLUDES THE $1/\!\!/_8"$ EPOXY MORTAR AT EACH END. SHOP DRAWINGS SHALL ADJUST HORIZONTAL DIMENSIONS FOR GRADE AND FABRICATION EFFECTS SUCH AS SHRINKAGE AND ELASTIC SHORTENING.
- 4. AT & BEARING, FORM A 134" DIAMETER X 7" DEEP HOLE AT THE FIXED ENDS AND A 4" X 13/4" X 7" DEEP SLOT AT THE EXPANSION ENDS FOR A 11/2" DIAMETER SMOOTH DOWEL. SEE PLAN AND ELEVATION SHEET FOR LOCATION OF FIXED AND EXPANSION ENDS.
- 5. TOPS OF BEAMS SHALL BE ROUGH FLOATED AT APPROXIMATELY THE TIME OF INITIAL SET. ENTIRE TOP SHALL BE SCRUBBED TRANSVERSELY WITH A COARSE BRUSH TO REMOVE ALL LAITANCE AND TO PRODUCE A ROUGHENED SURFACE FOR BONDING TO THE SLAB. ROUGHENED SURFACE SHALL HAVE AN AMPLITUDE OF APPROXIMATELY 1/4". CONCRETE FINS OR PROJECTIONS SHALL BE REMOVED TO PRODUCE A VERTICAL FACE AT THE EDGE OF THE BEAM.
- 6. NON-COMPOSITE DEAD LOAD DEFLECTION (ANC) AT THE MIDPOINT IS DUE TO THE WEIGHT OF THE SLAB AND COPING.
- 7. COMPOSITE DEAD LOAD DEFLECTION (AC) AT THE MIDPOINT IS DUE TO THE WEIGHT OF BARRIER AND RAISED MEDIAN.
- 8. STRANDS SHALL MEET ALL REQUIREMENTS OF ASTM A 416 GRADE 270.
- 9. PRESTRESSING DATA IS AS FOLLOWS:
 - A. USE 28 0.6" DIAMETER LOW-RELAXATION (A = 0.217 SQ IN) STRANDS. PRETENSION STRANDS TO 43,943 LBS EACH.
 - B. PRETENSIONED STRANDS SHALL BE RELEASED AFTER THE CONCRETE HAS REACHED A MINIMUM STRENGTH (fc1) OF 6,000 PSI.
 - C. INCLUDING THE TOP STRANDS, THE TOTAL JACKING FORCE OF PRETENSIONING IS 1.230.404 LBS
 - D. INCLUDING THE TOP STRANDS, THE NET PRESTRESSING FORCE OF THE STRANDS AFTER ALL LOSSES IS 956,445 LBS.
- IO. CONCRETE STRENGTH $(f_c) = 6,500$ PSI.
- 11. ALLOWABLE PSC BEAM TENSION = 484 PSI.

	DIMENSIONS								
SPAN	Δ	В	С	D	E	F	G	Н	J
2 AND 3	80-0"	79'-l"	78'-0"	51/2"	5 ¹ /2"	l'-0"	l'-0"	39'-6 ^l /2"	8½″
4	60-0"	78'-7"	77'-6″	111/2"	5 ¹ /2"	l'-6"	I'-0"	39'-31/2"	51/2"



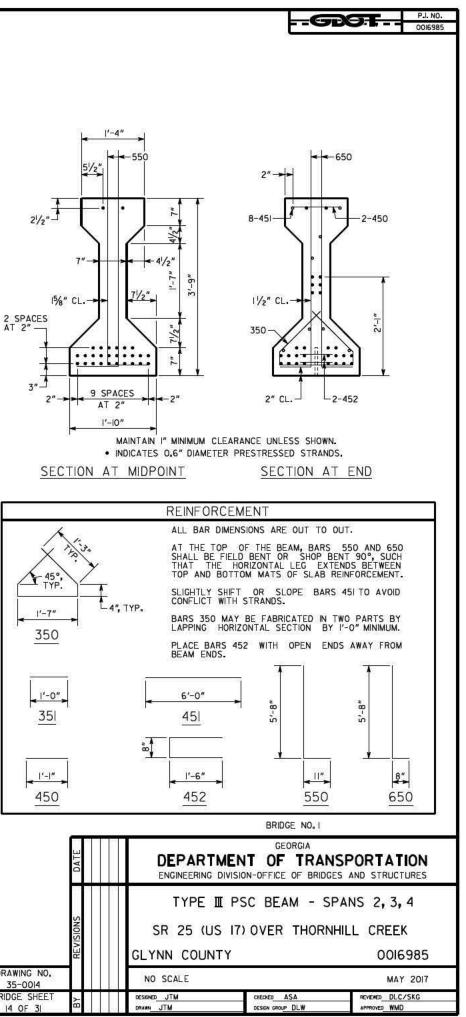
@ DIAPHRAGM BAR SHALL BE A I" DIAMETER PLAIN BAR, THREADED 5" ON EACH END, WITH $\frac{1}{4}$ " X $\frac{3}{2}$ " DIAMETER WASHERS AND HEX NUTS (ASTM A 709 GRADE 36).

TIGHTEN DIAPHRAGM BAR AS PER SUB-SECTION 507.3.05.C OF THE GEORGIA DOT SPECIFICATIONS.

AFTER EXCESS DIAPHRAGM BAR HAS BEEN CUT OFF, PAINT DIAPHRAGM BAR, WASHER, AND NUT EXPOSED IN RECESS WITH SPECIAL PROTECTIVE COATING NO. 2 P AS PER SECTION 535 OF THE GEORGIA DOT SPECIFICATIONS. AFTER PAINTING, FILL THE RECESS WITH AN APPROVED EPOXY GROUT.

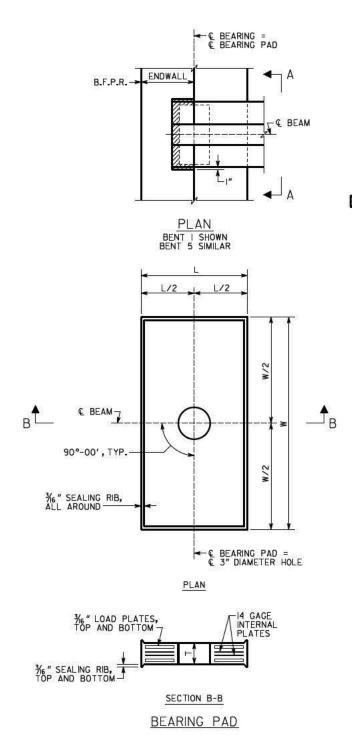
GALVANIZING OF THE DIAPHRAGM BAR AS PER SUB-SECTION 865.2.0I.B.I2 OF THE GEORGIA DOT SPECIFICATIONS IS NOT REQUIRED.

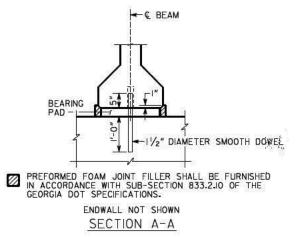
RECESS DETAIL FOR DIAPHRAGM BAR ENDS



	DRAWING NO.
	35-00 4
ZE	BRIDGE SHEET
ZE	14 OF 31

I INCH WHEN PRINTED FULL SIZ



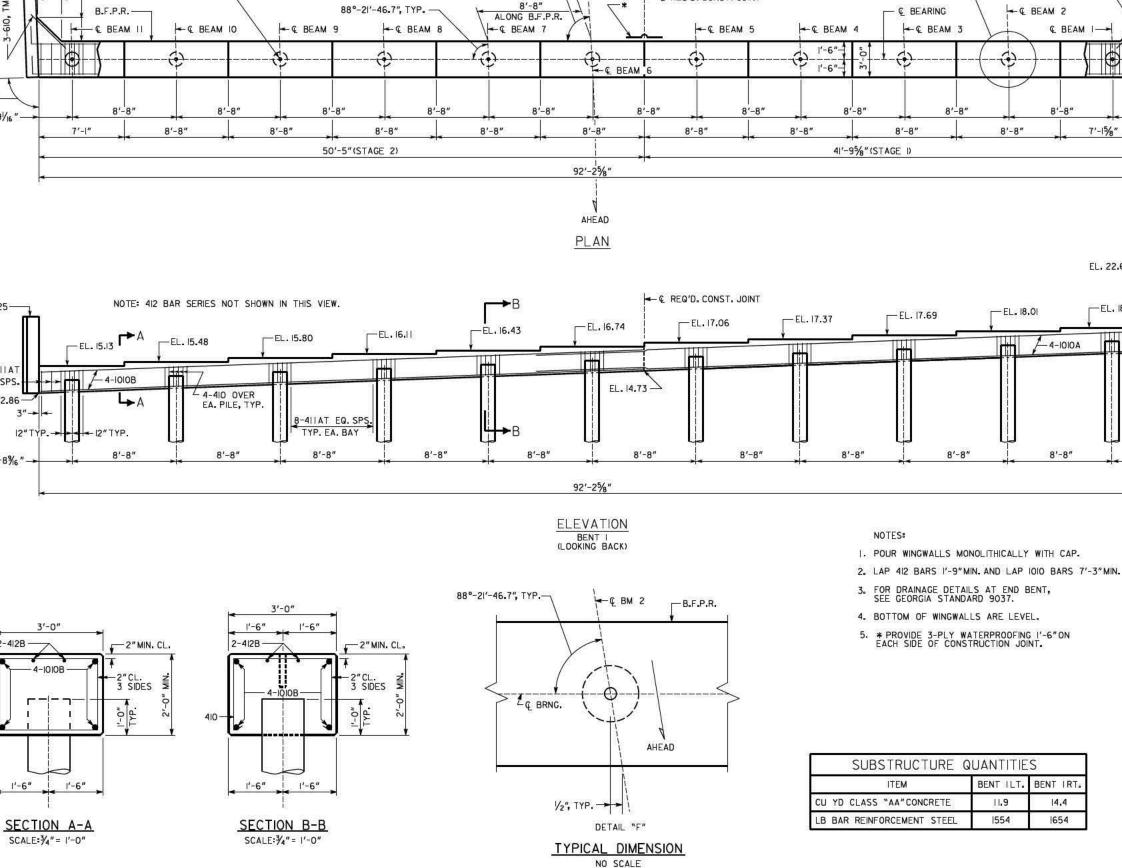


NOTES

- I. BEARING PADS HAVE BEEN DESIGNED ACCORDING TO AASHTO LRFD BRIDGE SECTION 14.7.6 METHOD A AND SHALL BE FURNISHED IN ACCORDANCE WITH CONSTRUCTION SPECIFICATIONS, SECTION 18, BEARING DEVICES.
- 2. 11/2" DIAMETER SMOOTH DOWELS SHALL BE ASTM A 709 GRADE 50.
- 3. BEARING PADS SHALL BE MADE OF 60 DUROMETER HARDNESS NEOPRENE,
- 4. 3" DIAMETER HOLE IN BEARING PADS MAY BE FORMED OR DRILLED.
- 5. BEARING PADS SHALL HAVE 1/4" COVER ON THE TOP, BOTTOM, AND SIDES
- 6. $\%_{6}$ " LOAD PLATES AND 14 GAGE INTERNAL PLATE(S) (IF REQUIRED) SHALL GRADE 36 OR ASTM A IOII GRADE 36.
- NUMBER OF INTERNAL PLATES SHOWN FOR ILLUSTRATION PURPOSES ONLY. PLATE(S) SPECIFIED SHALL BE EQUALLY SPACED BETWEEN LOAD PLATES. 8. USE OF 11/2° MOLD DRAFT IS OPTIONAL.

				BEAF	RING PAD	S
BENT						DESI
DLINI	W	L	Т	NUMBER OF INTERNAL PLATE(S)	DESIGN SHEAR DEFLECTION	DEAD LOA
1	16"	9"	23/4"	3	7⁄8″	43.7
2B	16"	9″	23/4"	3	5⁄8″	34.2
2A, 4B, 4A	20″	9″	23/4"	3	7∕16 "	78.6
3B & 3A	20″	9″	23/4"	3	0	78.6
5	20″	9″	23/4"	3	7⁄8″	88.5

GDOT	P.I. NO.
	0016985
RIDGE DESIGN SPECIFICATIONS, WITH AASHTO LRFD BRIDGE	
WITT ANOTIC ENTE DRIDGE	
ENE, GRADE 2 OR HIGHER.	
SIDES AND AROUND THE HOLE.	
HALL BE ASTM A 709	
ONLY. THE NUMBER OF INTERNAL	
ONLY. THE NUMBER OF INTERNAL ITES.	
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DESIGN LOADS (KIPS) LIVE LOAD DEAD LOAD (NO IMPACT) DEAD LOAD LIVE LOAD A1. LIVE LOAD DEAD LOAD LIVE LOAD A34.2 56.9 91.1	
DESIGN LOADS (KIPS) EAD LOAD LIVE LOAD EAD LOAD (NO IMPACT) 43.7 56.9 IOO.6 34.2 56.9 9I.I 78.6 76.I I54.7	
DESIGN LOADS (KIPS) DEAD LOAD DEAD LOAD LIVE LOAD DEAD LOAD 43.7 56.9 100.6 34.2 56.9 91.1 78.6 76.1 154.7	
DESIGN LOADS (KIPS) DEAD LOAD DEAD LOAD LIVE LOAD DEAD LOAD 43.7 56.9 100.6 34.2 56.9 91.1 78.6 76.1 154.7	
DESIGN LOADS (KIPS) LIVE LOAD (NO IMPACT) LIVE LOAD (NO IMPACT) LIVE LOAD (NO IMPACT) 100.6 34.2 56.9 91.1 78.6 76.1 154.7 88.5	
DESIGN LOADS (KIPS) EAD LOAD (NO IMPACT) DEAD LOAD LIVE LOAD LIVE LOAD 43.7 56.9 100.6 34.2 56.9 91.1 78.6 76.1 154.7 78.6 76.1 154.7	
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DESIGN LOADS (KIPS) EAD LOAD (NO IMPACT) EAD LOAD (NO IMPACT) LIVE LOAD 43.7 56.9 100.6 34.2 56.9 9I.1 78.6 76.1 154.7 78.6 76.1 154.7 88.5 76.1 164.6 BRIDGE NO.1 GEORGIA DEPARTMENT OF TRANSPORTAT ENGINEERING DIVISION-OFFICE OF BRIDGES AND STRUCT BEARING PAD DETAILS SR 25 (US 17) OVER THORNHILL CREEK	URES (
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DESIGN LOADS (KIPS) EAD LOAD (NO MPACT) EAD LOAD (NO MPACT) LIVE LOAD 43.7 56.9 100.6 34.2 56.9 9I.1 78.6 76.1 154.7 78.6 76.1 154.7 88.5 76.1 164.6 BRIDGE NO.1 GEORGIA DEPARTMENT OF TRANSPORTAT ENGINEERING DIVISION-OFFICE OF BRIDGES AND STRUCT BEARING PAD DETAILS SR 25 (US 17) OVER THORNHILL CREEK GLYNN COUNTY 0016	URES \$ 985 2017



BEAM ANGLE IS EXAGGERATED

- FORM 3"DIA. X 12"DEEP HOLE FOR DOWEL BAR, TYP.

- 2'-0" (TYP.)

TMB

610,

2'-9/16"

EL. 19.25-

3-411A

EQ. SPS

EL. 12.86

2'-8%6"

2-412B -

41

3"

,0-,6

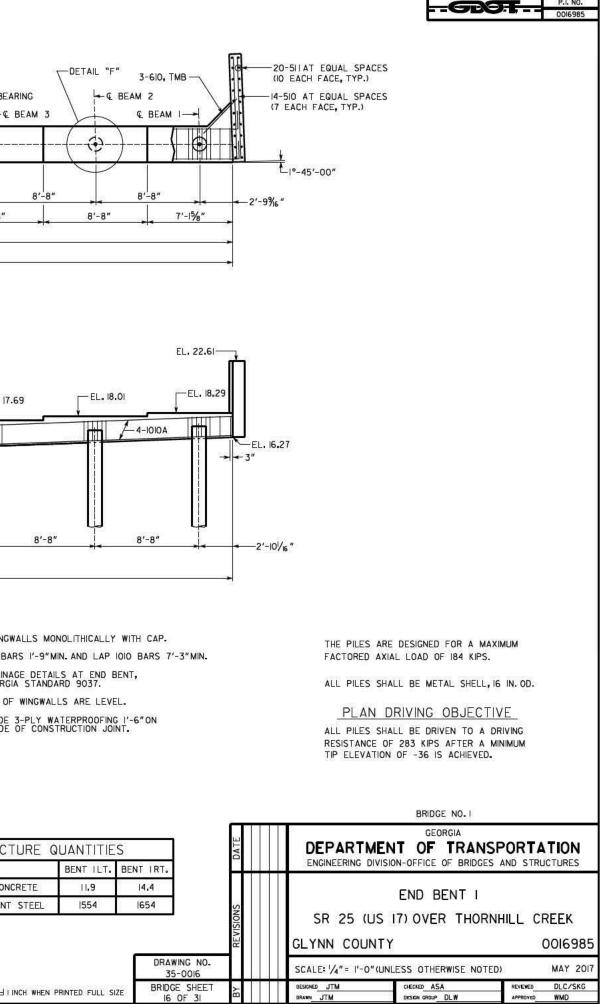
88°-00'-00"-

BEGIN BRIDGE B.F.P.R. = STA. 56I+74.00

88°-02'-08.1" TO TANGENT OF & CONST .-

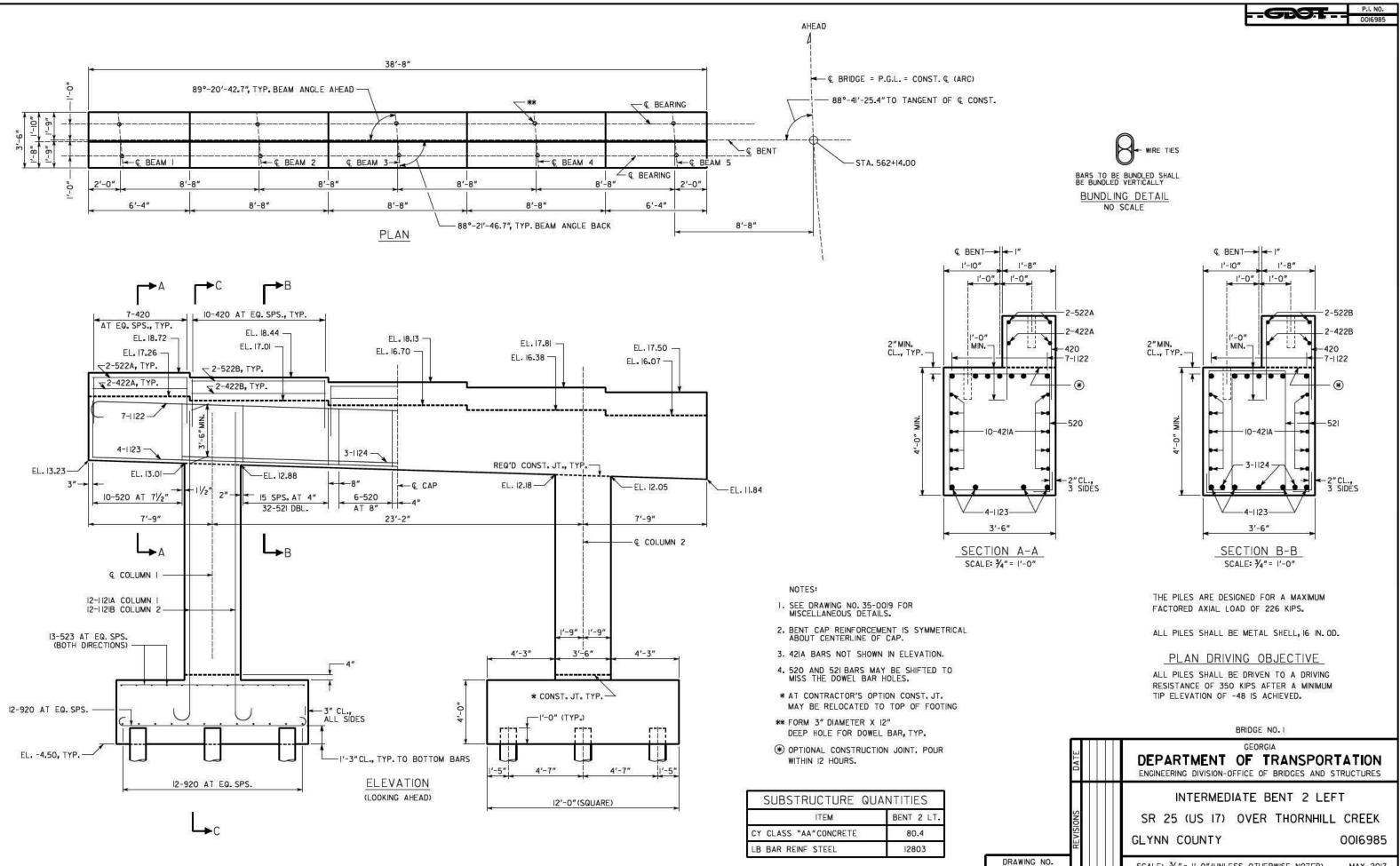
E BRIDGE = P.G.L. = CONST. & (ARC)

- Q REQ'D. CONST. JOINT

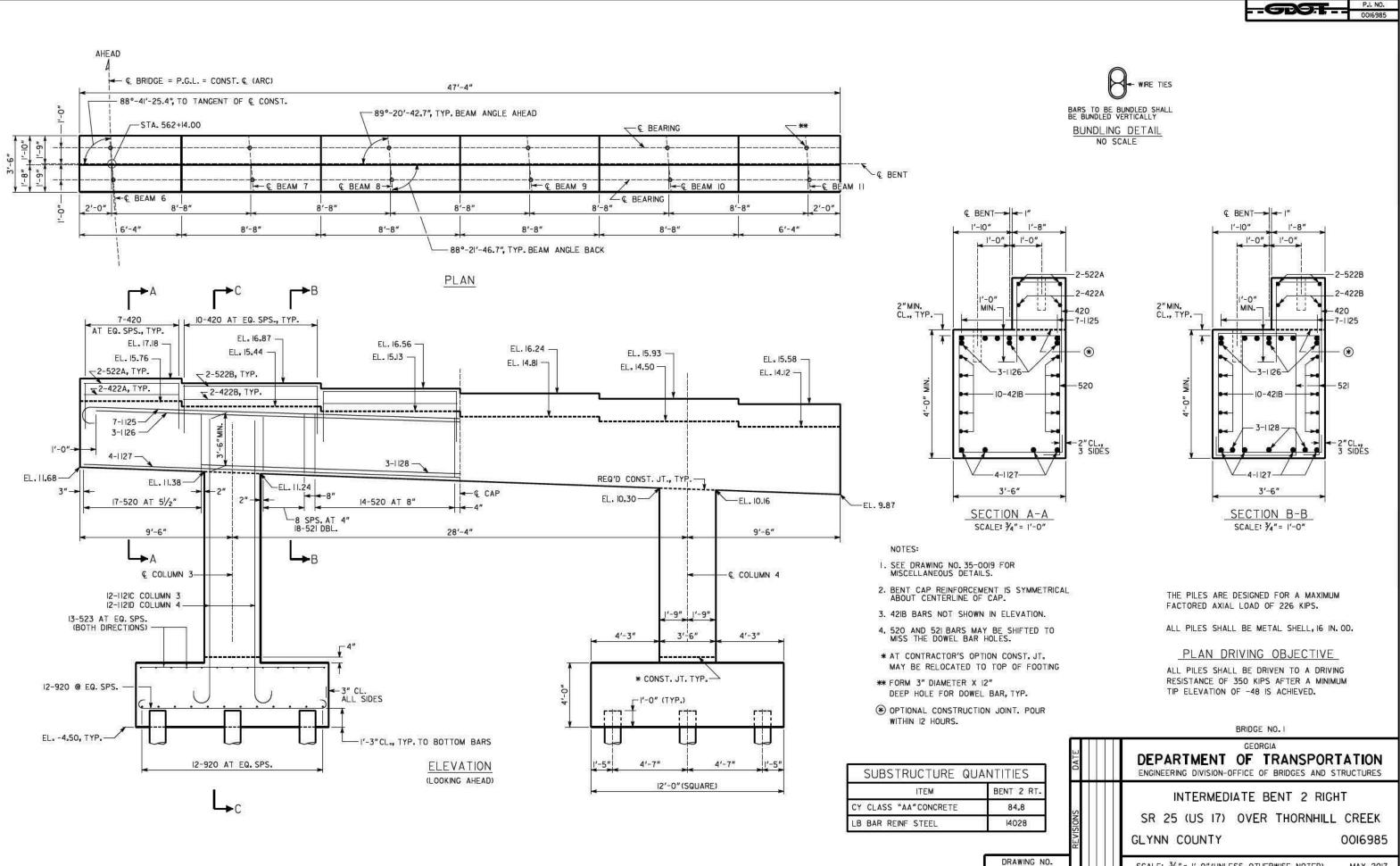


P.I. NO.

Lacated I INCH WHEN PRINTED FULL SIZE



	SCALE: 3/8" = 1'-	O" (UNLESS OTHERWISE NO)TED)	MAY 2017
>	DESIGNED JTM	CHECKED ASA	REVEWED	DLC/SKG
BY	DRAWN JTM	DESIGN GROUP DLW	APPROVED	WMD

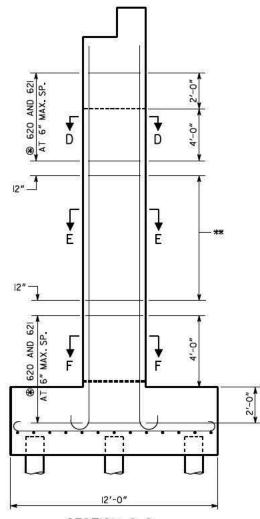


35-00 BRIDGE S 18 OF

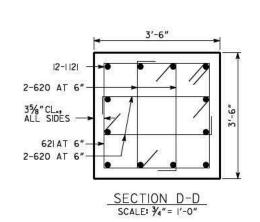
		BRIDGE NO. I	
DATE	요구가 있는 것 같은 것 같	GEORGIA	
T .		ERMEDIATE BENT 2 IS 17) OVER THOR JNTY	
NO- 018	SCALE: 3/8" = 1'	-O" (UNLESS OTHERWISE NO	DTED) MAY 2017
	DESIGNED JTM DRAWN JTM	CHECKED ASA DESIGN GROUP DLW	REVENED DLC/SKG

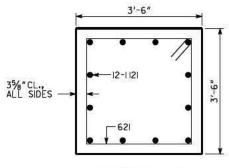
4-620 AT EACH LOCATION I-62I AT EACH LOCATION

** 5-62| AT EQ. SPS. (12" MAX.) COL. I 4-62| AT EQ. SPS. (12" MAX.) COL. 2 3-62| AT EQ. SPS. (12" MAX.) COL. 3 2-62| AT 12" MAX. SP. COL. 4

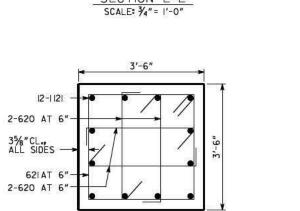


SECTION C-C NO SCALE

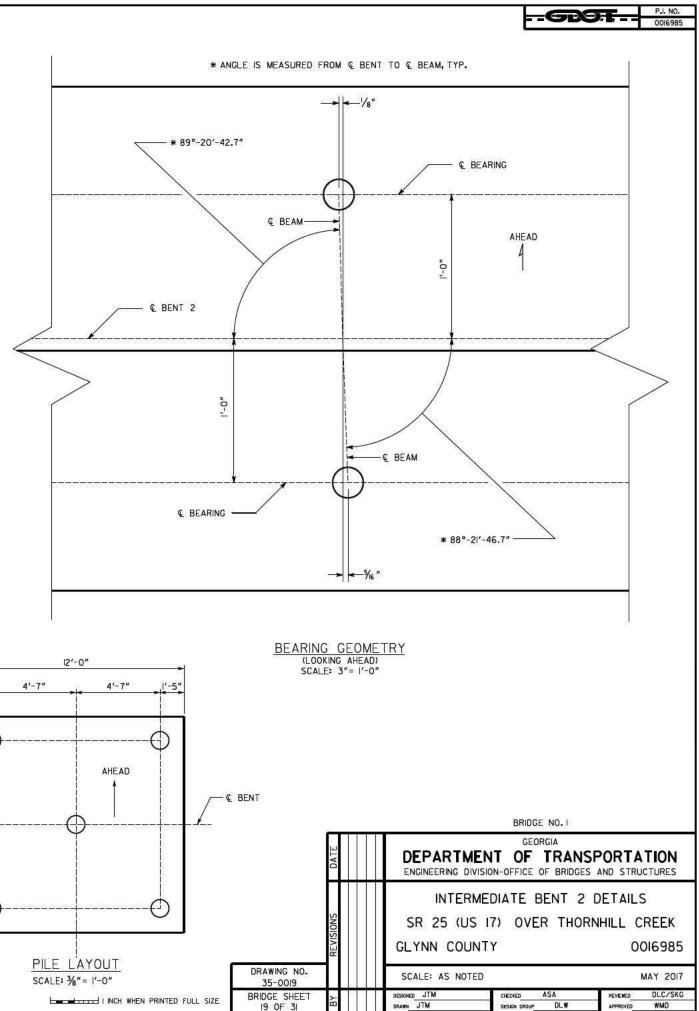


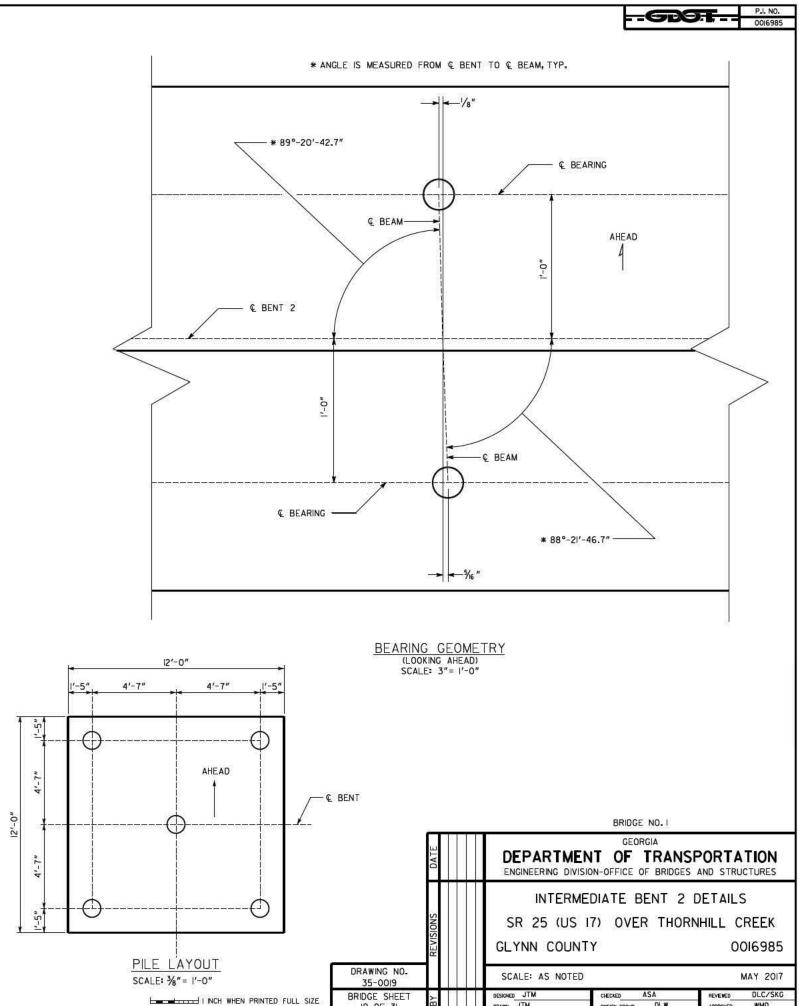


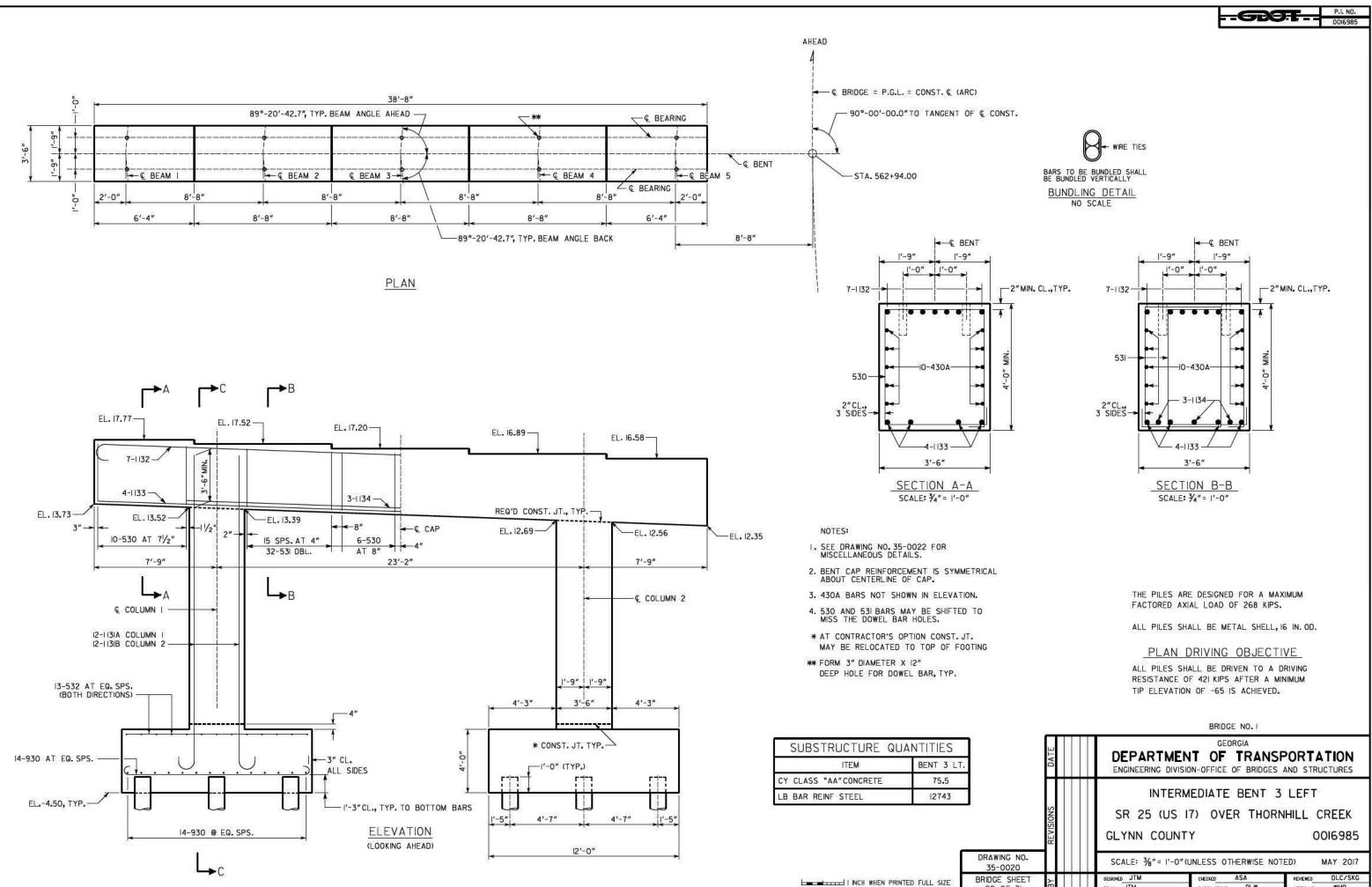
SECTION E-E scale: 3/4" = 1'-0"



SECTION F-F SCALE: 34" = 1'-0"

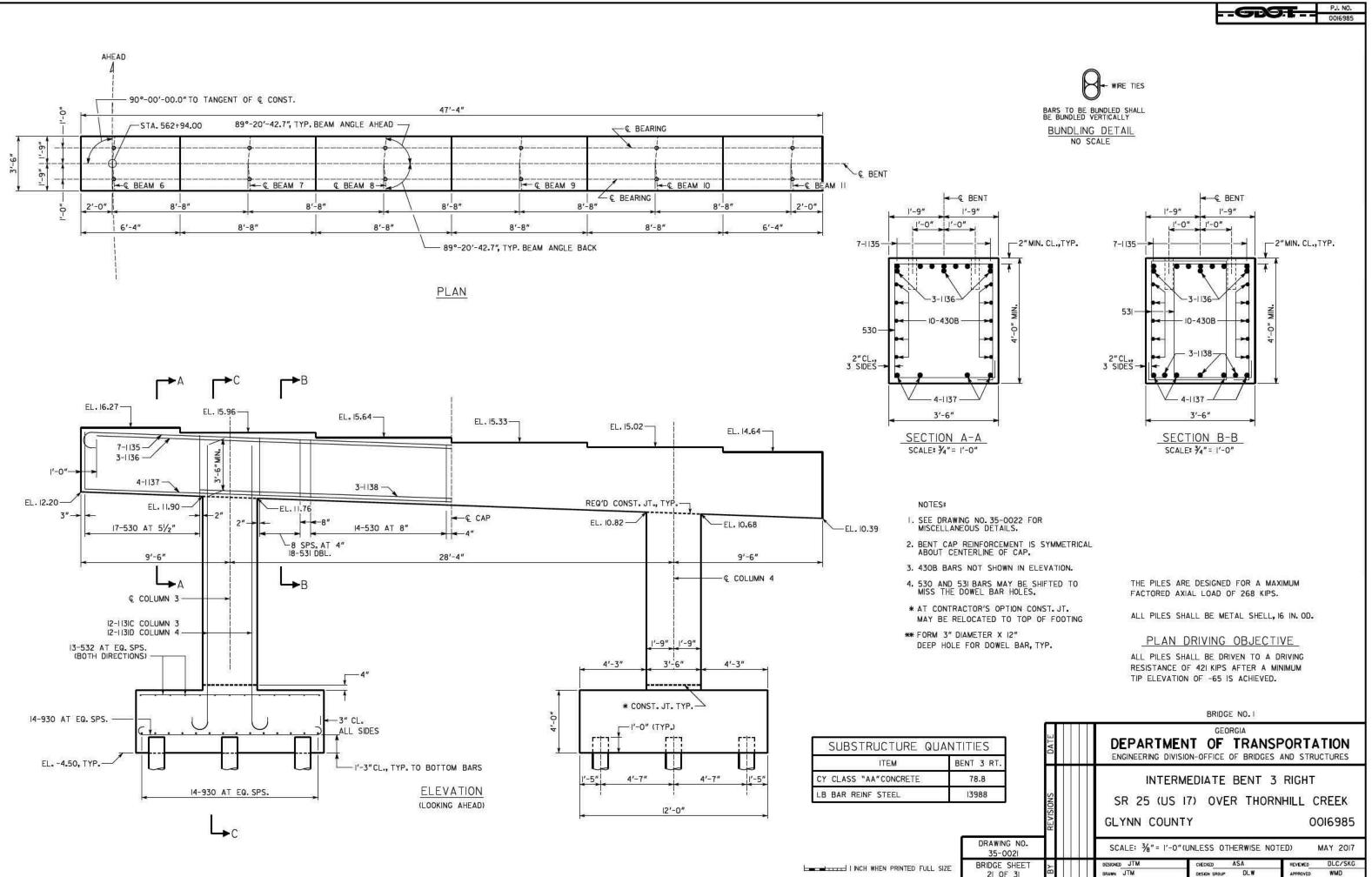


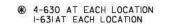




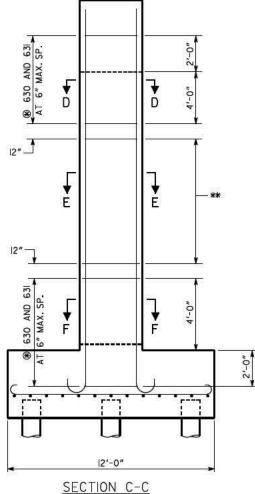
20 OF

100	BRIDGE NO. I	
DATE	CEORGIA DEPARTMENT OF TRANS ENGINEERING DIVISION-OFFICE OF BRIDGES	
REVISIONS	INTERMEDIATE BENT SR 25 (US 17) OVER THOP GLYNN COUNTY	
	SCALE: 3/8" = 1'-0" (UNLESS OTHERWISE NO	andradiwa musel koswani
BY	DESIGNED_JTMCHECKED_ASA DRAWN_JTMDESIGN_GROUP_DLW	REVEWED DLC/SKG APPROVED WMD

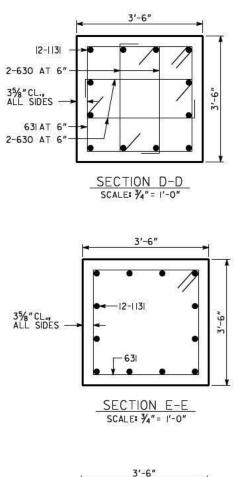


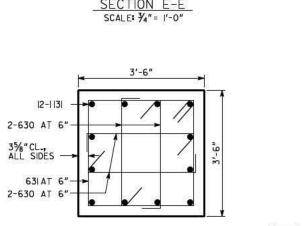


*** 5-631 AT EQ. SPS. (12" MAX.) COL. 1 5-631 AT EQ. SPS. (12" MAX.) COL. 2 4-631 AT EQ. SPS. (12" MAX.) COL. 3 3-631 AT 12" MAX. SP. COL. 4



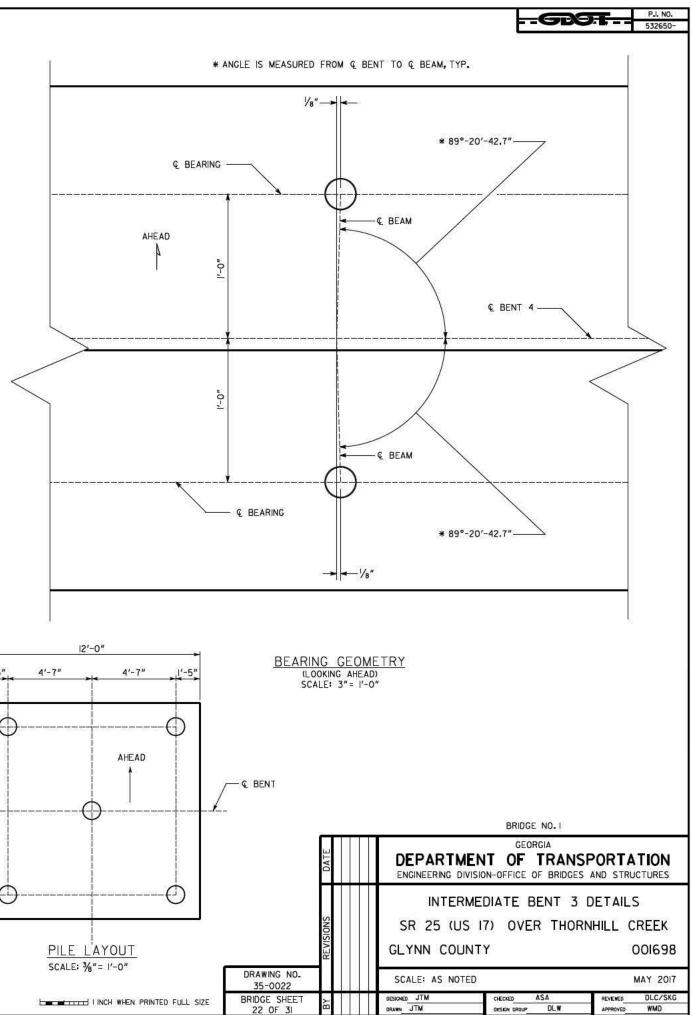
NO SCALE

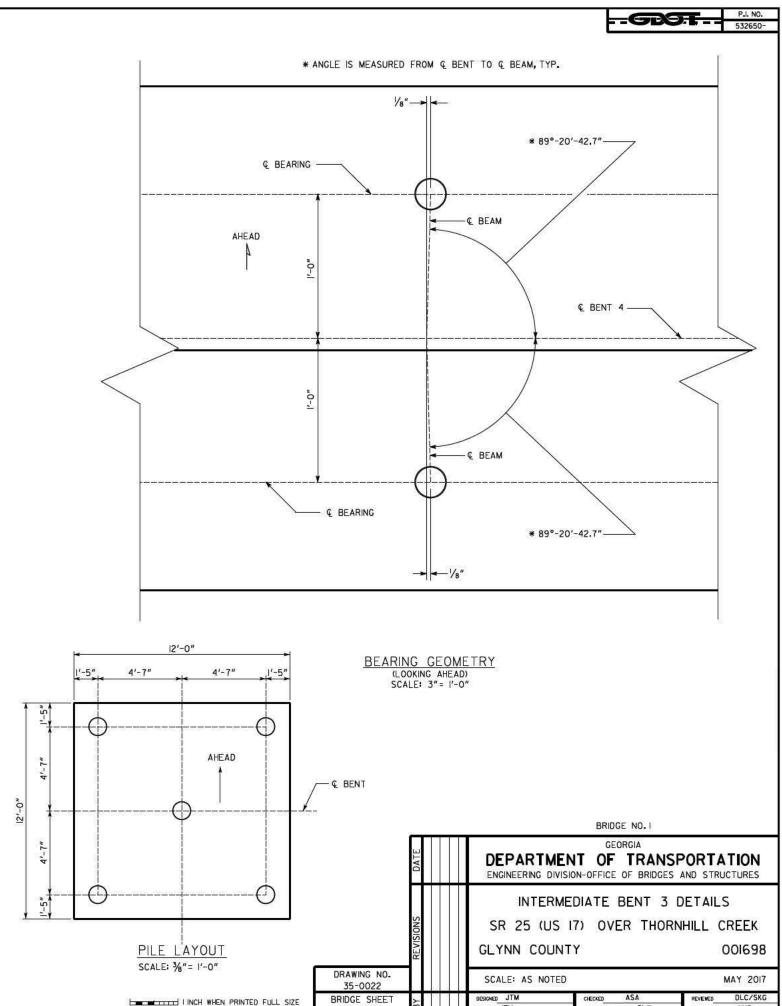


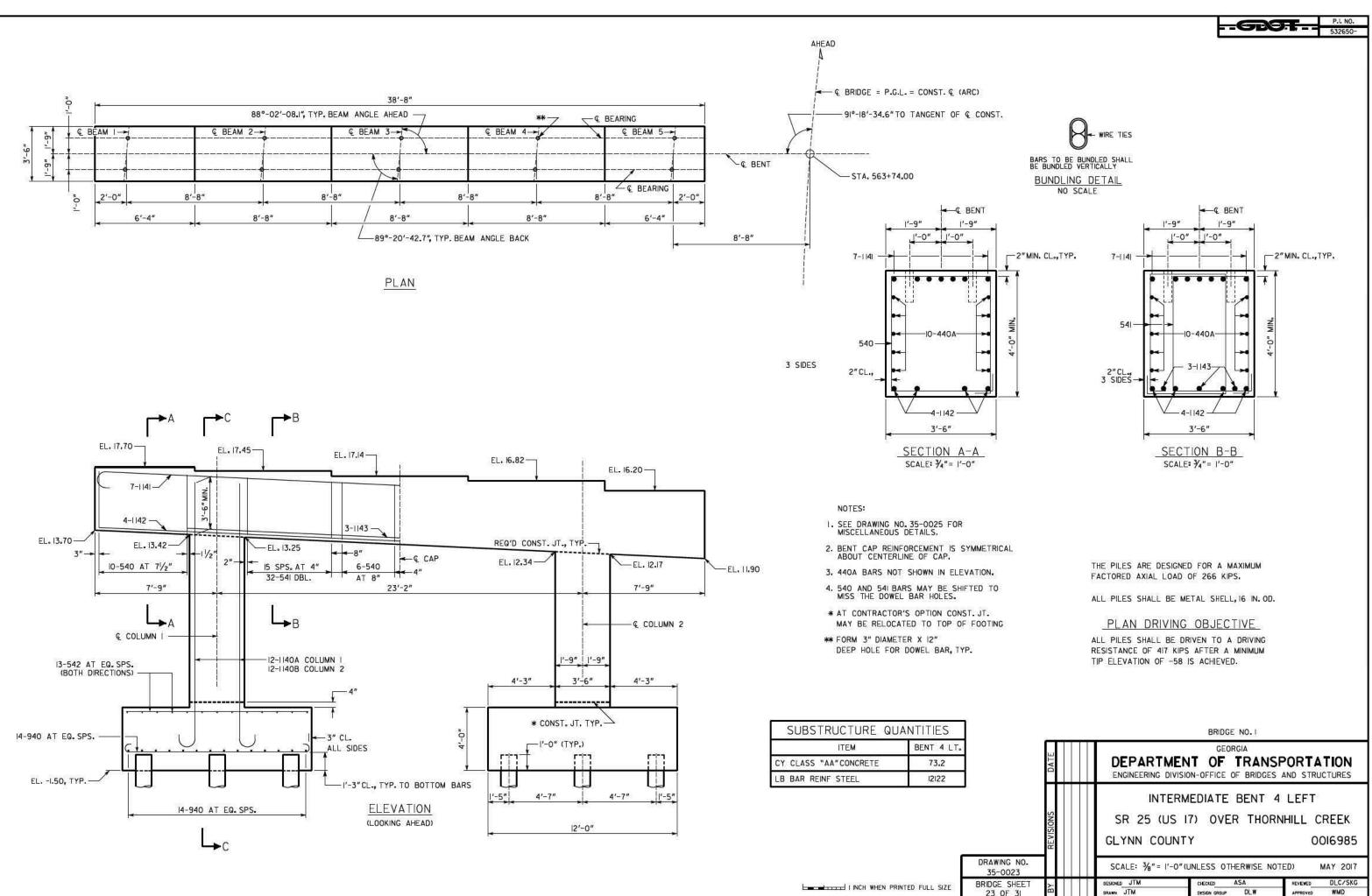


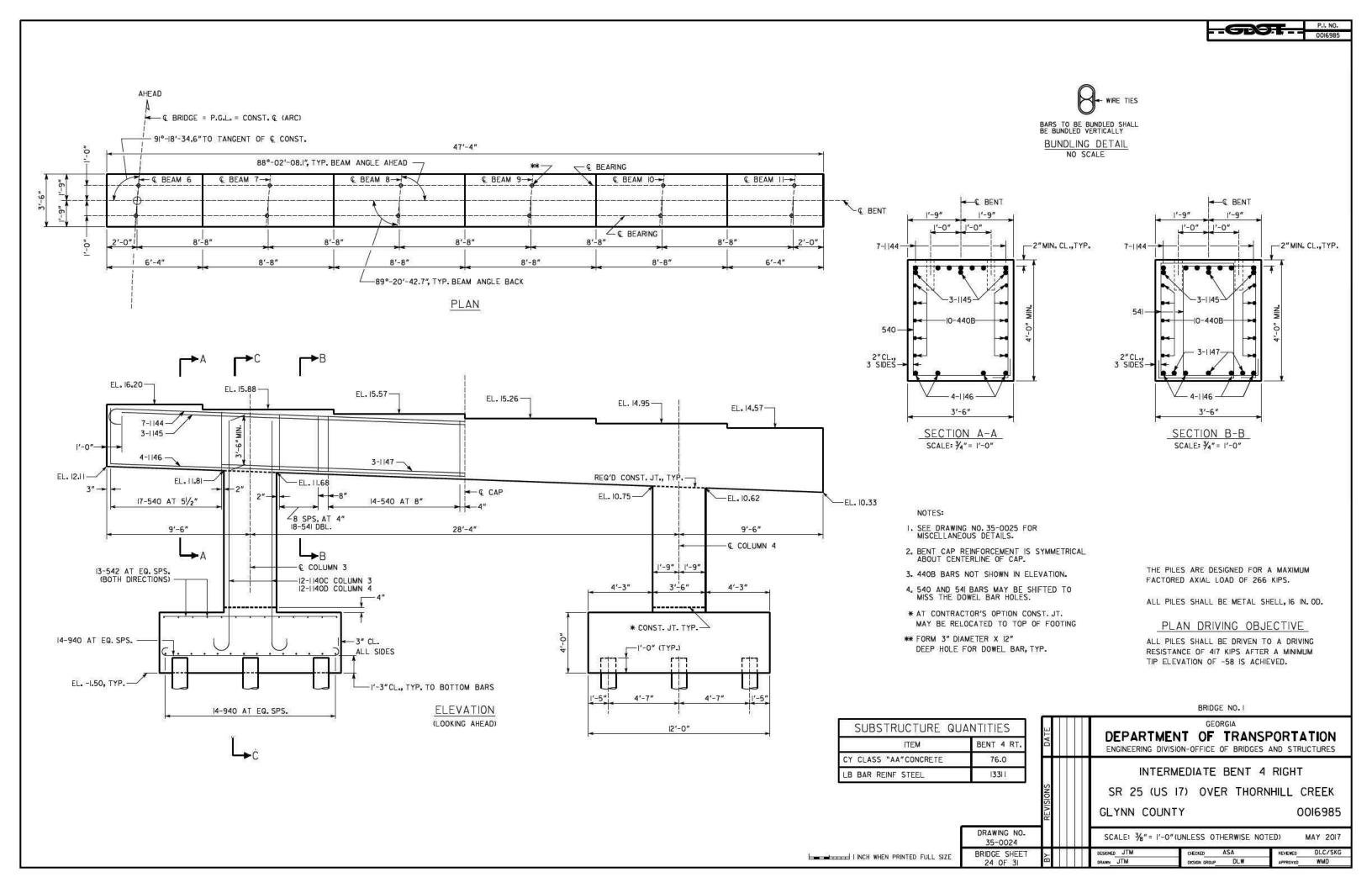
SECTION F-F

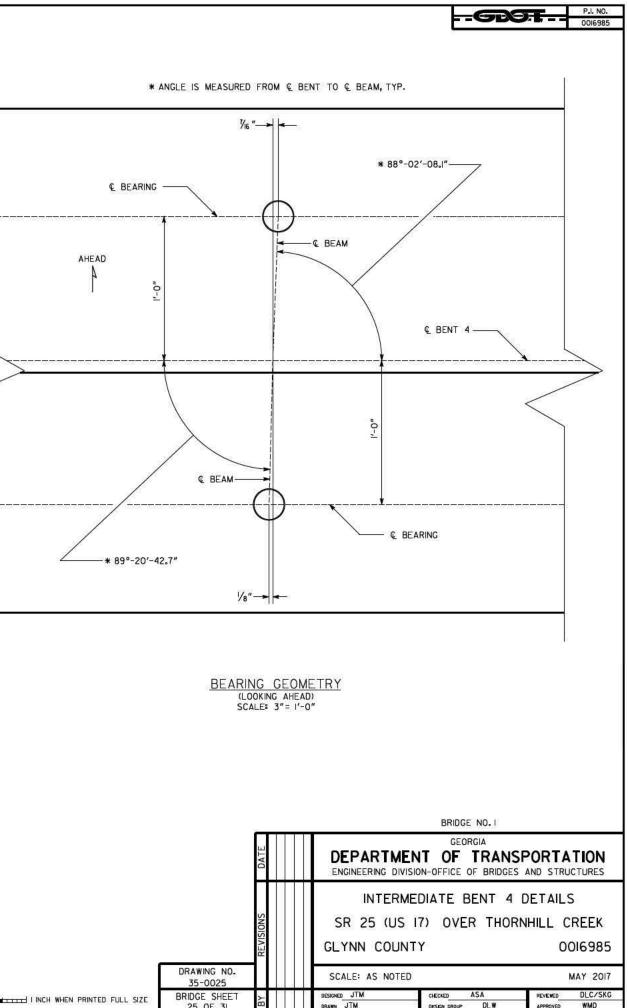
SCALE: 3/4" = 1'-0"





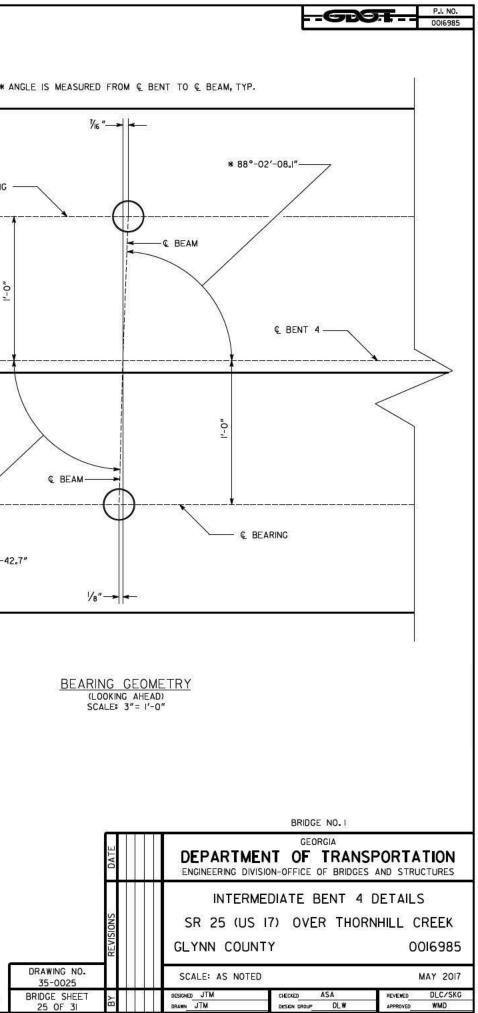












11'-6" SECTION C-C BENT 4 LEFT NO SCALE

❀ 4-640 AT EACH LOCATION I-64I AT EACH LOCATION

* 640 AND 641 AT 6" MAX. SP.

6″ _

6" —

640 AND 641 6" MAX. SP.

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* D

*** 3-641 AT EQ. SPS. (12"MAX.) COL. 1 2-641 AT 12"MAX. SP. COL. 2

"C

4'-0"

4'-0"

2'-0"

1

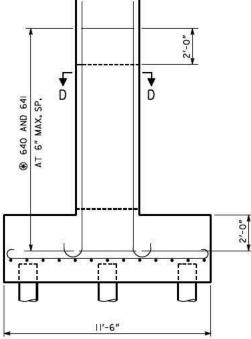
D

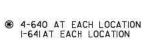
.

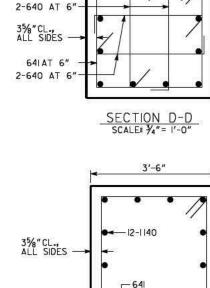
Ε

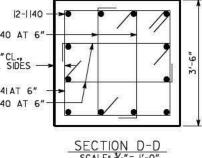
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SECTION C-C BENT 4 RIGHT NO SCALE









SECTION E-E SCALE: 3/4" = 1'-0"

12'-0"

 \oplus

PILE L'AYOUT

SCALE: 3/8" = 1'-0"

4'-7"

AHEAD

1'-5"

-)

Ð

-€ BENT

4'-7"

1-5"

(+

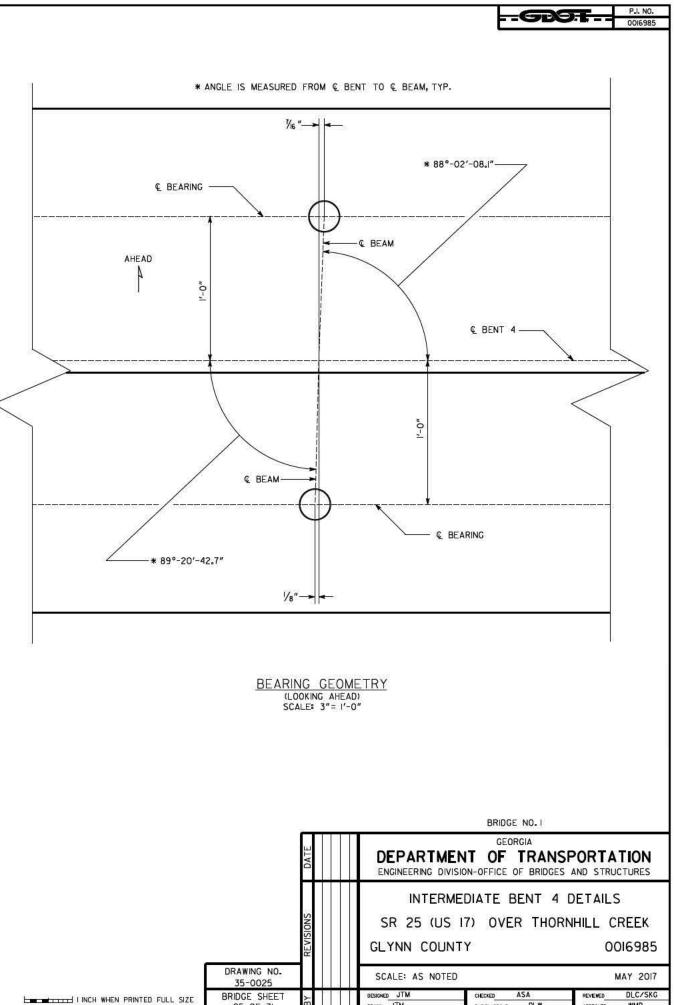
5-1

1

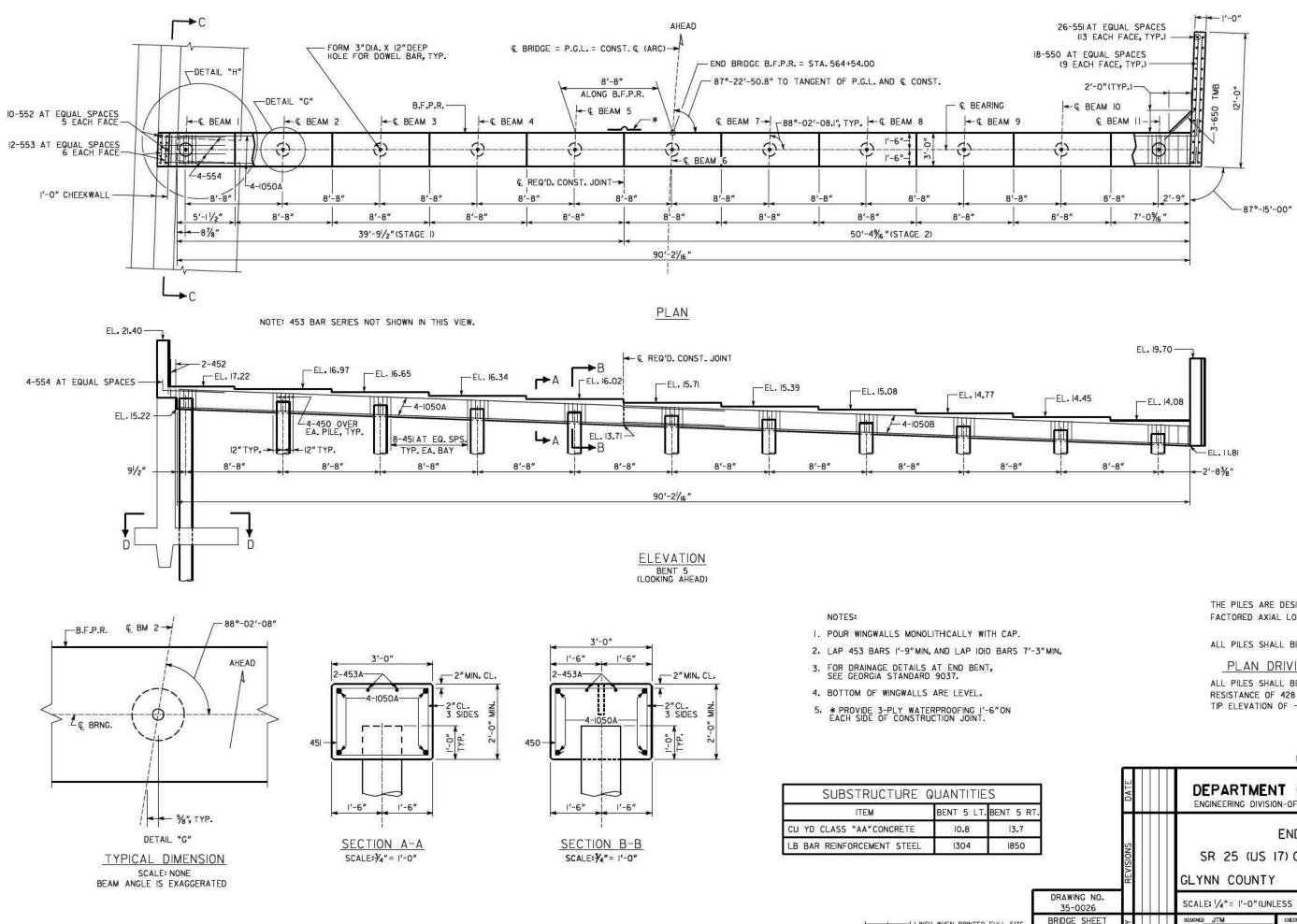
ì

2'-0"

3'-6"







26 OF

THE PILES ARE DESIGNED FOR A MAXIMUM FACTORED AXIAL LOAD OF 278 KIPS.

ALL PILES SHALL BE METAL SHELL, 16 IN. OD.

P.I. NO.

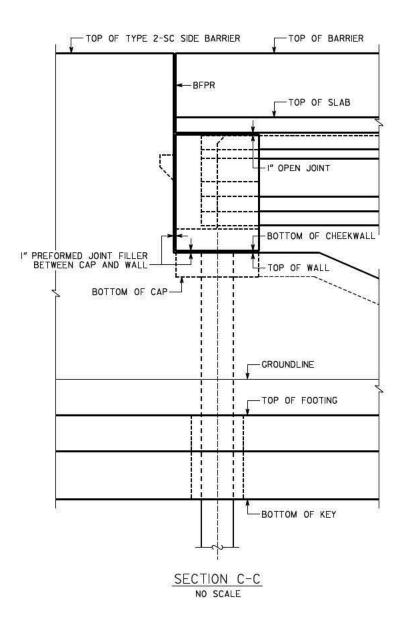
00/6985

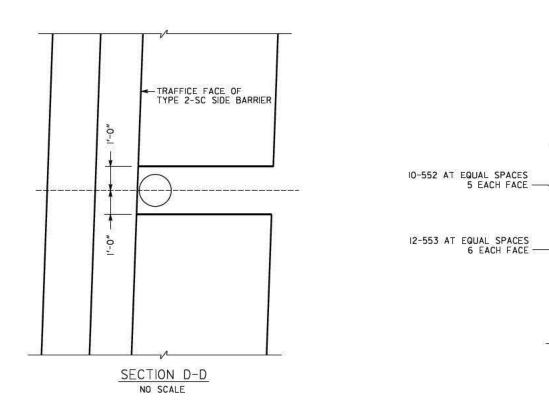
GROT

PLAN DRIVING OBJECTIVE

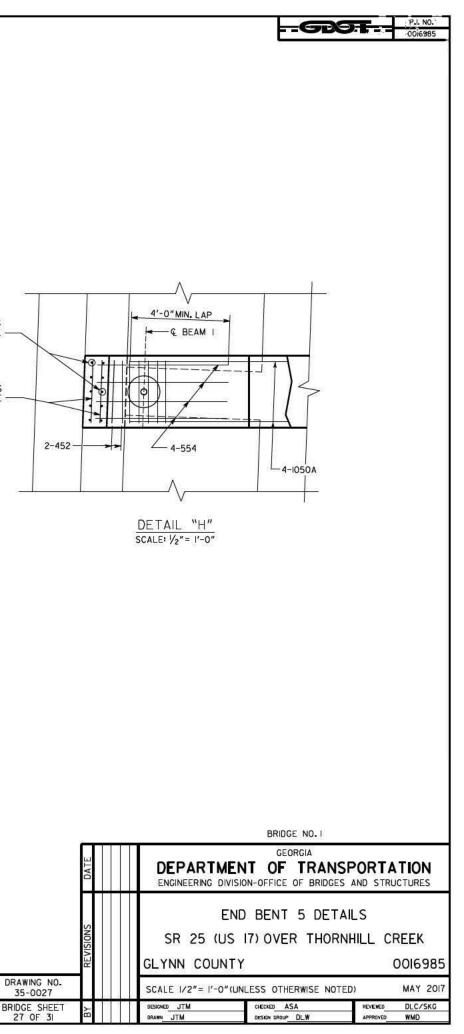
ALL PILES SHALL BE DRIVEN TO A DRIVING RESISTANCE OF 428 KIPS AFTER A MINIMUM TIP ELEVATION OF -54 IS ACHIEVED.

	BRIDGE NO. I
DATE	GEORGIA DEPARTMENT OF TRANSPORTATION ENGINEERING DIVISION-OFFICE OF BRIDGES AND STRUCTURES
REVISIONS	END BENT 5 SR 25 (US 17) OVER THORNHILL CREEK
REV	GLYNN COUNTY 0016985
NO. 6	SCALE: 1/4" = 1'-0" (UNLESS OTHERWISE NOTED) MAY 2017
EET 🚡	DESIGNED JTM CHECKED ASA REVEWED DLC/SKG DRAWN JTM DESIGN GROUP DLW APPROVED WMD



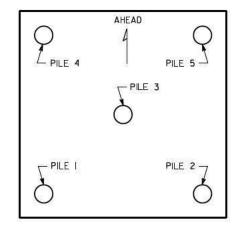


35-0027 BRIDGE SHEET 27 OF 31



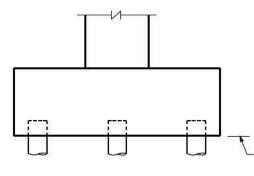
AS) B	Taxo Murtan Mat	ATION INFORM	ATION
BENT NUMBER		PILE	PILE TIP ELEVATION	ELEV. "/
HOMDEN		BEAM	LEETALINA	1
		BEAM 2		-
		BEAM 3		-
		BEAM 4		- $/$
		BEAM 5		$- \land /$
Ĕ		BEAM 6		- V
		BEAM 7		\neg
	-	BEAM 8		-/
		BEAM 9		- / `
		BEAM IO		-/
	-	BEAM II		-/
		PILE I		
	1	PILE 2		
	NWD	PILE 3		
	COLUMN	PILE 4		
	0	PILE 5		
2 LEFT		PILE I		-5
	N	PILE 2		-2
	NWI	PILE 3		-2
	COLUMN	PILE 4		
	0	PILE 5		
		PILE I		
	=	PILE 2		-2
	NWD	PILE 3		_2
	COLUMN	PILE 4		-2.
	0	PILE 5		
2 RIGHT		PILE		
	N	PILE 2		-2
	NWI	PILE 3		-2
	COLUMN	PILE 4		-2
	0	PILE 5		
	H	PILE I		
	-	PILE 2		5
	COLUMN	PILE 3		
	5	PILE 4		
C1453903949935		PILE 5		
3 LEFT		PILE I		
	2	PILE 2		- 5
	NWC	PILE 3		-2
	COLUMN 2	PILE 4		
	0	PILE 5		
		PILE I		
		PILE 2		- 57
	NMU	PILE 3		-2
	COLUMN	PILE 4		
()		PILE 5		-2
3 RIGHT	H	PILE		
	~	PILE 2		
	COLUMN	PILE 3		-5
	OLL	PILE 4		172
	111			

AS		ATION INFORM	TION
BENT NUMBER	PILE LOCATION	PILE TIP ELEVATION	ELEV. "A"
4 LEFT	PILE I PILE 2 PILE 3 PILE 3 PILE 4 PILE 5 PILE I		
	PILE 2 PILE 3 PILE 4 PILE 5 PILE 1		
4 RIGHT	PILE 2 PILE 3 PILE 4 PILE 5 PILE 1		
4 RIGHT	N PILE 2 PILE 3 PILE 4 PILE 5		
5	BEAM I BEAM 2 BEAM 3 BEAM 4 BEAM 5 BEAM 6 BEAM 7 BEAM 8		
	BEAM 9 BEAM IO BEAM II		



PLAN

INTERMEDIATE BENT FOOTING



ELEVATION

INTERMEDIATE BENT FOOTING

DRAWING N 35-0028 BRIDGE SHE 28 OF 31

						Gi		P.I. NO. 0016985
						-		
A =	BO	ΤT	OM	OF	FOOTING			
					NOTE - THIS "AS-BUILT S TO BE FILLED IN BY	FOUNDATION INFOR THE PROJECT ENG	RMATION" SHE	ET THE
				E (F	NTIRE SHEET FORWARD COMPLETION OF PILE DR FOR POSTING TO THE P	D TO THE BRIDGE VING AND FOOTING ANS AS A PERM	OFFICE UP CONSTRUCT	ION RD
				(OF THE BRIDGE CONSTRU	ICTION.		
				3	PROJECT ENGINE	ER	DATE	
				2	() (AREA CODE)	TELEPHONE NUME	R	
						BRIDGE NO. I		
	ΤE				DEPARTMEN			
	DAT				ENGINEERING DIVISION			
	340				AS BUILT	FOUNDATION	INFORMA	TION
	REVISIONS				SR 25 (US	7) OVER TH	ORNHILL	CREEK
	REVIS				GLYNN COUNTY			0016985
10. 3	5 8 - 8				NO SCALE			MAY 2017
ET I	ВΥ				designed JTM drawn JTM	CHECKED ASA Design group DLW	REVENED	DLC/SKG WMD

LOCATION	NO.		LENGTH	NO.	T,		В	С	D	E	F	Н	J	K			LOCATION	NO.	LENGTH	NO.	T _y		В	С	D	E	E	Н	J	κ
LOCATION	OF I	MARK	FT. IN.	BARS	. P	AG	FT. IN.	FT. IN.	FT. IN.	FT. IN.	FT. IN.	FT. IN.	FT. IN.	FT.IN.	N	θ	LOCATION	OF MARK	FT. IN	BARS REQ'D.	PE	AG	FT. IN.	FT. JN.	FT. IN.	FT. IN.	FT. IN.	FT. IN.	FT. IN. FT	. IN.
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	22 34	3		4	12	88-0					12	2		g 3		~		- C.T.				4 4	0-7	2-9		2 3				_
			39-8			-										-		407	7- 9	9 16	<u> </u>									_
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		500A	40-8	79	3	1038-0	39-10	0-10			17. 19		1						40- (20 0				й				
		500C	6-2	79	3		5-4	0-10										502C	19- 1	7 48	1 E									
		501 A	40- 0	58	1)	503A		92										
		502A	18- 0		1					-	2							503B							-					
		503A	8-11		23		1-3	3-10	3-10		14. 14.	2	8	a		96		24 T 2 C 4 C 2 C 4 C 2 C 4 C 2 C 4 C 2 C 4 C 4	38- 2						-	1. 1.				
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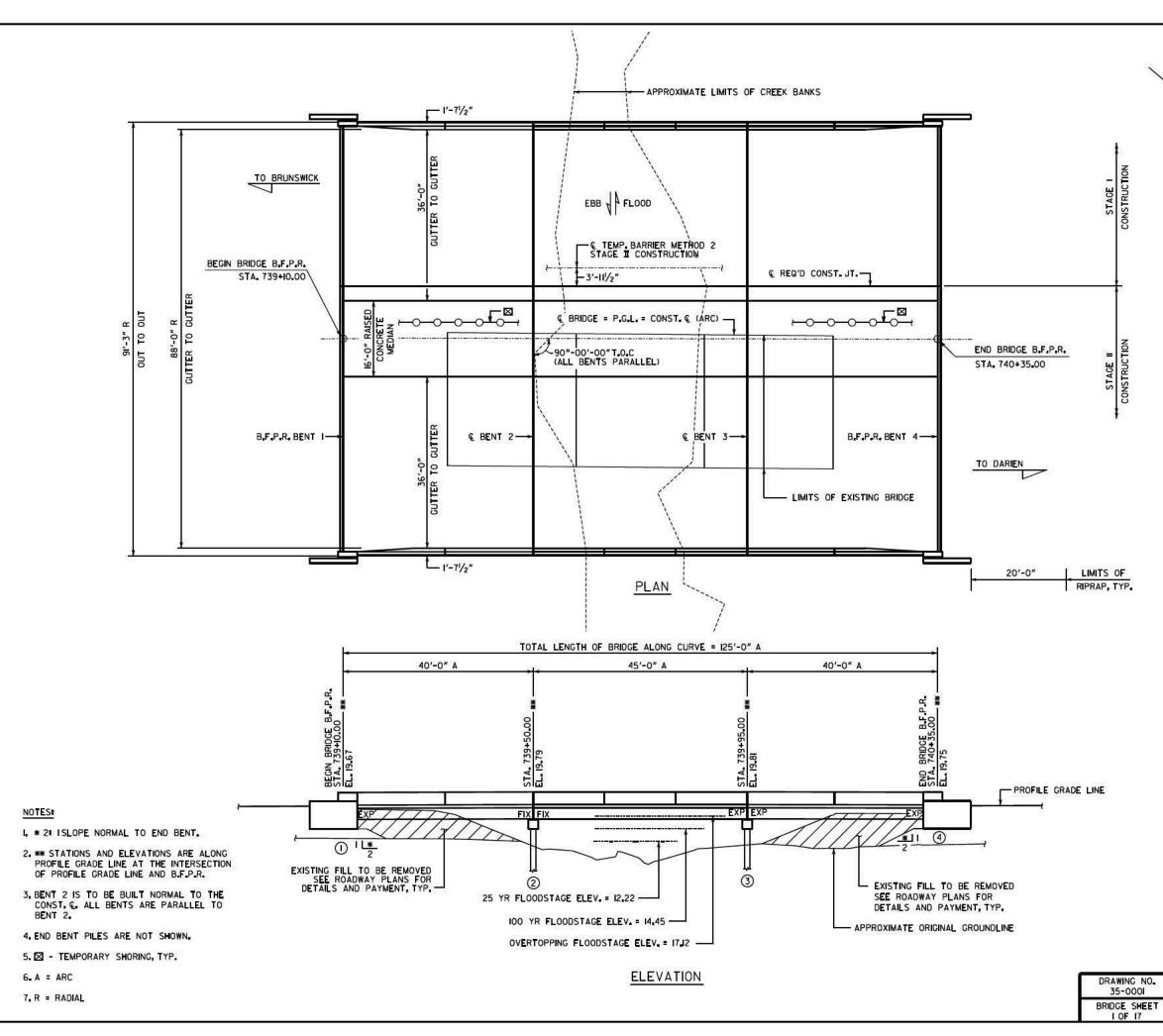
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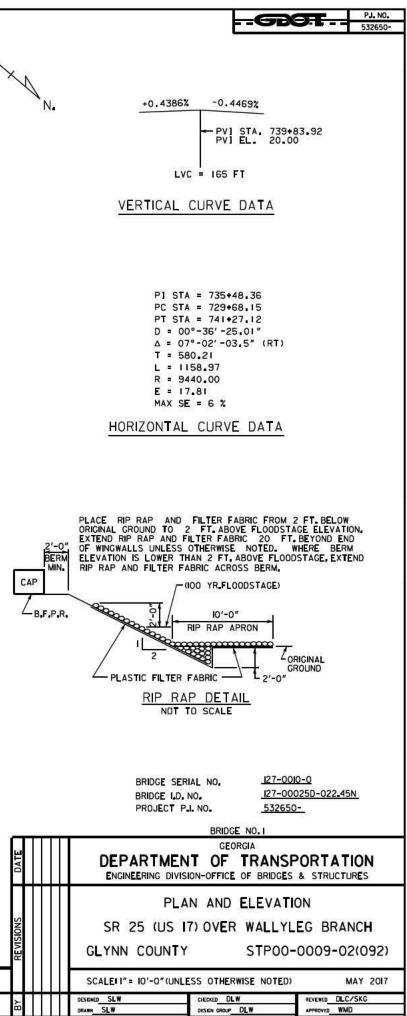
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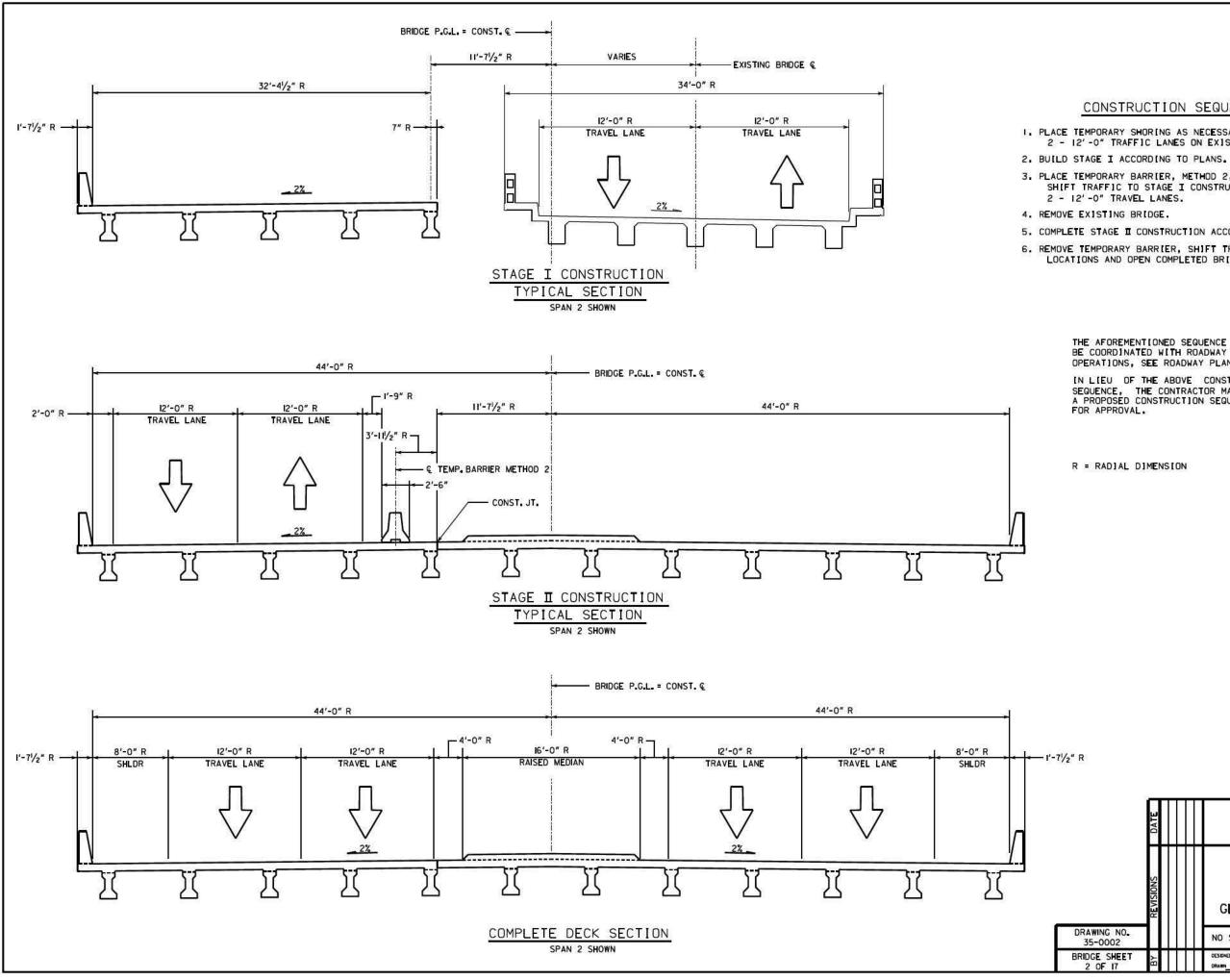
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			BAR RE	INFORCEMENT DE	TAILS
		REVISIONS	SR 25 (US I	7) OVER THORN	HILL CREEK
_		REV	GLYNN COUNTY		0016985
NO. 31			NO SCALE	- 646899 - 1126-	MAY 2017
HEET 3I	Γ	ВΥ	designed JTM drawn JTM	CHECKED ASA DESIGN GROUP DLW	REVEWED DLC/SKG APPROVED WMD







	P. NO.
F-GLO-F-7	532650-

CONSTRUCTION SEQUENCE

I. PLACE TEMPORARY SHORING AS NECESSARY. MAINTAIN 2 - 12'-0" TRAFFIC LANES ON EXISTING BRIDGE.

3. PLACE TEMPORARY BARRIER, METHOD 2, ACCORDING TO PLANS. SHIFT TRAFFIC TO STAGE I CONSTRUCTION, MAINTAINING

5. COMPLETE STAGE I CONSTRUCTION ACCORDING TO PLANS.

6. REMOVE TEMPORARY BARRIER, SHIFT TRAFFIC TO PERMANENT LOCATIONS AND OPEN COMPLETED BRIDGE TO TRAFFIC.

THE AFOREMENTIONED SEQUENCE SHALL BE COORDINATED WITH ROADWAY OPERATIONS, SEE ROADWAY PLANS.

IN LIEU OF THE ABOVE CONSTRUCTION SEQUENCE, THE CONTRACTOR MAY SUBMIT A PROPOSED CONSTRUCTION SEQUENCE

1	BRIDGE NO. I
DATE	GEORGIA DEPARTMENT OF TRANSPORTATION ENGINEERING DIVISION-OFFICE OF BRIDGES & STRUCTURES
REVISIONS	CONSTRUCTION STAGING SR 25 (US 17) OVER WALLYLEG BRANCH GLYNN COUNTY STP00-0009-02(092)
- 4/104 - 2	NO SCALE MAY 2017
BΥ	DESIGNED SLW CHECKED DLW REVEWED DLC/SKG DRAWN SLW DESIGN GROUP DLW APPROVED WMD

BRIDGE CONSISTS OF

- 2 40'-0" TYPE I MOD PSC BEAM SPANS ------ SPECIAL DESIGN
- I 45'-0" TYPE I MOD PSC BEAM SPAN ------ SPECIAL DESIGN
- 2 PSC PILE END BENTS ------ SPECIAL DESIGN
- 2 PSC PILE INTERMEDIATE BENTS ------ SPECIAL DESIGN
- 4 END POST AND GUARDRAIL ATTACHMENT DETAIL ---- GA, STD, 3054 (9-30-02)
- (L = 4' 0''; W = 1' 1''; H = 3' 6'')
- BAR BENDING DETAILS ----- GA. STD. 3901 (8-69)
- CONCRETE BARRIERS TEMP. ------ GA. STD. 4960 (5-10-07)
- TYPICAL FILL DETAIL AT END OF BRIDGE ----- GA. STD. 9037 (9-99)
- SQUARE PRESTRESSED CONCRETE PILES ----- GA. STD. 3215 (2-22-84)

DRAINAGE DATA

DRAINAGE AREA				I.5 SQ MILES
FLOOD FREQUENCY	TOTAL DISCHARGE		AREA OF OPENING UNDER FLOODSTAGE	BACKWATER
25 YEAR	314 CFS	I.95 FPS	161 SQ FT	0.18 FT
100 YEAR	1,099 CFS	2.68 FPS	410 SQ FT	N/A FT
OVER TOPPING STORM	3,594 CFS	5.52 FPS	651 SQ FT	N/A FT

TRAFFIC DATA

TRAFF[C	ADT	=	7,350	(2022)
	ADT	=	8,250	(2042)

DESIGN SPEED	55	, MF	۶H
TRUCKS		2	%
24 HR TRUCKS	3	.5	%
D[RECT]ONAL	00000	57	%

EXISTING UTILITIES

NO UTILITIES ON BRIDGE

GENERAL NOTES

- SPECIFICATIONS GEORGIA STANDARD SPECIFICATIONS, 2013 EDITION, AND 2016 SUPPLEMENTAL SPECIFICATIONS AS MODIFIED BY CONTRACT DOCUMENTS.
- REINFORCING STEEL PLACE AND TIE ALL REINFORCING STEEL IN ACCORDANCE WITH THE GEORGIA DOT SPECIFICATIONS, DO NOT WELD REINFORCING STEEL, MAINTAIN 2" MINIMUM CLEARANCE ON ALL REINFORCEMENT UNLESS OTHERWISE NOTED.

CHAMFER - CHAMFER ALL EXPOSED CONCRETE EDGES 3/4" UNLESS OTHERWISE NOTED.

- TEMPORARY BARRIERS, METHOD 2 PLACE TEMPORARY BARRIERS AS SHOWN ON THE PLANS AND GEORGIA STANDARD NO. 4960 TO PROVIDE FOR 2 - 12'-0" TRAFFIC LANES. SUPPLY AND USE THE BARRIER IN ACCORDANCE WITH SECTION 620 OF THE GEORGIA DOT SPECIFICATIONS.
- TRAFFIC CONTROLS SEE ROADWAY PLANS FOR TRAFFIC CONTROLS AND TRAFFIC CONTROL PAYMENT.

EXISTING BRIDGE PLANS -	ORIGINAL E	BRIDGE	PLANS I	MAY E	BE OBTAINED	ON	THE GEORGIA DOT
DOT WEBSITE AT:							

SPECIFICATIONS HTTP://WWW.DOT.GA.GOV/BS/PROJECTS/PROJECTSEARCH THE ORIGINAL BRIDGE WAS BUILT UNDER PROJECT NUMBER B.A. (2)1791-A(15) (PROJECT 1D DESIGN VEHICLE ND. H007492). EPOXY RESIN ADHESIVE - APPLY EPOXY RESIN ADHESIVE TYPE IT TO ALL HARDENED CONCRETE FUTURE PAVING A SURFACES JUST PRIOR TO POURING THE CONCRETE FOR THE NEXT STAGE OF CONSTRUCTION, SEE SECTION 886 OF THE GEORGIA DOT SPECIFICATIONS, INCLUDE THE CONCRETE: SUPER COST OF EPDXY ADHESIVE AND ITS APPLICATION IN THE OVERALL BID SUBMITTED. BARR PSC B WAITING PERIOD - NONE REQUIRED. PSC B PLAN DRIVING OBJECTIVE - SEE SUBSTRUCTURE DETAILS. SUBST DRIVING RESISTANCE - DETERMINE DRIVING RESISTANCE FOR PILES USING DYNAMIC PILE REINFORCEMENT S TESTING IN ACCORDANCE WITH SPECIAL PROVISION 520. DYNAMIC PILE TESTING SHALL BE REQUIRED FOR ONE PILE AT EACH OF BENT I LEFT AND BENT 3 RIGHT. PRETENSIONING S DYNAMIC PILE TESTING - PERFORM PILE TESTING USING THE PILE DRIVING ANALYZER (PDA) IN ACCORDANCE WITH SPECIAL PROVISION SECTION 523. NOTIFY THE GEOTECHNICAL BUREAU OF THE GEORGIA DOT OFFICE OF MATERIALS AND TESTING AT 404-608-4720 TWO WEEKS PRIOR TO DRIVING PILES. WAVE EQUATION - PERFORM WAVE EQUATION ANALYSIS (WEAP) IN ACCORDANCE WITH SPECIAL PAY ITEM PROVISION 520, PROVIDE RESULTS OF THE WEAP TO THE GEOTECHNICAL BUREAU OF THE NUMBER QUANT GEORGIA DOT OFFICE OF MATERIALS AND TESTING FOR REVIEW AND APPROVAL TWO WEEKS PRIOR TO DRIVING PILES. 500-0100 PILING - JETTING OR SPUDDING OF PSC PILING MAY BE NECESSARY AT THIS SITE TO ACHIEVE THE INDICATED PLAN DRIVING OBJECTIVE. 500-1011 TEST PILES - DRIVE TEST PILES AT THE FOLLOWING LOCATIONS: 500-2100 ONE 16 IN SQ PSC X 40 FT AT BENT I LEFT ONE 14 IN SQ PSC X 40 FT AT BENT 3 RIGHT 500-3101 HIGH PERFORMANCE CONCRETE (HPC) - PRESTRESSED CONCRETE PILES FOR THIS BRIDGE UTILIZE HIGH PERFORMANCE CONCRETE, SPECIAL REQUIREMENTS ARE REQUIRED AS 507-8900 1 DETAILED IN SPECIAL PROVISIONS SECTIONS 500 AND 865. HPC PSC PILES WILL BE PAID FOR AS "PILING, PSC". 511-1000 12 PRESTRESSED CONCRETE PILES - ALL PRESTRESSED CONCRETE (PSC) PILES UTILIZED AS 511-3000 L PERMANENT PILES FOR THIS PROJECT SHALL BE REINFORCED WITH STAINLESS STEEL WIRE STRAND, STAINLESS STEEL WIRE AND STAINLESS STEEL BAR REINFORCEMENT IN 520-2414 ACCORDANCE WITH SPECIAL PROVISION 853 - "REINFORCEMENT AND TENSIONING STEEL" AND AS SHOWN IN THE PLANS. 520-2416 SMOOTH DOWEL BARS - PLACE SMOOTH DOWEL BARS IN FORMED 3" DIAMETER X 12" DEEP HOLES AND GROUT IN PLACE SIMILAR TO ANCHOR BOLTS, SEE SUB-SECTION 520-3414 501.3.05.B.3 OF THE GEORGIA DOT SPECIFICATIONS. STIRRUPS MAY BE SHIFTED SLIGHTLY TO CLEAR FORMED HOLES. 520-3416 GROOVED CONCRETE - GROOVE THE ENTIRE LENGTH OF THE BRIDGE TRANSVERSELY AS PER 523-1100 SUB-SECTION 500.3.05.T.9.C OF THE GEORGIA DOT SPECIFICATIONS. DO NOT GROOVE UNDER RAISED MEDIAN. 540-1101 EXTERIOR BEAM BRACING - THE CONTRACTOR SHALL PROVIDE BRACING BETWEEN EXTERIOR BEAM AND THE FIRST INTERIOR BEAM UNTIL THE DECK HAS BEEN POURED AND THE 603-2024 OVERHANG FORMS REMOVED FOR SPANS I AND 3, ALL COST FOR DES[GN[NG, PROV]D[NG, 603-7000 INSTALLING AND REMOVING BRACING SHALL BE INCLUDED IN PRICE BID FOR LUMP -SUPERSTRUCTURE CONCRETE. 620-0200 WELDING - ALL WELDING ON GEORGIA DOT PROJECTS SHALL BE PERFORMED BY CERTIFIED WELDERS THAT HAVE IN THEIR POSSESSION A CURRENT WELDING CERTIFICATION CARD [SSUED BY THE OFFICE OF MATERIALS AND TESTING, USE ONLY E70XX (EXCLUDING E7014 AND E7024) LOW HYDROGEN ELECTRODES FOR MANUAL SHIELDED METAL ARC WELDING. BRIDGE REMOVAL - REMOVE EXISTING BRIDGE AS PER SUB-SECTION 540.3.05 OF THE GEORGIA DOT SPECIFICATIONS.

SALVAGE MATERIAL - NO MATERIAL REMOVED FROM THE EXISTING STRUCTURE SHALL BE SALVAGED FOR USE BY THE GEORGIA DOT.

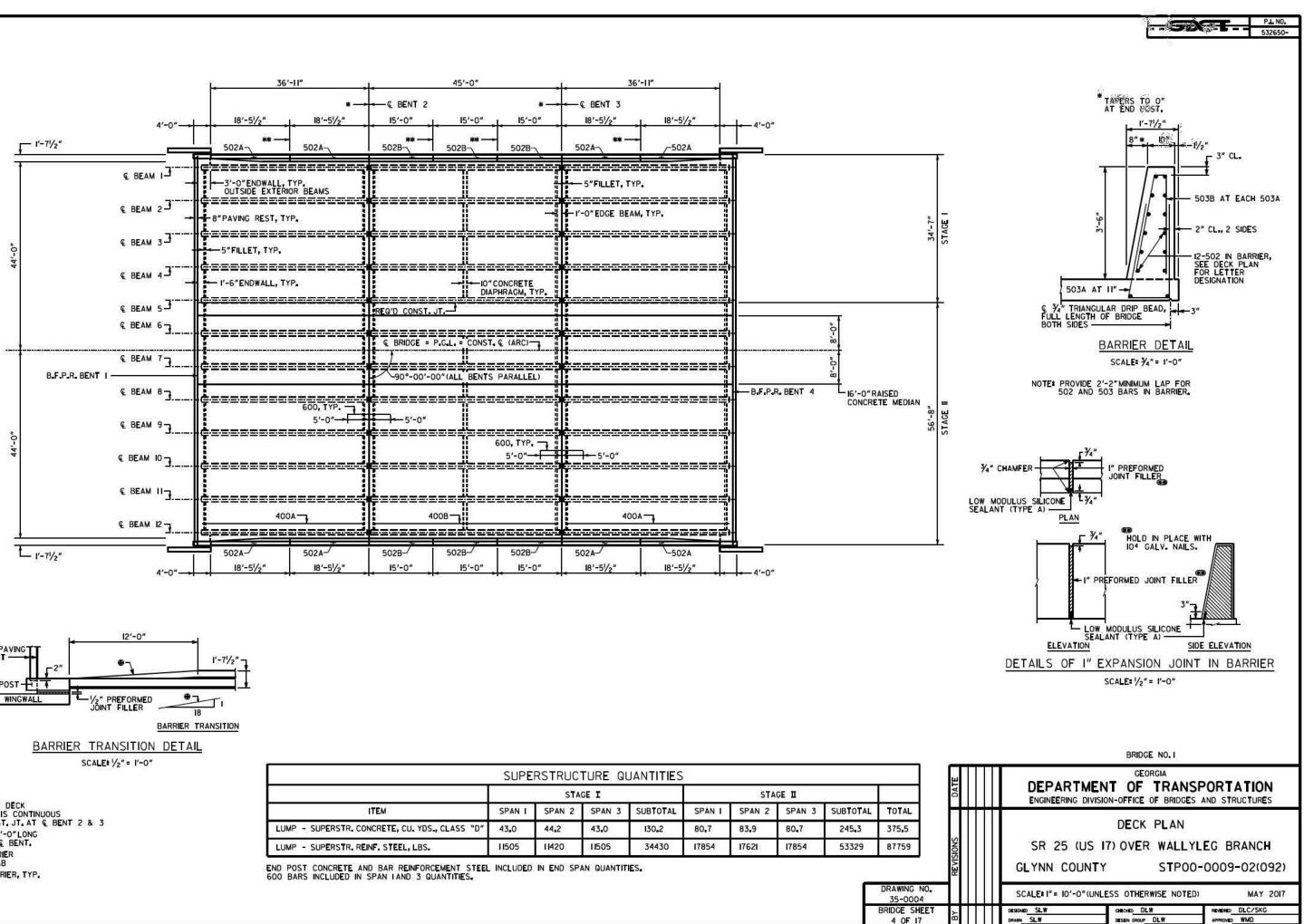
INCIDENTAL [TEMS-INCLUDE THE COST [NCIDENTAL TO THE WORK THAT IS NOT SPECIF]CALLY COVERED BY THE GEORGIA STANDARD SPECIFICATIONS, SUPPLEMENTAL SPECIFICATIONS AND/OR SPECIAL PROVISIONS IN THE OVERALL BID SUBMITTED. THIS INCLUDES THE COST OF WATERPROOFING, JOINT FILLERS AND OTHER INCIDENTAL ITEMS NECESSARY TO COMPLETE THE WORK.

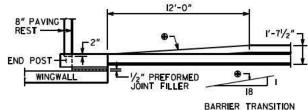
STEEL DIAGRAMS- SUBSTITUTION FOR STEEL DIAPHRAGMS IS NOT ALLOWED FOR THIS BRIDGE.

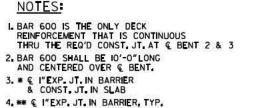


DESIGN DATA
(DESIGNED FOR SEISMIC PERFORMANCE ZONE 2, SDI = 0.165)
LIVE LOAD HL-93
ALLOWANCE 30 LBS PER SQ FT
RSTRUCTURE CLASS D, $f_c = 4,000$ PSI IER CLASS D, $f_c = 4,000$ PSI BEAMS CLASS AAA, $f_c = SEE$ BEAM SHEETS BEAM ALLOWABLE TENSION SEE BEAM SHEETS TRUCTURE CLASS A, $f_c = 3,000$ PSI
STEEL: GRADE 60, fy = 60,000 PSI
STRANDS: fs = 270,000 PSI
SUMMARY OF QUANTITIES
TITY UNIT PAY ITEM
945 SY GROOVED CONCRETE
LUMP LS SUPERSTR CONCRETE, CL D, BR NO - I (376)
238 LF CONCRETE BARRIER
IOI CY CLASS A CONCRETE
1458 LF PSC BEAMS, AASHTO TYPE I MOD, BR NO - I
2022 LB BAR REINF STEEL
LUMP LS SUPERSTR REINF STEEL, BR ND - I (87759)
990 LF PILING, PSC - SS REINF, 14 IN SQ
1045 LF P[LING, PSC - SS RE]NF, 16 [N SQ
I EA TEST PILE, PSC - SS REINF, 14 IN SQ
I EA TEST PILE, PSC - SS REINF, 16 [N SQ
2 EA DYNAMIC PILE TEST
LUMP LS REMOVAL OF EXISTING BR, STA NO - 739+73
743 SY STN DUMPED RIP RAP, TP I, 24 IN
743 SY PLASTIC FILTER FABRIC
461 LF TEMPORARY BARRIER, METHOD NO. 2

8	BRIDGE NO. I
DATE	GEORGIA DEPARTMENT OF TRANSPORTATION ENGINEERING DIVISION-OFFICE OF BRIDGES & STRUCTURES
REVISIONS	CONSTRUCTION STAGING SR 25 (US I7) OVER WALLYLEG BRANCH GLYNN COUNTY STP00-0009-02(092)
	NO SCALE MAY 2017
ВΥ	DESIDNED SLW CHECKED DLW REVENED DLC/SKG DRAWN SLW DESIDN GROUP DLW APPROVED WMD

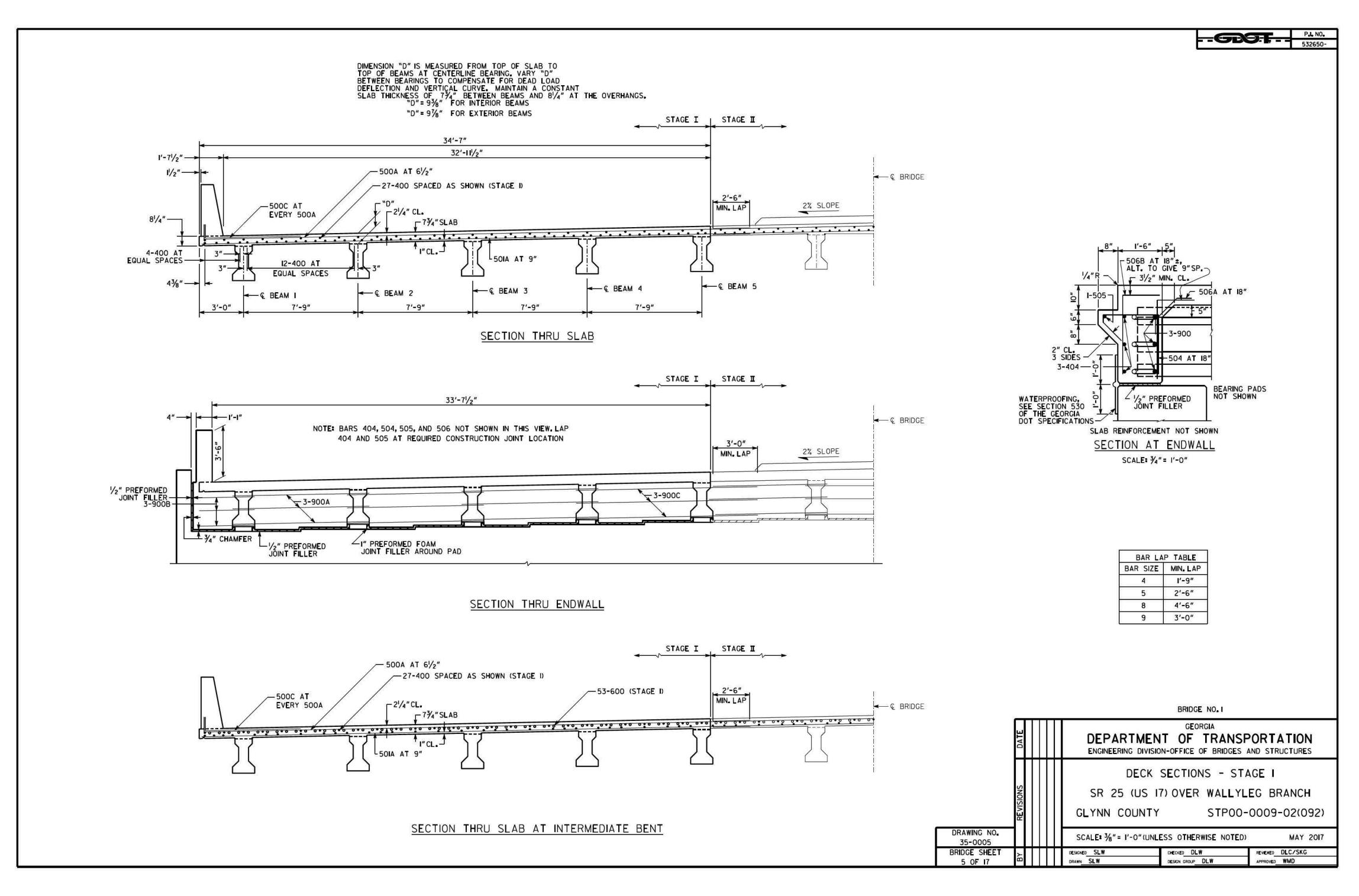


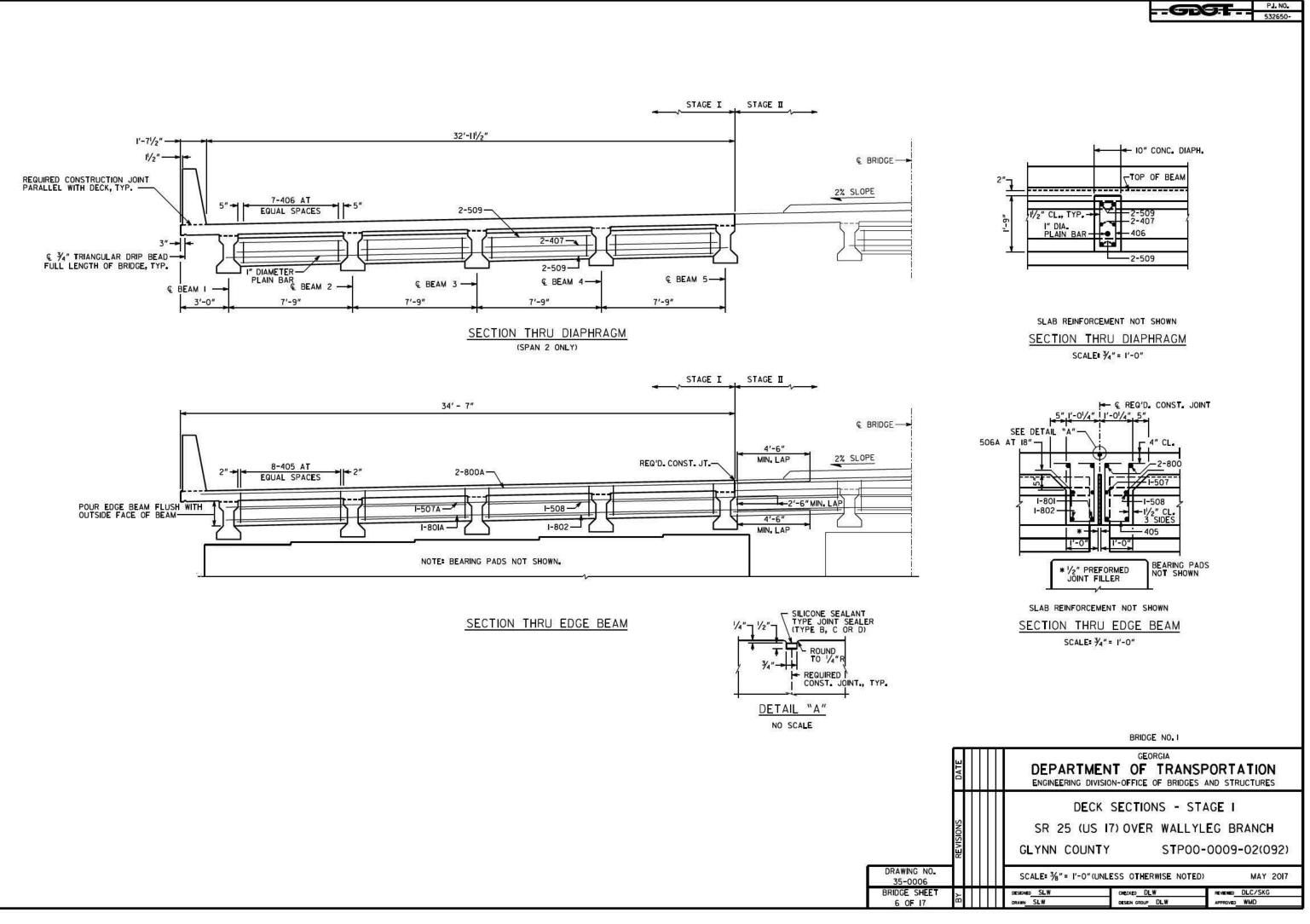


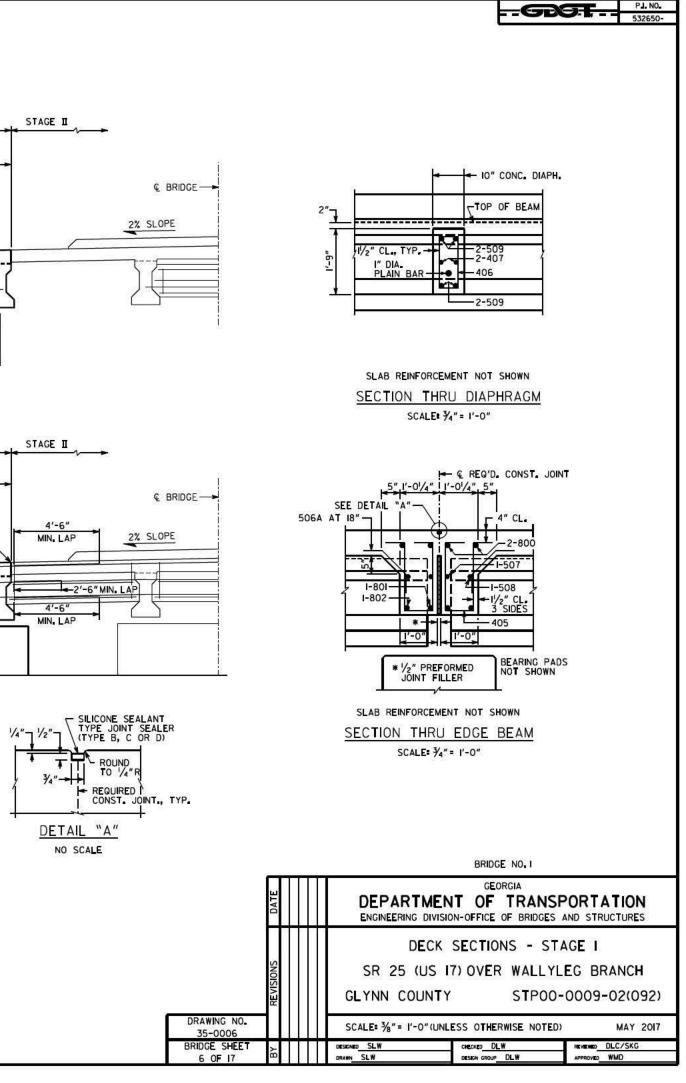


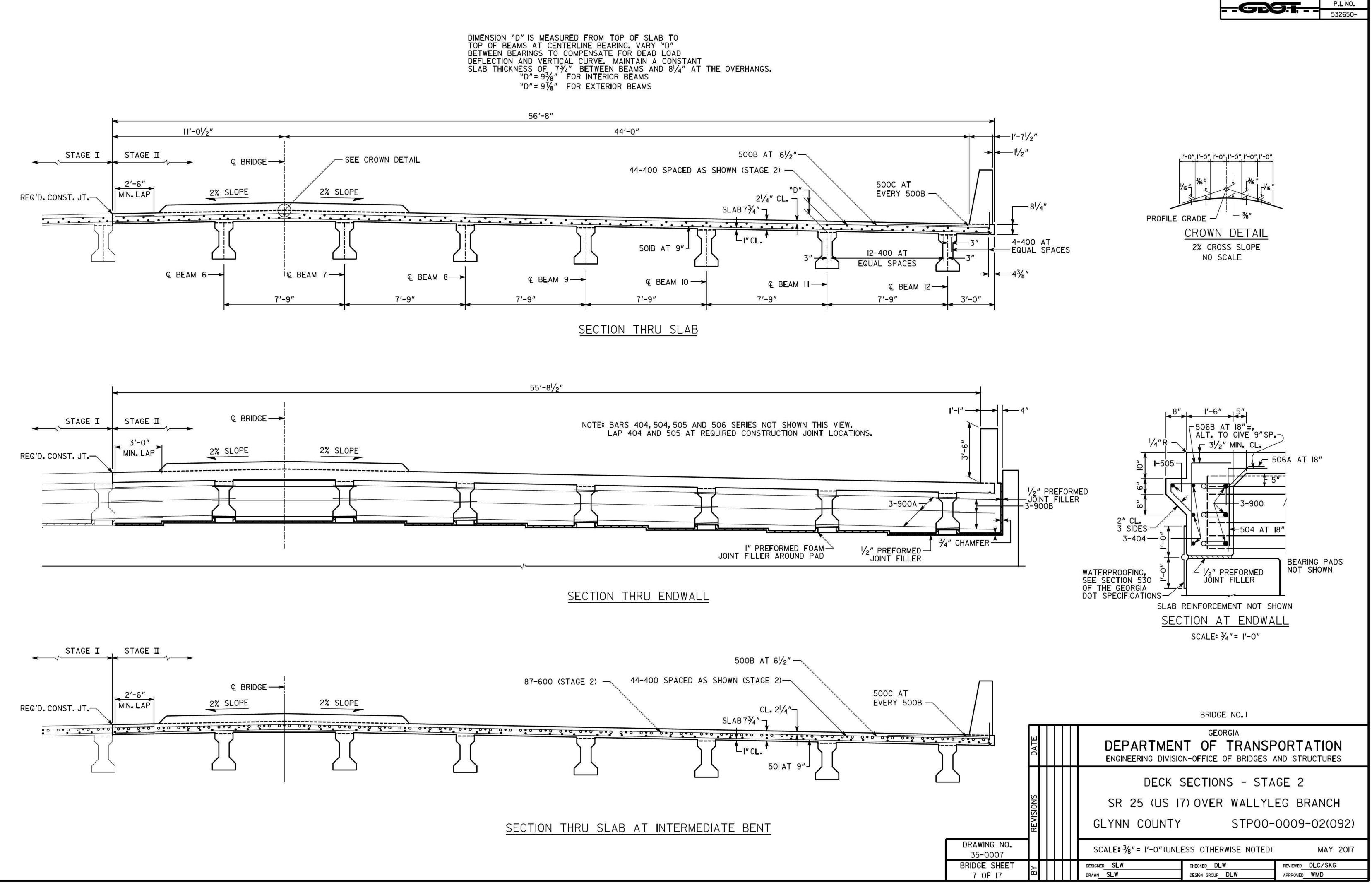
	SUPERSTRUCTURE QUANTITIES							
		STA	G e I			GEII	- 1940 -	
ITEM	SPAN I	SPAN 2	SPAN 3	SUBTOTAL	SPAN I	SPAN 2	SPAN 3	SUBT
LUMP - SUPERSTR. CONCRETE, CU. YDS., CLASS "D"	43.0	44.2	43.0	130,2	80.7	83.9	80.7	24
LUMP - SUPERSTR. REINF. STEEL, LBS.	11505	11420	11505	34430	17854	17621	17854	53

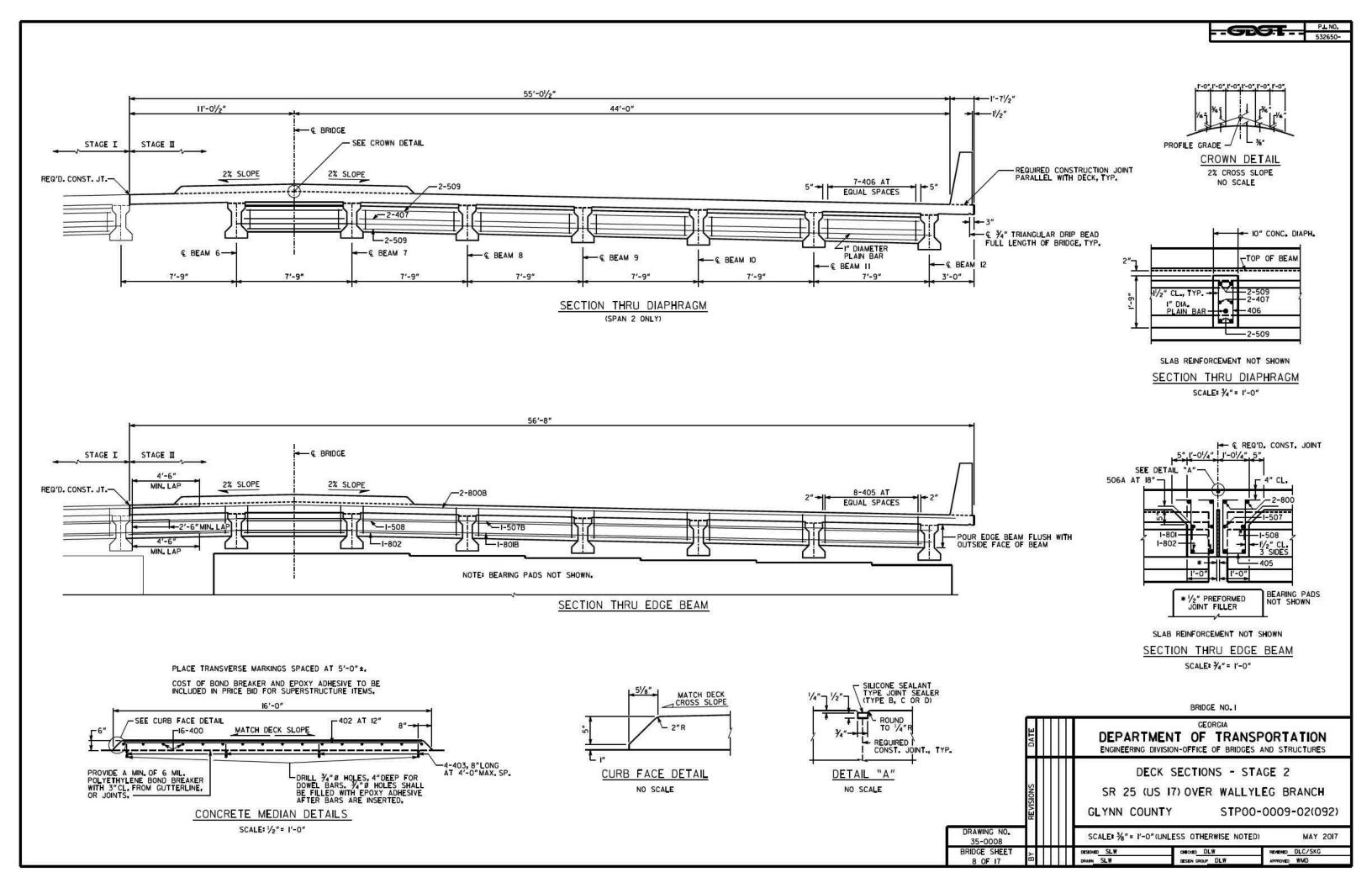
4 OF 17



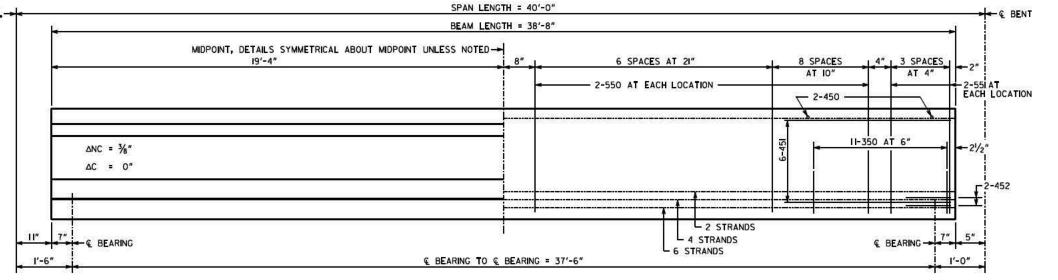








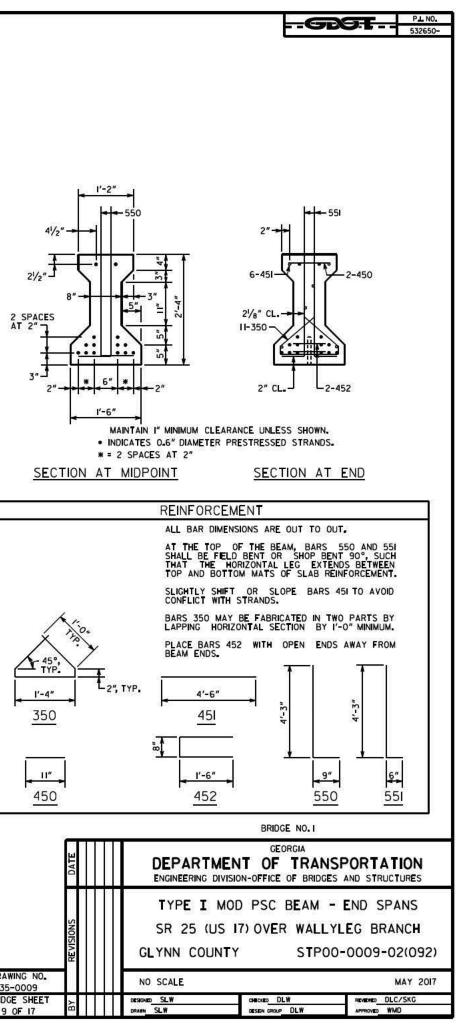




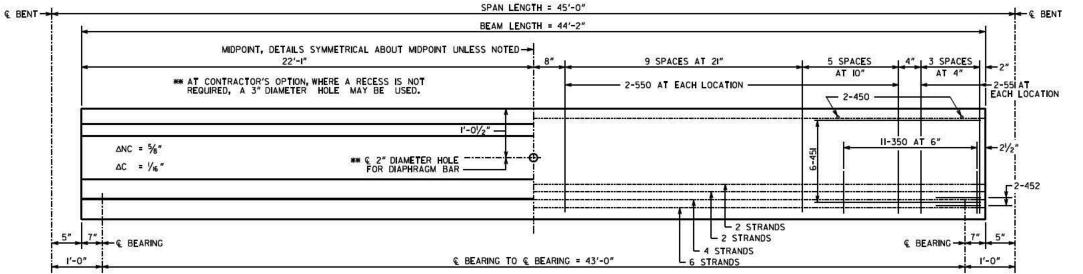
ELEVATION

NOTES

- I. BEAMS SHALL BE MAINTAINED IN AN UPRIGHT POSITION AT ALL TIMES AND SHALL BE PICKED UP WITHIN 3'-6" FROM THEIR ENDS. DISREGARDING THIS REQUIREMENT COULD LEAD TO COLLAPSE OF THE BEAM. PICK-UPS SHALL BE EMBEDDED TO WITHIN 4" OF THE BOTTOM OF THE BEAM. DETAILS OF PICK-UPS SHALL BE INCLUDED IN THE SHOP DRAWINGS.
- 2. CHAMFER EDGES OF BEAMS 1/2" OR 3/4".
- 3. HORIZONTAL DIMENSIONS ARE IN PLACE DIMENSIONS. THE BEAM LENGTH INCLUDES THE ${\cal Y}_8{}''$ EPOXY MORTAR AT EACH END. SHOP DRAWINGS SHALL ADJUST HORIZONTAL DIMENSIONS FOR GRADE AND FABRICATION EFFECTS SUCH AS SHRINKAGE AND ELASTIC SHORTENING.
- 4. AT & BEARING, FORM A I ¼" DIAMETER X 7" DEEP HOLE AT THE FIXED ENDS AND A 4" X I ¼" X 7" DEEP SLOT AT THE EXPANSION ENDS FOR A I ½" DIAMETER SMOOTH DOWEL, SEE PLAN AND ELEVATION SHEET FOR LOCATION OF FIXED AND EXPANSION ENDS.
- 5. TOPS OF BEAMS SHALL BE ROUGH FLOATED AT APPROXIMATELY THE TIME OF INITIAL SET. ENTIRE TOP SHALL BE SCRUBBED TRANSVERSELY WITH A COARSE BRUSH TO REMOVE ALL LAITANCE AND TO PRODUCE A ROUGHENED SURFACE FOR BONDING TO THE SLAB. ROUGHENED SURFACE SHALL HAVE AN AMPLITUDE OF APPROXIMATELY 1/4". CONCRETE FINS OR PROJECTIONS SHALL BE REMOVED TO PRODUCE A VERTICAL FACE AT THE FORCE OF THE FORM FACE AT THE EDGE OF THE BEAM.
- 6. NON-COMPOSITE DEAD LOAD DEFLECTION (ANC) AT THE MIDPOINT IS DUE TO THE WEIGHT OF THE SLAB AND COPING.
- 7. COMPOSITE DEAD LOAD DEFLECTION (AC) AT THE MIDPOINT IS DUE TO THE WEIGHT OF BARRIER AND RAISED MEDIAN.
- 8. STRANDS SHALL MEET ALL REQUIREMENTS OF ASTM A 416 GRADE 270.
- 9. PRESTRESSING DATA IS AS FOLLOWS:
 - A. USE 14 0.6" DIAMETER LOW-RELAXATION (A = 0.217 SQ IN) STRANDS, PRETENSION STRANDS TO 43,943 LBS EACH.
 - B. PRETENSIONED STRANDS SHALL BE RELEASED AFTER THE CONCRETE HAS REACHED A MINIMUM STRENGTH (fci) OF 5,500 PSI.
 - C. INCLUDING THE TOP STRANDS, THE TOTAL JACKING FORCE OF PRETENSIONING IS 615,202.
 - D. INCLUDING THE TOP STRANDS, THE NET PRESTRESSING FORCE OF THE STRANDS AFTER ALL LOSSES IS 492,188 LBS,
- IO. CONCRETE STRENGTH (fc) = 6,000 PSI.
- IL ALLOWABLE PSC BEAM TENSION = 465 PSL



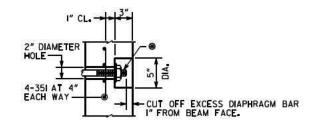
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-	9 OF 1	7



ELEVATION

NOTES

- I. BEAMS SHALL BE MAINTAINED IN AN UPRIGHT POSITION AT ALL TIMES AND SHALL BE PICKED UP WITHIN 3'-6" FROM THEIR ENDS, DISREGARDING THIS REQUIREMENT COULD LEAD TO COLLAPSE OF THE BEAM. PICK-UPS SHALL BE EMBEDDED TO WITHIN 4" OF THE BOTTOM OF THE BEAM, DETAILS OF PICK-UPS SHALL BE INCOLUCED IN THE COLD DO ANNOS INCLUDED IN THE SHOP DRAWINGS.
- 2. CHAMFER EDGES OF BEAMS 1/2" OR 3/4".
- 3. HORIZONTAL DIMENSIONS ARE IN PLACE DIMENSIONS. THE BEAM LENGTH INCLUDES THE V_8 " EPOXY MORTAR AT EACH END. SHOP DRAWINGS SHALL ADJUST HORIZONTAL DIMENSIONS FOR GRADE AND FABRICATION EFFECTS SUCH AS SHRINKAGE AND ELASTIC SHORTENING.
- 4. AT & BEARING, FORM A I ¼" DIAMETER X 7" DEEP HOLE AT THE FIXED ENDS AND A 4" X I ¼" X 7" DEEP SLOT AT THE EXPANSION ENDS FOR A I ½" DIAMETER SMOOTH DOWEL, SEE PLAN AND ELEVATION SHEET FOR LOCATION OF FIXED AND EXPANSION ENDS.
- 5. TOPS OF BEAMS SHALL BE ROUGH FLOATED AT APPROXIMATELY THE TIME OF INITIAL SET. ENTIRE TOP SHALL BE SCRUBBED TRANSVERSELY WITH A COARSE BRUSH TO REMOVE ALL LAITANCE AND TO PRODUCE A ROUGHENED SURFACE FOR BONDING TO THE SLAB. ROUGHENED SURFACE SHALL HAVE AN AMPLITUDE OF APPROXIMATELY 1/4". CONCRETE FINS OR PROJECTIONS SHALL BE REMOVED TO PRODUCE A VERTICAL FACE AT THE FORCE OF THE FORM FACE AT THE EDGE OF THE BEAM.
- 6. NON-COMPOSITE DEAD LOAD DEFLECTION (ANC) AT THE MIDPOINT IS DUE TO THE WEIGHT OF THE SLAB AND COPING.
- 7. COMPOSITE DEAD LOAD DEFLECTION (AC) AT THE MIDPOINT IS DUE TO THE WEIGHT OF BARRIER AND RAISED MEDIAN.
- 8. STRANDS SHALL MEET ALL REQUIREMENTS OF ASTM A 416 GRADE 270.
- 9. PRESTRESSING DATA IS AS FOLLOWS:
 - A. USE 16 0.6" DIAMETER LOW-RELAXATION (A = 0.217 SQ IN) STRANDS. PRETENSION STRANDS TO 43,943 LBS EACH.
 - B. PRETENSIONED STRANDS SHALL BE RELEASED AFTER THE CONCRETE HAS REACHED A MINIMUM STRENGTH (fci) OF 6,000 PSI.
 - C. INCLUDING THE TOP STRANDS, THE TOTAL JACKING FORCE OF PRETENSIONING IS 703,088 LBS.
 - D. INCLUDING THE TOP STRANDS, THE NET PRESTRESSING FORCE OF THE STRANDS AFTER ALL LOSSES IS 559,107 LBS.
- IO. CONCRETE STRENGTH (fc) = 6,500 PSL
- II. ALLOWABLE PSC BEAM TENSION = 484 PSI.



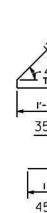
 $\textcircled{\label{eq:constraint}$ B Diaphragm bar shall be a 1" diameter plain bar, threaded 5" on each end, with 1/4" x 31/2" diameter washers and hex nuts (astm a 709 grade 36).

TIGHTEN DIAPHRAGM BAR AS PER SUB-SECTION 507.3.05.C OF THE GEORGIA DOT SPECIFICATIONS.

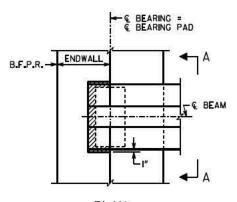
AFTER EXCESS DIAPHRAGM BAR HAS BEEN CUT OFF, PAINT DIAPHRAGM BAR, WASHER, AND NUT EXPOSED IN RECESS WITH SPECIAL PROTECTIVE COATING NO.2 P AS PER SECTION 535 OF THE GEORGIA DOT SPECIFICATIONS. AFTER PAINTING, FLL THE RECESS WITH AN APPROVED EPOXY GROUT.

GALVANIZING OF THE DIAPHRAGM BAR AS PER SUB-SECTION 865.2.0I.B.I2 OF THE GEORGIA DOT SPECIFICATIONS IS NOT REQUIRED.

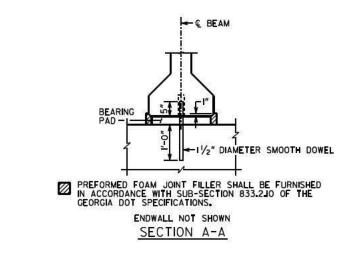
RECESS DETAIL FOR DIAPHRAGM BAR ENDS



		GR	PL NO. 532650-
NT			
l ⊲ ^{l'-2"}	-¥		
	- 550	 55I	
N 41/2"	2	2″ →>	
21/2"		451 (+ + + 2	-450
8"		···) [·	
3 SPACES	5" = 7 2½" II-35	X	
	<u>N </u>		
3"-			
2"	* 2″ 2	2″ CL.J L2-452	
<mark>⊸ 1′-6″</mark>			
• IND	AINTAIN I" MINIMUM CLEARANCE I DICATES 0.6" DIAMETER PRESTR 2 SPACES AT 2"		
SECTION AT		ECTION AT E	ND
jut.		_	
	REINFORCEMENT ALL BAR DIMENSIONS		
	AT THE TOP OF THI SHALL BE FIELD BEN		0 AND 551
	THAT THE HORIZON TOP AND BOTTOM MA	ITAL LEG EXTENDS	BETWEEN
	SLIGHTLY SHIFT OR CONFLICT WITH STRAM	SLOPE BARS 451 NDS.	TO AVOID
10.	BARS 350 MAY BE F LAPPING HORIZONTAL	ABRICATED IN TWO I L SECTION BY I'-0'	PARTS BY "MINIMUM.
15.0	PLACE BARS 452 WI BEAM ENDS	TH OPEN ENDS A	WAY FROM
45°, TYP.	a ana 2 <mark>1</mark>		
≪ 1-4 →	2", TYP. 4'-6"	3"	4'-3"
350	451	4'-3"	4'.
	" ⁸ "		
< 11″ >	< ''−6″	< 9″ -	6"
450	452	550	551
87		BRIDGE NO.I	
DATE	DEPARTMENT		ORTATION
à	ENGINEERING DIVISION-OF		2014년 - 1917년 - 1917년 - 1917년 -
	TYPE IMOD PSC B	EAM - INTERN	EDIATE SPAN
REVISIONS	SR 25 (US 17) 0	VER WALLYLE	G BRANCH
RE	GLYNN COUNTY	STP00-0	0009-02(092)
DRAWING NO. 35-0010	NO SCALE		MAY 2017
BRIDCE SHEET > IO OF I7 ☆	CONTRACT CONTRACT OF A DECISION OF A DECISIONO OF A DECISIO	NED DLW CRICEROUP DLW	APPROVED BLC/SKG







 BEARING PADS HAN SECTION 14.7.6 ME CONSTRUCTION SPI
 1½" DIAMETER SM
 BEARING PADS SHA
 3" DIAMETER HOLE
 BEARING PADS SHA
 3" LOAD PLATE GRADE 36 OR AST
 NUMBER OF INTERN PLATE(S) SPECIFIEI

8.	USE	OF	1/2"	MOLD	DRAI

	BEARING PADS											
						DESIG	N LOADS	(KIPS)				
BENT	W	L	Т	NUMBER OF INTERNAL PLATE(S)	DESIGN SHEAR DEFLECTION	DEAD LOAD	LIVE LOAD (NO IMPACT)	DEAD LOAD				
I	14″	9"	23⁄4″	3	1/4"	40,9	52,7	93.6				
2B	I4″	9"	2∛4″	3	0"	40.9	52.7	93.6				
24	I4″	9"	2¾"	3	0"	45.5	55.9	101_4				
3B	14″	9"	27/4"	3	5∕16 ″	45.5	55.9	101.4				
3A	14″	9"	27/4"	3	5⁄16 ″	40.9	52.7	93_6				
4	I4″	9"	23⁄4"	3	%6 "	40.9	52.7	93.6				

	1	
B & BEAM 7 90°-00'-00", TYP. — %" SEALING RIB, ALL AROUND		B
	PLAN	NG PAD = Meter Hole
% " LOAD PLAT TOP AND BOTT MG " SEALING RIB, TOP AND BOTTOM		GAGE TERNAL ATES
	SECTION B-B	

L/2 1 L/2

BEARING PAD



I. BEARING PADS HAVE BEEN DESIGNED ACCORDING TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, SECTION 14.7.6 METHOD A AND SHALL BE FURNISHED IN ACCORDANCE WITH AASHTO LRFD BRIDGE CONSTRUCTION SPECIFICATIONS, SECTION 18, BEARING DEVICES. P.L.NO.

532650-

-GBGT-

2. 1/2" DIAMETER SMOOTH DOWELS SHALL BE ASTM A 709 GRADE 50.

3. BEARING PADS SHALL BE MADE OF 60 DUROMETER HARDNESS NEOPRENE, GRADE 2 OR HIGHER.

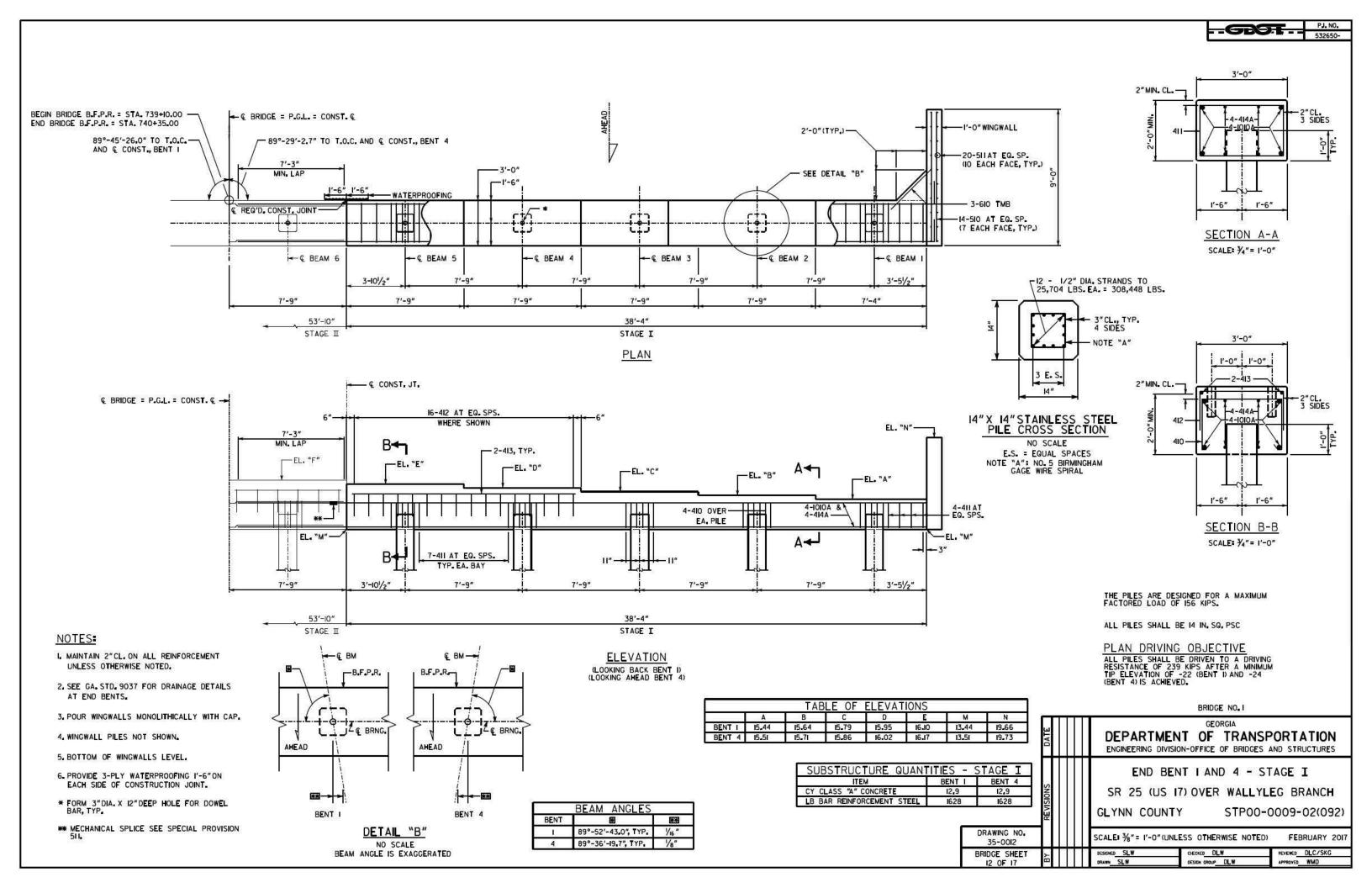
4. 3" DIAMETER HOLE IN BEARING PADS MAY BE FORMED OR DRILLED.

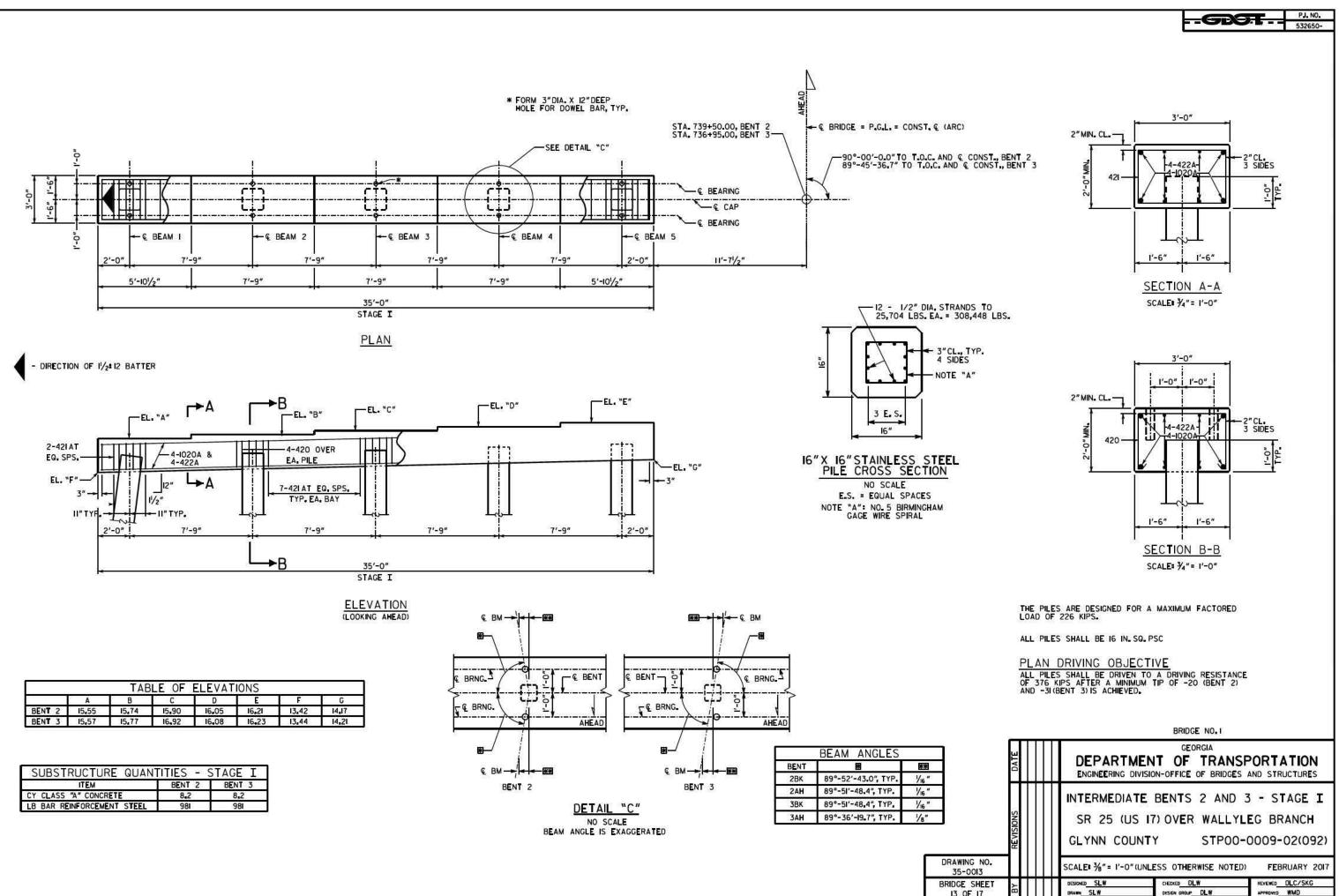
5. BEARING PADS SHALL HAVE $\frac{1}{4}$ " COVER ON THE TOP, BOTTOM, AND SIDES AND AROUND THE HOLE. 6. $\frac{3}{6}$ " LOAD PLATES AND 12 GAGE INTERNAL PLATE(S) (IF REQUIRED) SHALL BE ASTM A 709 GRADE 36 OR ASTM A IOII GRADE 36.

7. NUMBER OF INTERNAL PLATES SHOWN FOR ILLUSTRATION PURPOSES ONLY. THE NUMBER OF INTERNAL PLATE(S) SPECIFIED SHALL BE EQUALLY SPACED BETWEEN LOAD PLATES.

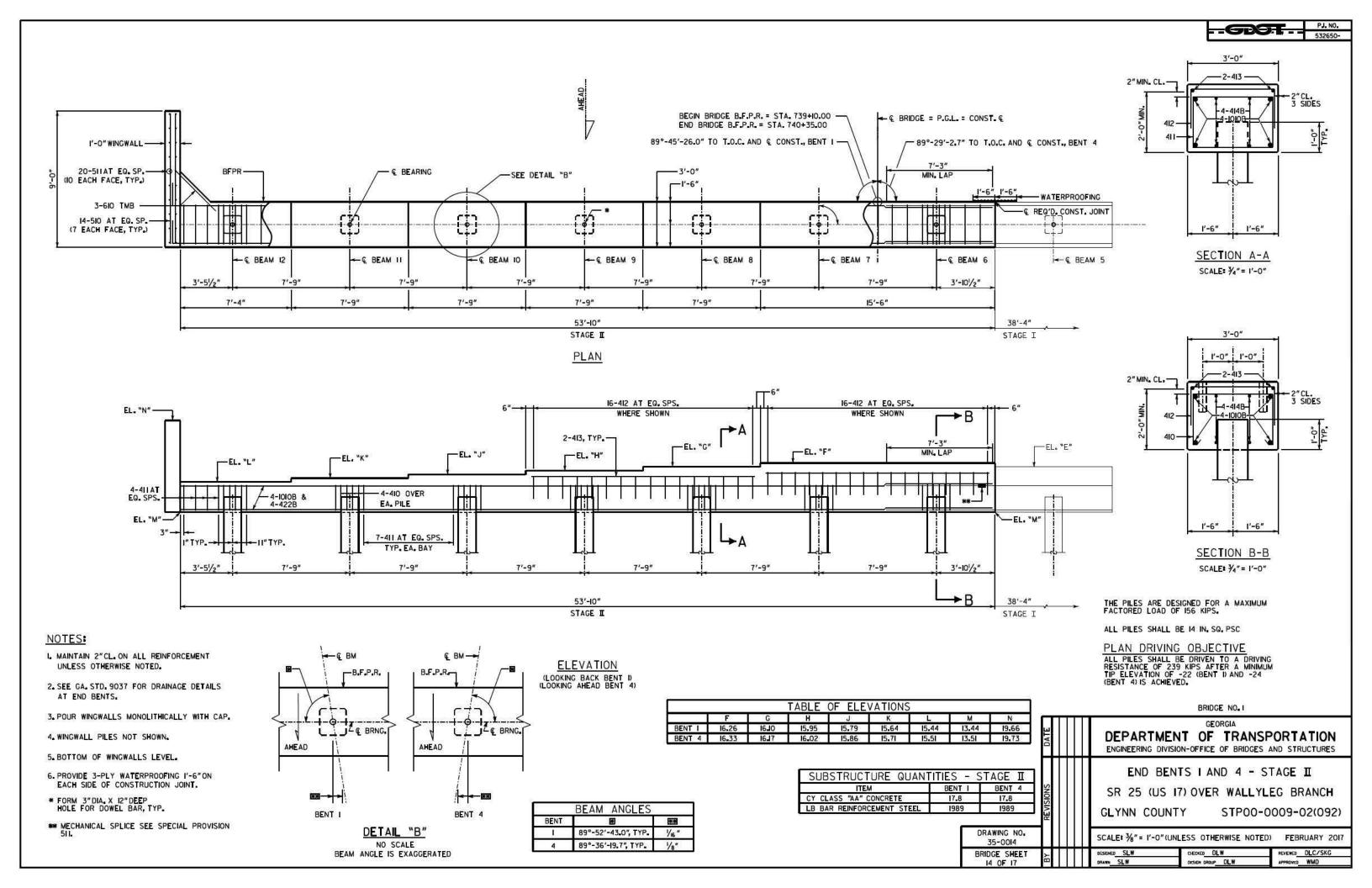
т	IS	OPTIONAL.

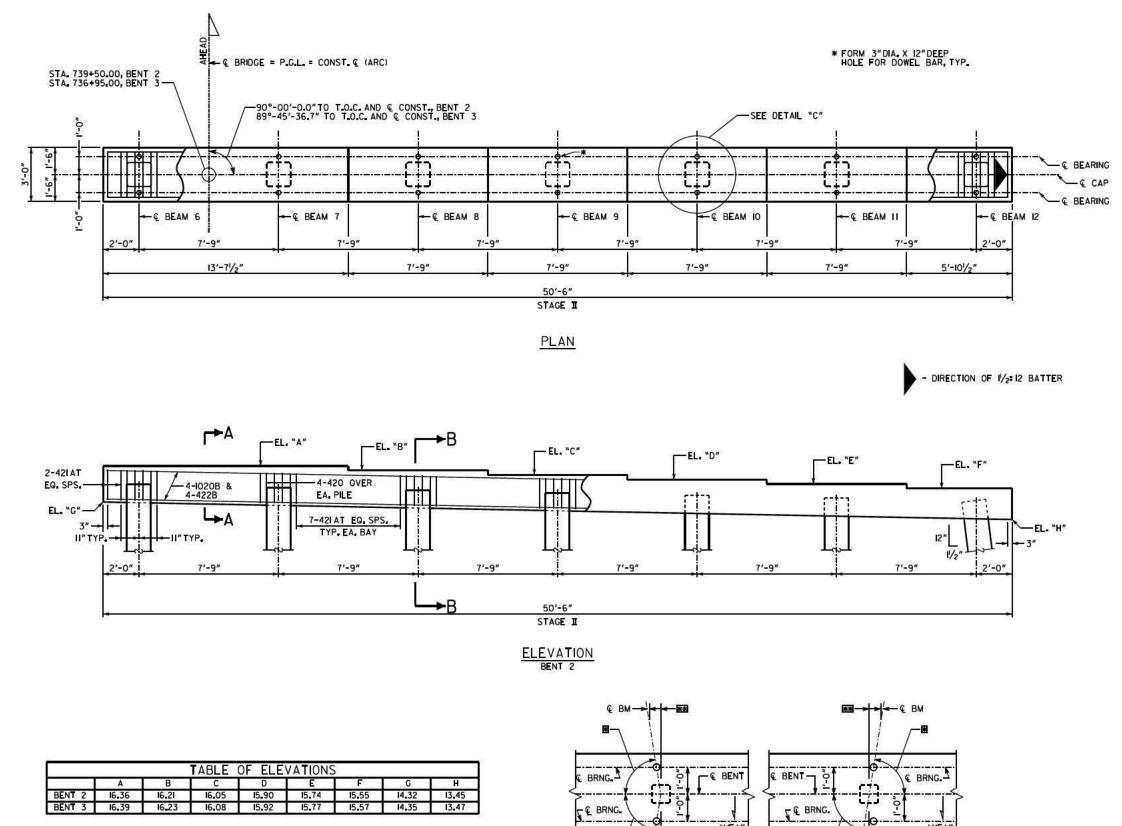
8		BRIDGE NO. I	
DATE	************************************	GEORGIA ENT OF TRAN VISION-OFFICE OF BRIDGI	
REVISIONS	DiFi	EARING PAD DE 17) OVER WALL TY STPC	2012
	NO SCALE		MAY 2017
ВΥ	DESIGNED SLW DRAWN SLW	CHECKED DLW DESEN GROUP DLW	REVENED DLC/SKG



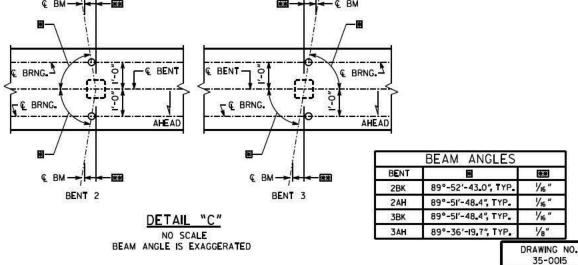


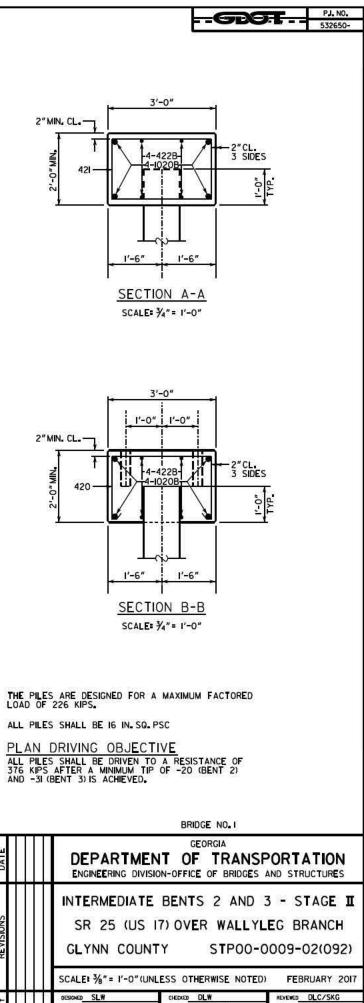
13 OF 17





SUBSTRUCTURE QUAN	TITIES - S	STAGE I
ITEM	BENT 2	BENT 3
CY CLASS "AA" CONCRETE	115	11.5
LB BAR REINFORCEMENT STEEL	1413	1413





DRAWN SLW

DESIGN GROUP DLW

APPROVED WMD

BRIDGE SHEET

15 OF 17

AS BUIL	T FOUNDATIC. STAGE	N INFORMATION
BENT NO.	PILE LOCATION	PILE TIP Elevation
	BEAM I	
	BEAM 2	
T F	BEAM 3	
	BEAM 4	
	BEAM 5	
	BEAM I	
800	BEAM 2	
2	BEAM 3	
E	BEAM 4	
	BEAM 5	
	BEAM I	
	BEAM 2	
3	BEAM 3	
Г	BEAM 4	
Г	BEAM 5	
	BEAM I	
	BEAM 2	
4	BEAM 3	
E	BEAM 4	
	BEAM 5	

Ť	STAGE	2
BENT NO.	PILE LOCATION	PILE TIP ELEVATION
	BEAM 6	
	BEAM 7	
1000	BEAM 8	
1	BEAM 9	
	BEAM IO	
	BEAM II	-
	BEAM 12	
	BEAM 6	
	BEAM 7	
	BEAM 8	
2	BEAM 9	
Ĩ	BEAM IO	
	BEAM II	8
	BEAM 12	2
	BEAM 6	
	BEAM 7	
Г	BEAM 8	
3	BEAM 9	
	BEAM IO	-
Г	BEAM II	
1	BEAM 12	
	BEAM 6	8
	BEAM 7	
	BEAM 8	
4	BEAM 9	
	BEAM IO	
	BEAM II	

2		BRIDGE NO. I	
DATE	한 바람이 다 나는 것 같아요. 가지 않는 것 같아요. 아이들 것 같아요. 아이들 것 같아요.	GEORGIA NT OF TRAN ON-OFFICE OF BRIDGE	
REVISIONS	SR 25 (US I	T FOUNDATIO 7) OVER WALL STPO	
	NO SCALE		MAY 2017
ВУ	DESIGNED SLW DRAWN SLW	CHECKED DLW DESIGN GROUP DLW	REVENED DLC/SKG

PROJECT ENGINEER

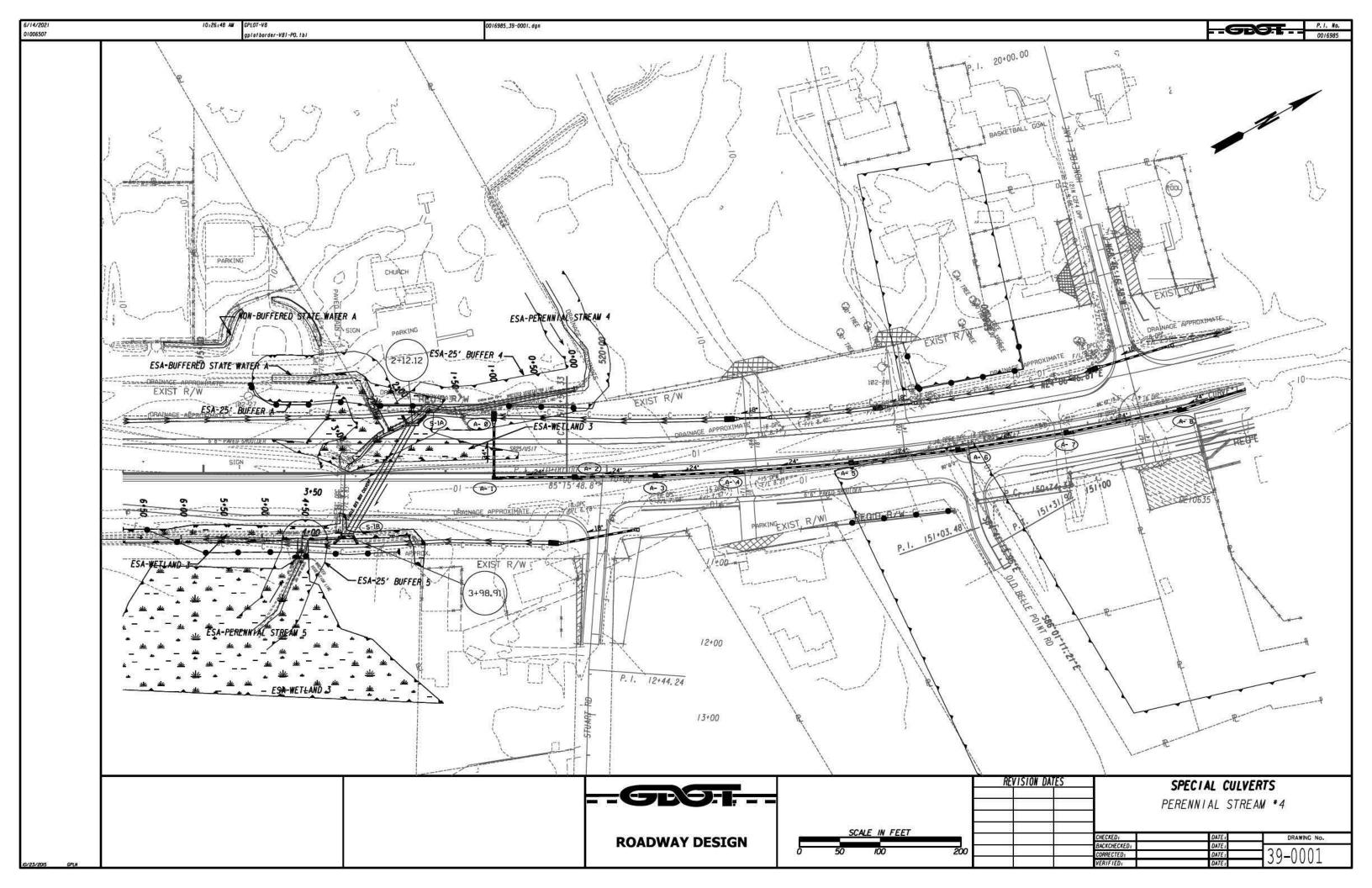
() (AREA CODE) DATE

TELEPHONE NUMBER

THIS SHEET IS TO BE FILLED IN BY THE PROJECT ENGINEER AND THE ENTIRE SHEET FORWARDED TO THE BRIDGE OFFICE UPON COMPLETION OF PILE DRIVING FOR POSTING TO THE PLANS AS A PERMANENT RECORD OF THE BRIDGE CONSTRUCTION.

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NOTE:

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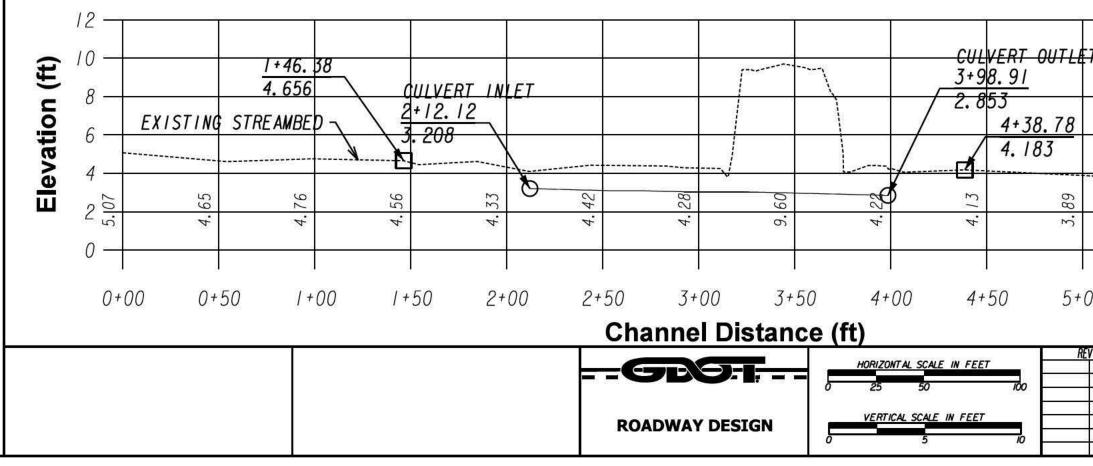
> THE PROFILE REPRESENTS THE EXISTING STREAM ALIGNMENT THROUGH AN EXISTING DOUBLE BARREL 32" CIRCULAR PIPE CULVERT. PREPRESENTATIVE CROSS SECTION LOCATIONS.

O - CROSS SECTION STATION

THE PERENNIAL STREAM *4 CROSSING IS A REALIGNED CULVERT REPLACEMENT.

THE PROPOSED STREAM ALIGNMENT THROUGH STREAM STATION 2+12 TO 3+97 IS NOT THE SAME AS THE EXISTING ALIGNMENT.

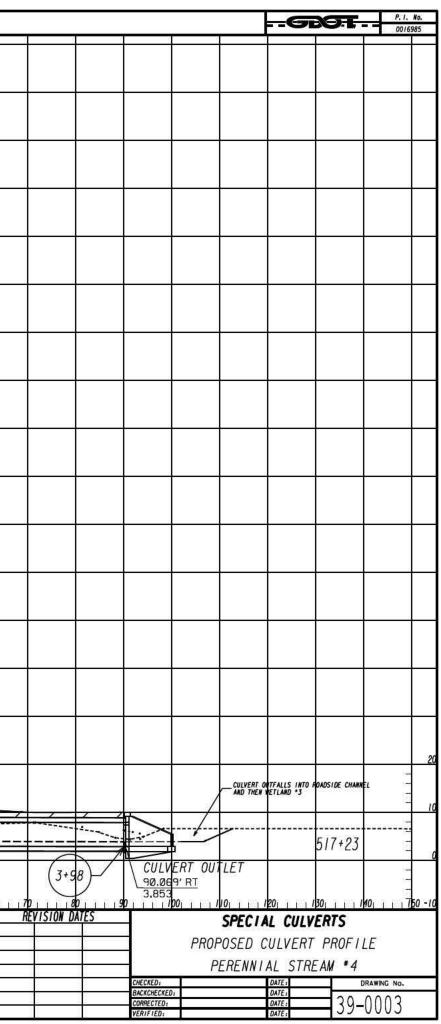
CROSS SECTIONS ARE CUT PERPENDICULAR TO THE EXISTING STREAM ALIGNMENT. THE CULVERT EXTENTS REPRESENTED IN THE CROSS SECTIONS ARE THE ACTUAL CULVERT LOCATION AND A PROJECTION FURTHER INTO THE CULVERT AS TO SHOW THE FULL EXTENT OF THE CULVERT BARREL.

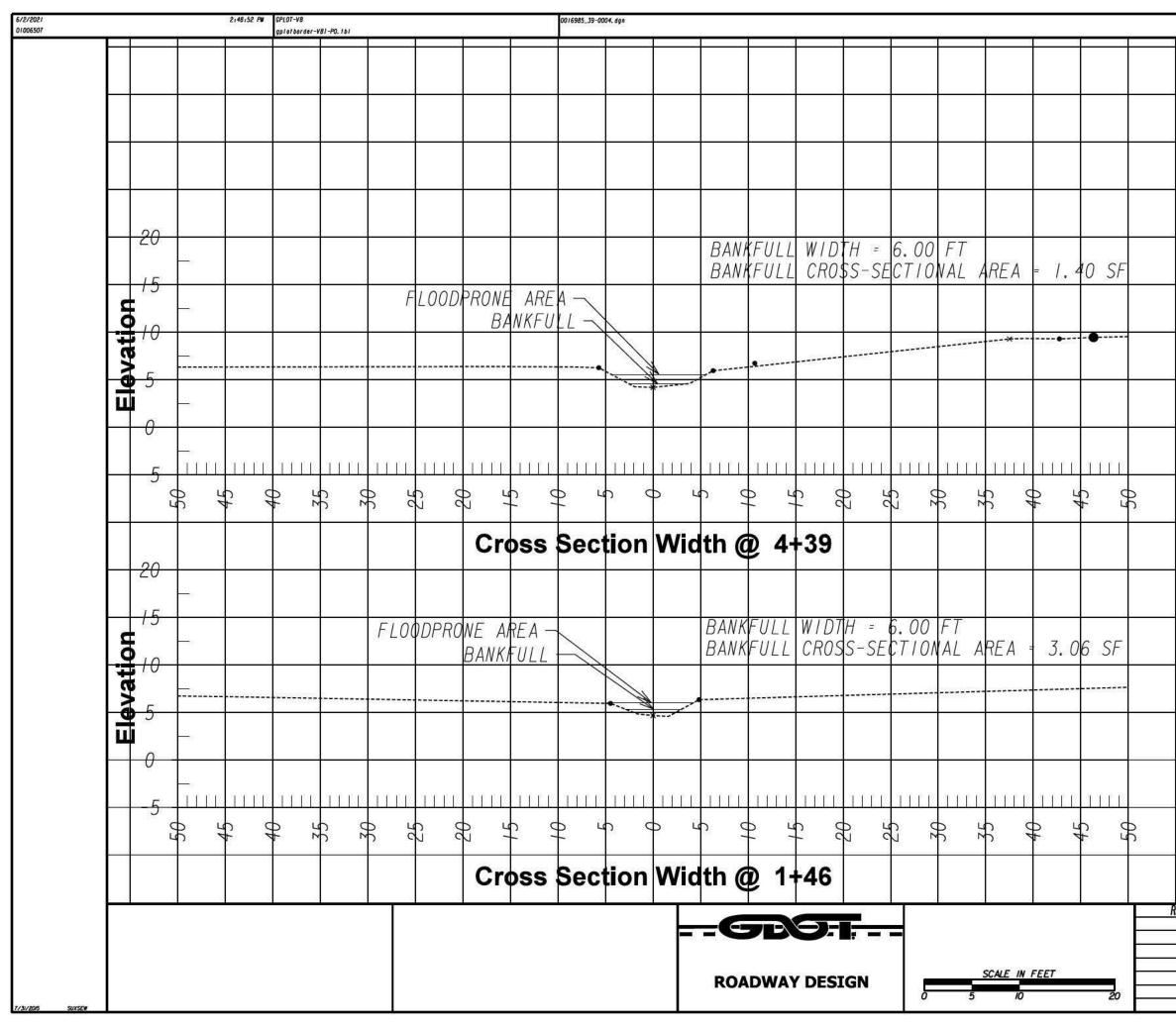


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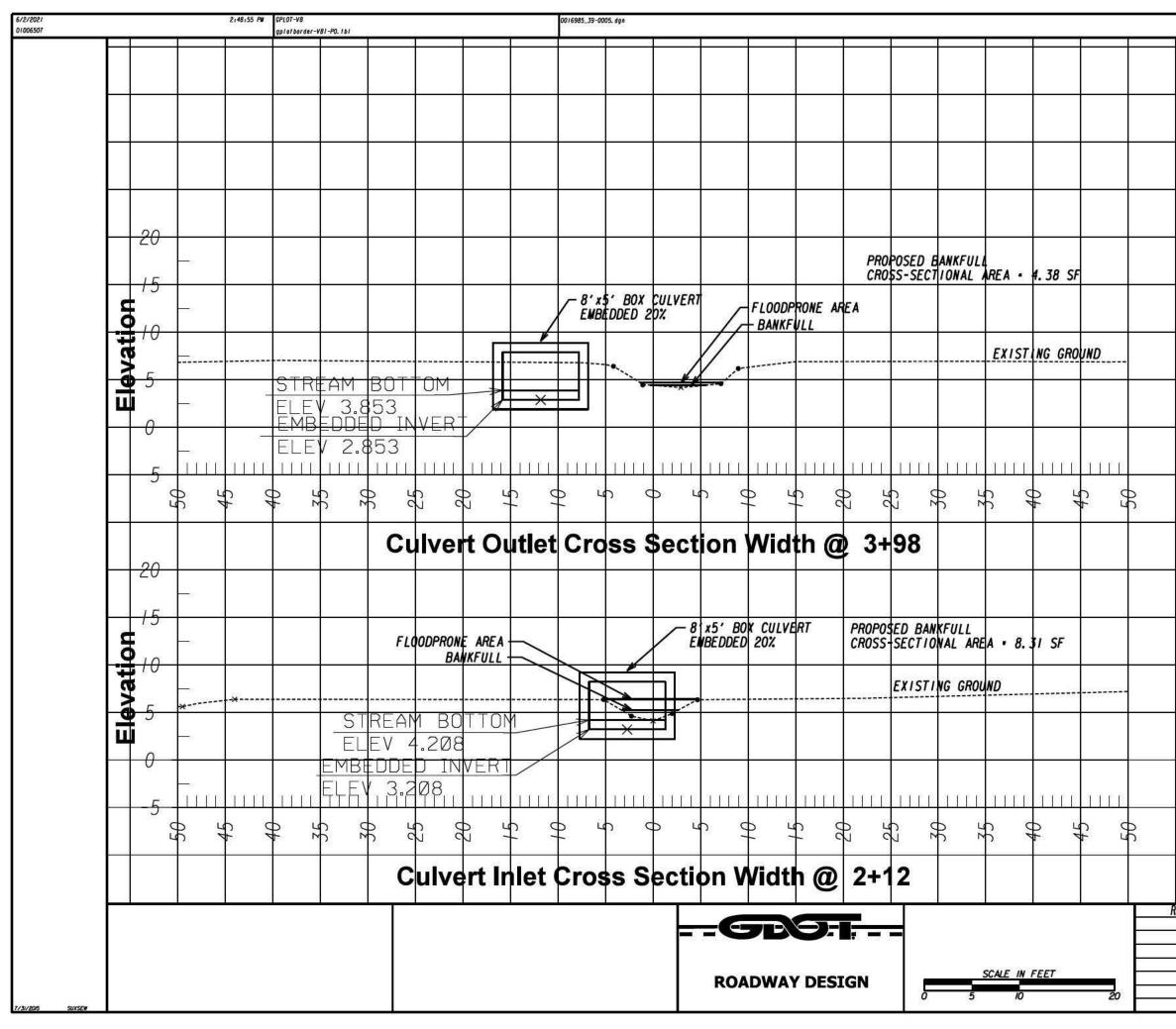
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